



water & sanitation

Department:
Water and Sanitation
REPUBLIC OF SOUTH AFRICA

P WMA 12/T30/00/5212/10

DIRECTORATE: OPTIONS ANALYSIS

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT

GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME

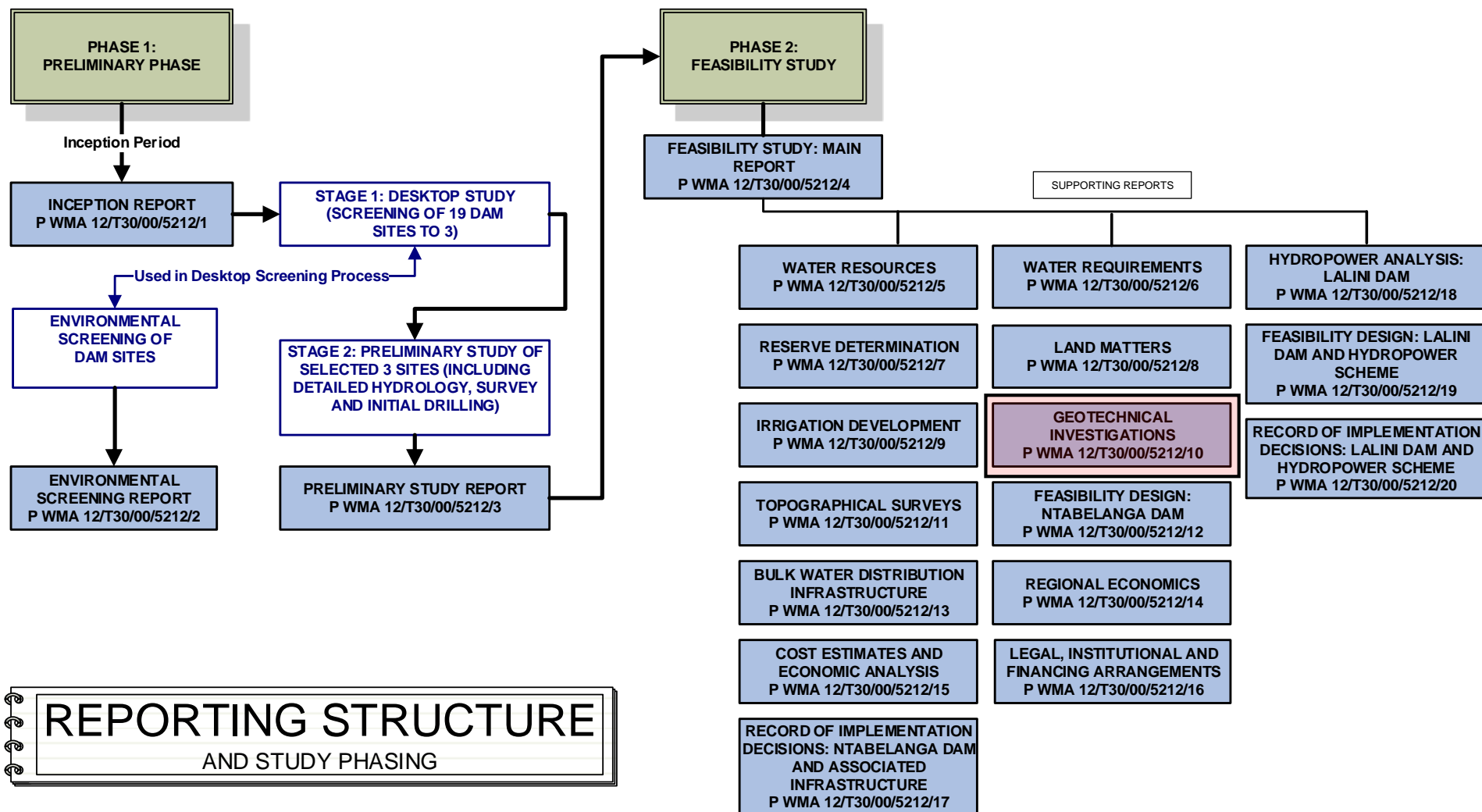
APPENDICES



OCTOBER 2014

LIST OF REPORTS

REPORT TITLE	DWS REPORT NUMBER
Inception Report	P WMA 12/T30/00/5212/1
Environmental Screening	P WMA 12/T30/00/5212/2
Preliminary Study	P WMA 12/T30/00/5212/3
Feasibility Study: Main Report	P WMA 12/T30/00/5212/4
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FEASIBILITY STUDY: SUPPORTING REPORTS:	
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Water Requirements	P WMA 12/T30/00/5212/6
Reserve Determination	P WMA 12/T30/00/5212/7
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Volume 4: Lalini Dam and Hydropower Scheme: Appendices	
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Feasibility Design: Ntabelanga Dam	P WMA 12/T30/00/5212/12
Bulk Water Distribution Infrastructure	P WMA 12/T30/00/5212/13
Regional Economics	P WMA 12/T30/00/5212/14
Cost Estimates and Economic Analysis	P WMA 12/T30/00/5212/15
Legal, Institutional and Financing Arrangements	P WMA 12/T30/00/5212/16
Record of Implementation Decisions: Ntabelanga Dam and Associated Infrastructure	P WMA 12/T30/00/5212/17
Hydropower Analysis: Lalini Dam	P WMA 12/T30/00/5212/18
Feasibility Design: Lalini Dam and Hydropower Scheme	P WMA 12/T30/00/5212/19
Record of Implementation Decisions: Lalini Dam and Hydropower Scheme	P WMA 12/T30/00/5212/20



REFERENCE

This report is to be referred to in bibliographies as:

*Department of Water and Sanitation (2014). **Feasibility Study for the Mzimvubu Water Project: Geotechnical Investigations: Lalini Dam and Hydropower Scheme: Appendices***

DWS Report No: P WMA 12/T30/00/5212/10

Prepared for: Directorate – Options Analysis

Prepared by: Jeffares & Green (Pty) Ltd, P O Box 794, Hilton, 3245

Tel: 033 343 6700, Fax: 033 343 6701

Contact: Mr A Pepperell

Email: pepperella@jgi.co.za

Note on Departmental Name Change:

In 2014, the Department of Water Affairs changed its name to the Department of Water and Sanitation, which happened during the course of this study. In some cases this was after some of the study reports had been finalized. The reader should therefore kindly note that references to the Department of Water Affairs and the Department of Water and Sanitation herein should be considered to be one and the same.

Note on Spelling of Laleni:

The settlement named Laleni on maps issued by the Surveyor General is locally known as Lalini and both names therefore refer to the same settlement.

VOLUME 4

APPENDICES

APPENDIX A: SITE PLANS

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WATER PRESSURE TESTS**

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APPENDIX E: LABORATORY TEST RESULTS

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APPENDIX A

SITE PLANS

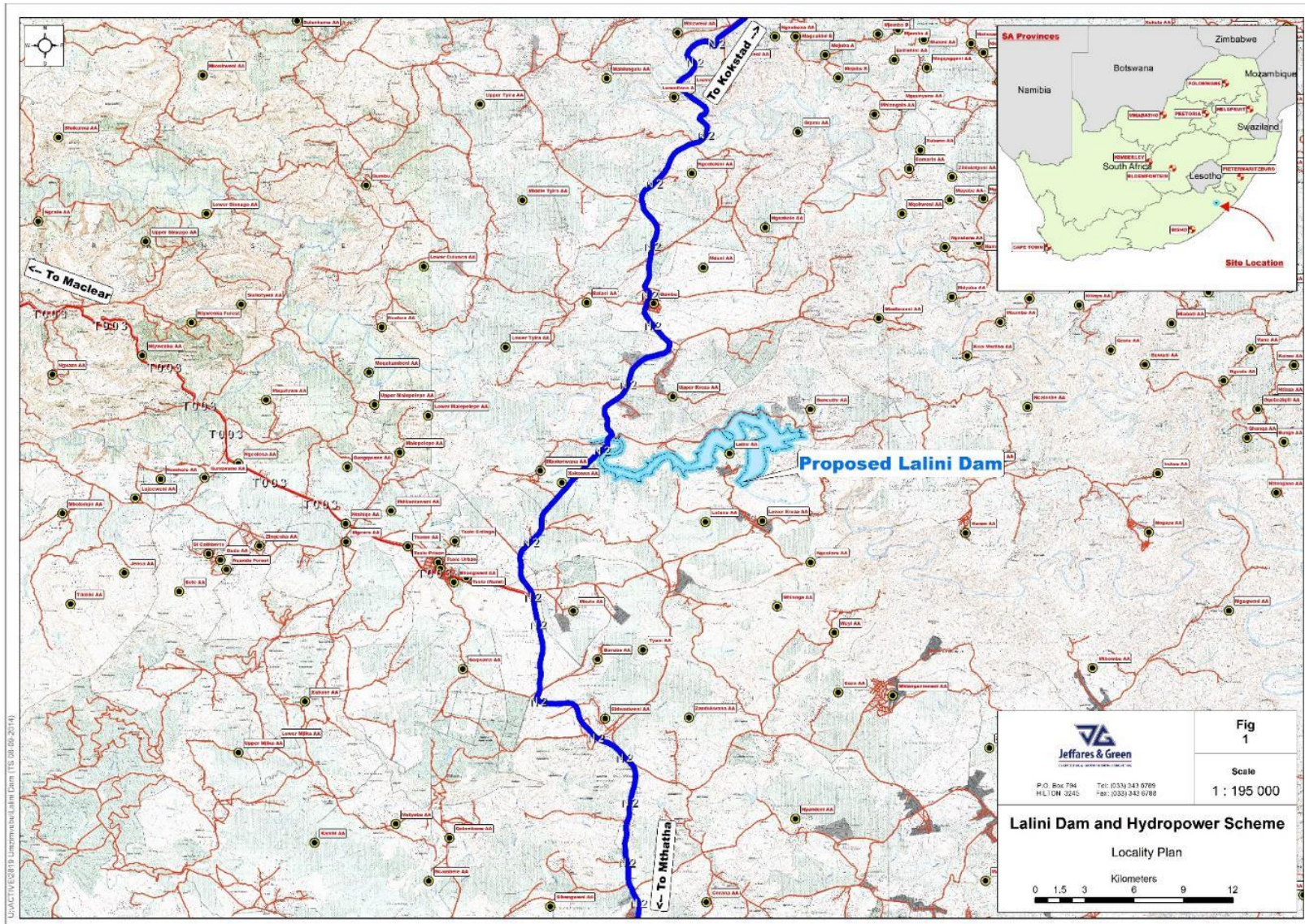


Fig A-1: Locality Plan

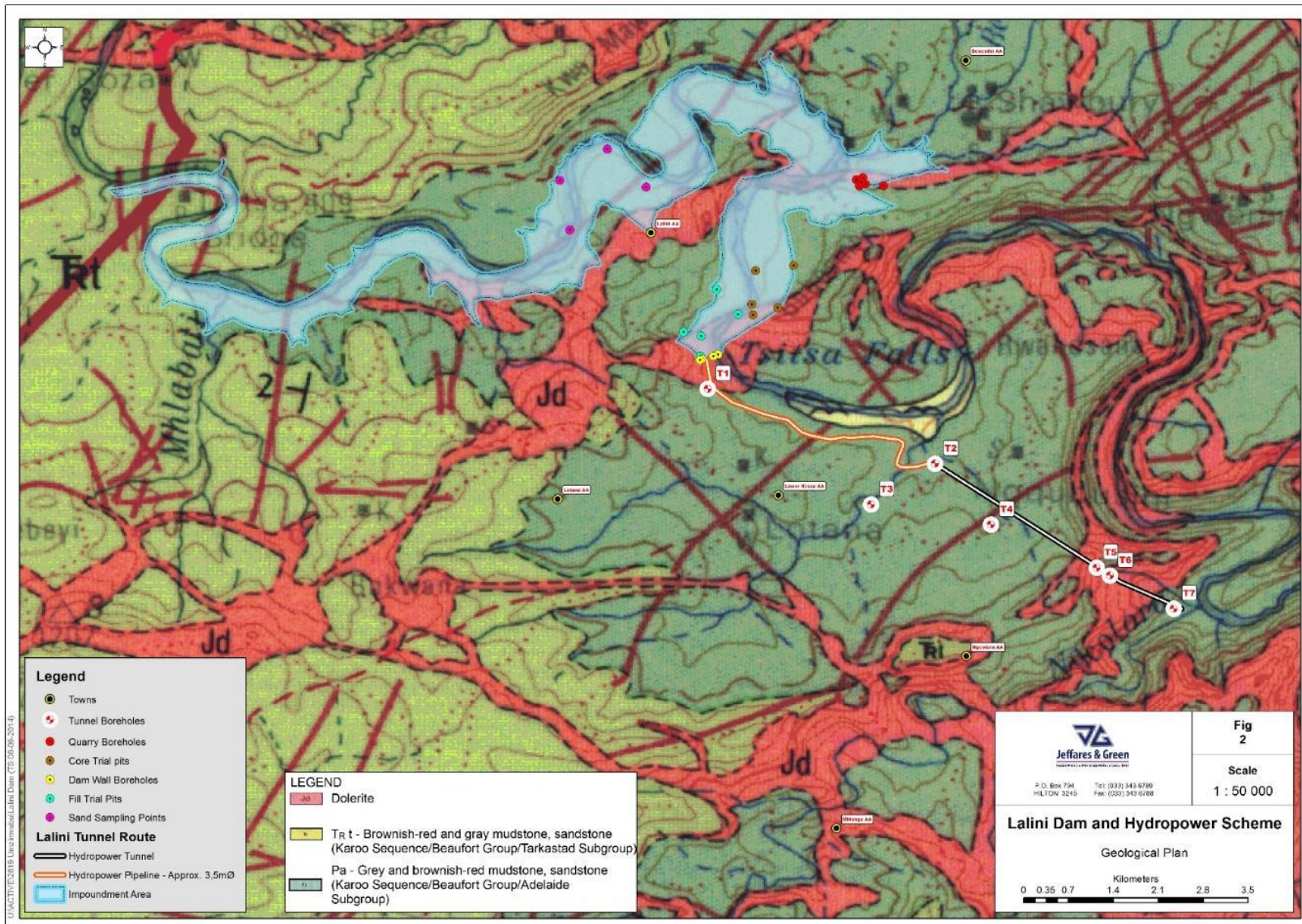


Fig A-2: Geological Plan

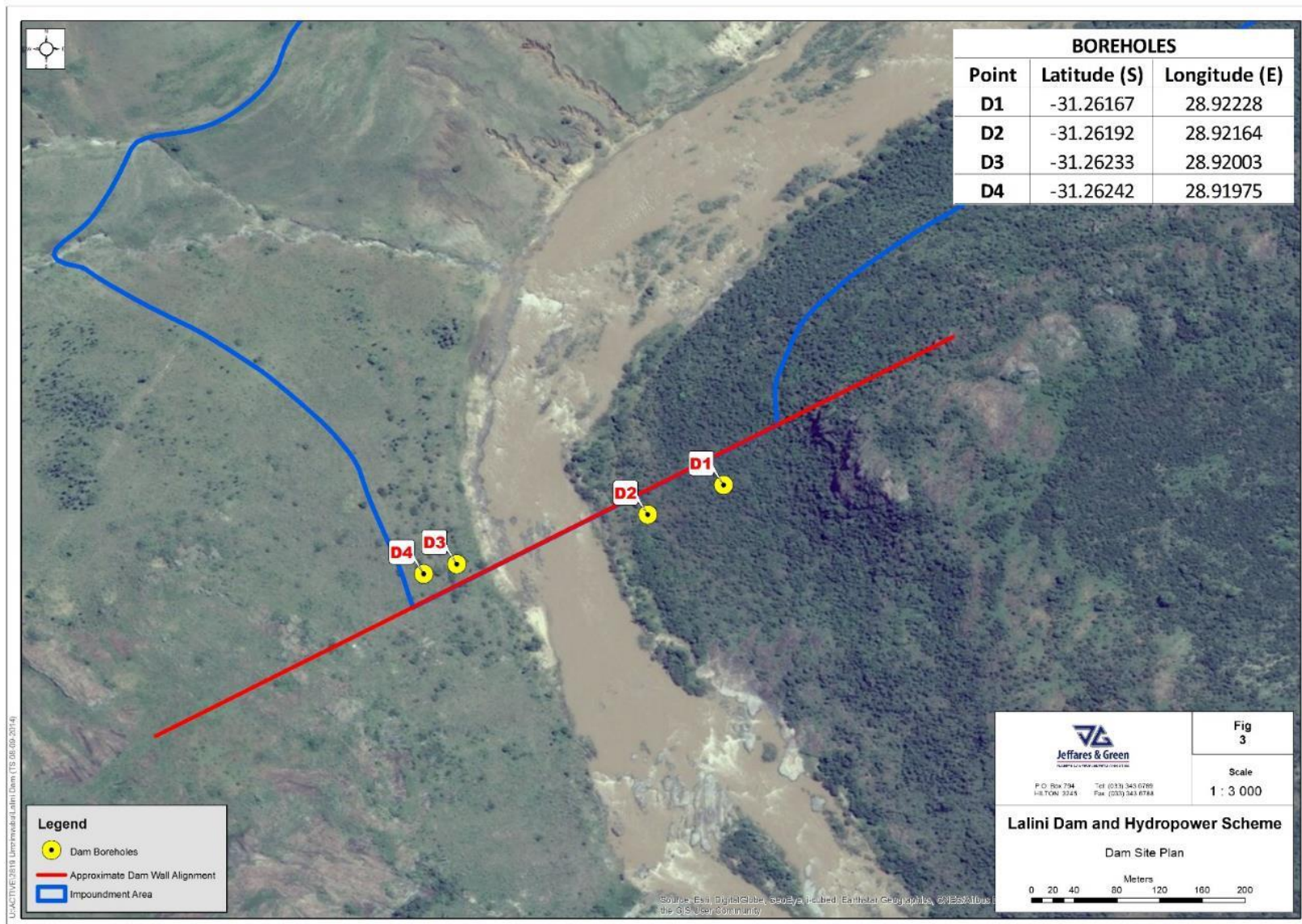


Fig A-3: Dam Site Plan

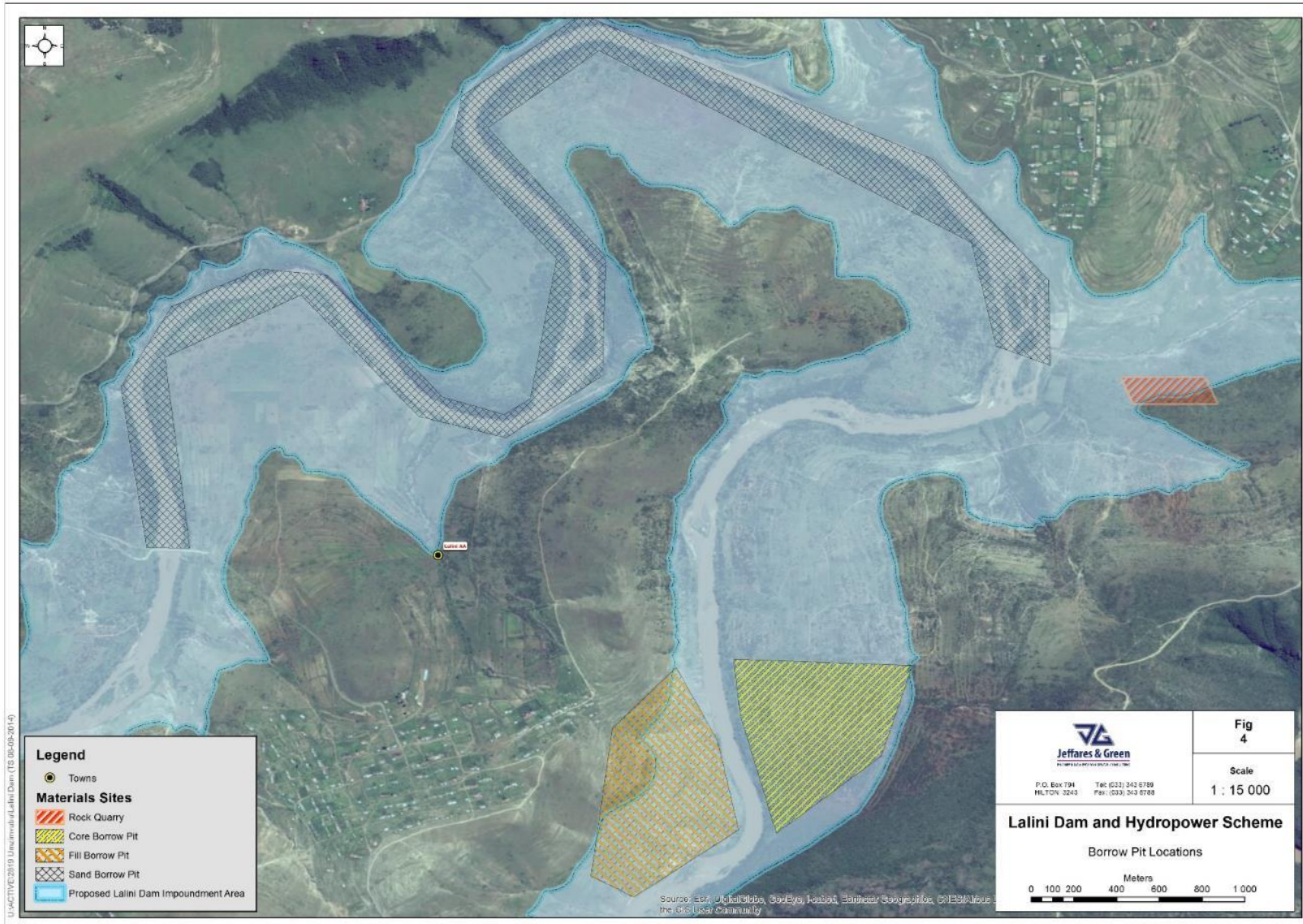


Fig A-4: Borrow Pit Locations

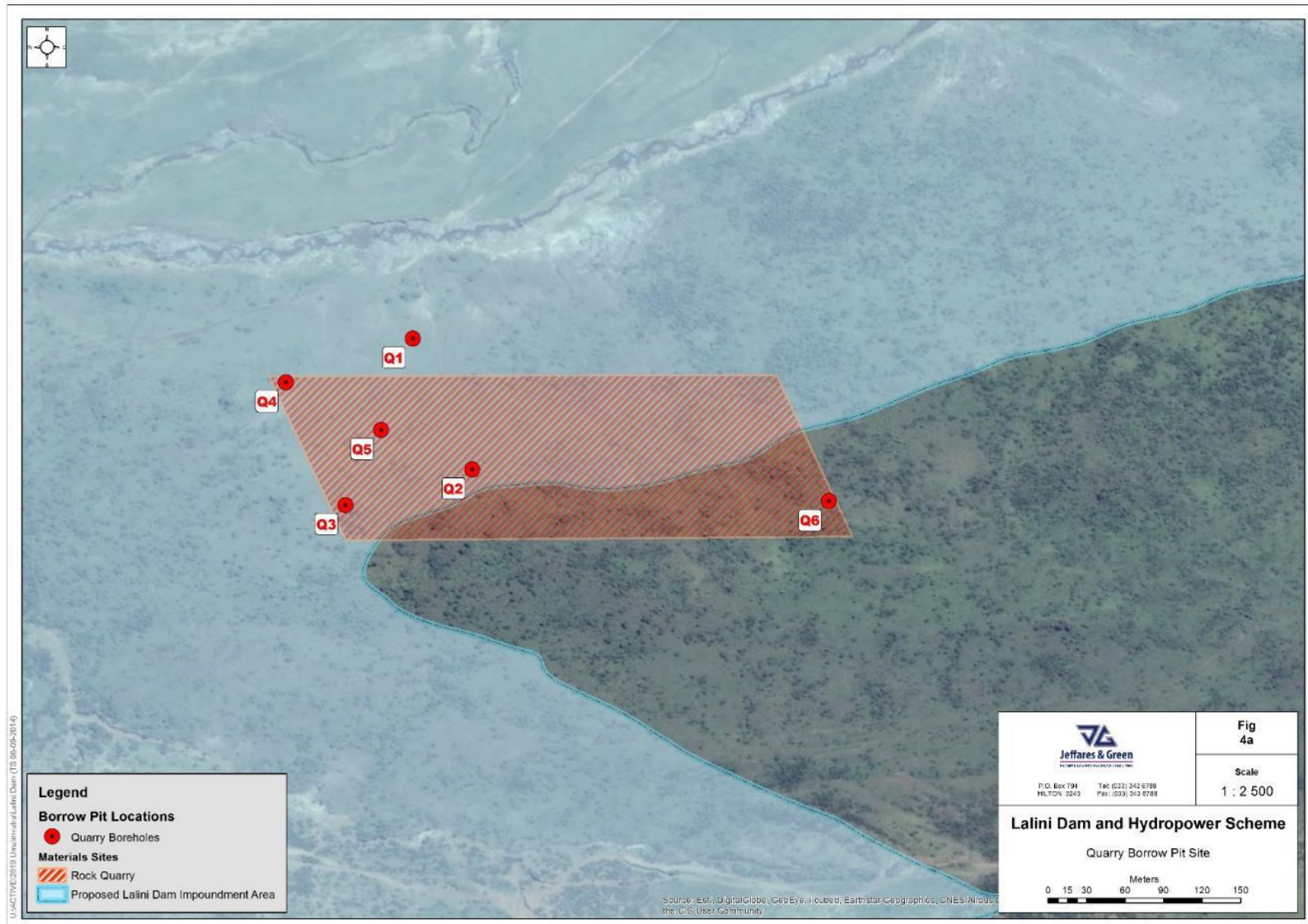


Fig A-4a: Quarry Borrow Pit Site

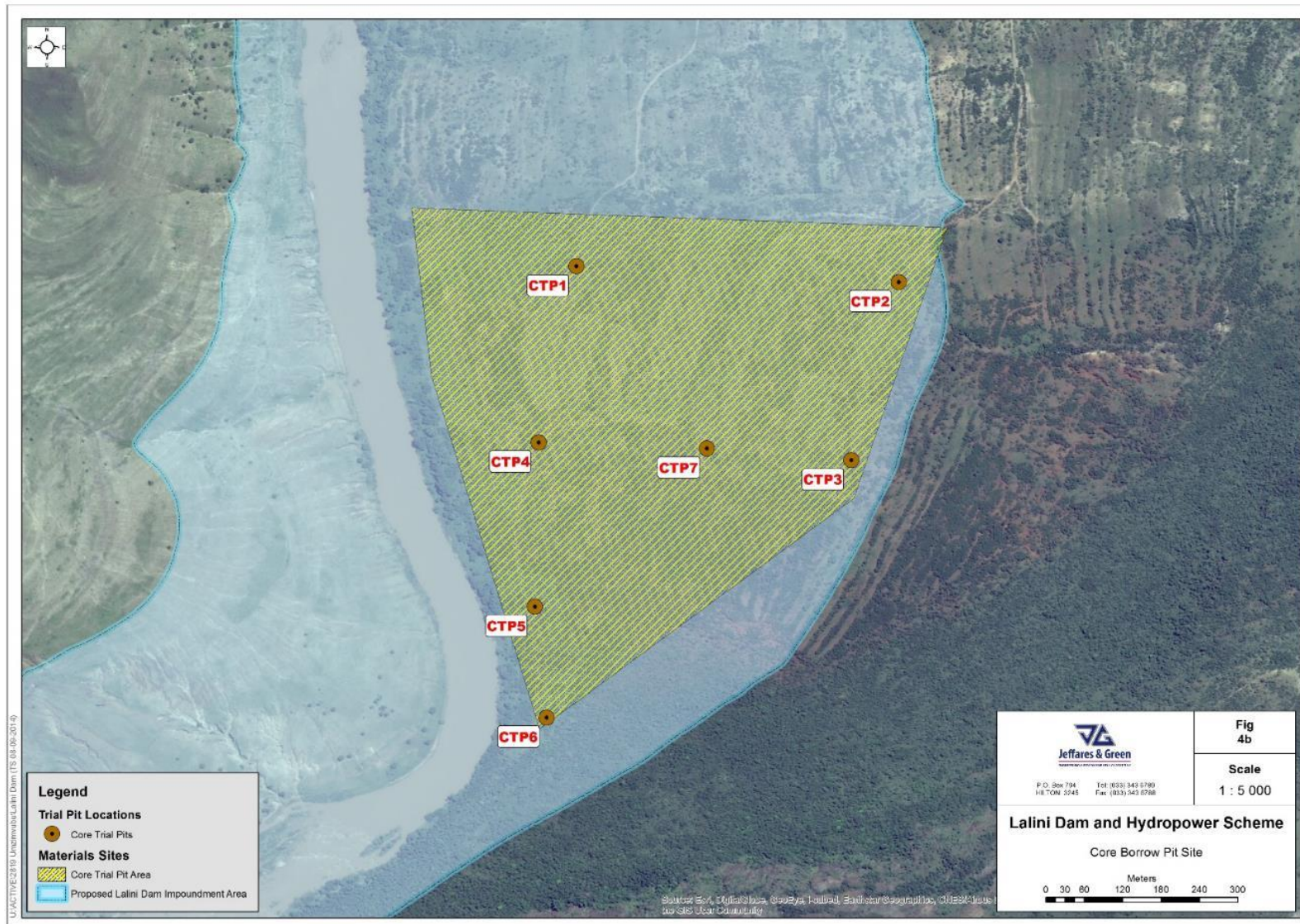


Fig A-4b: Core Borrow Pit Site

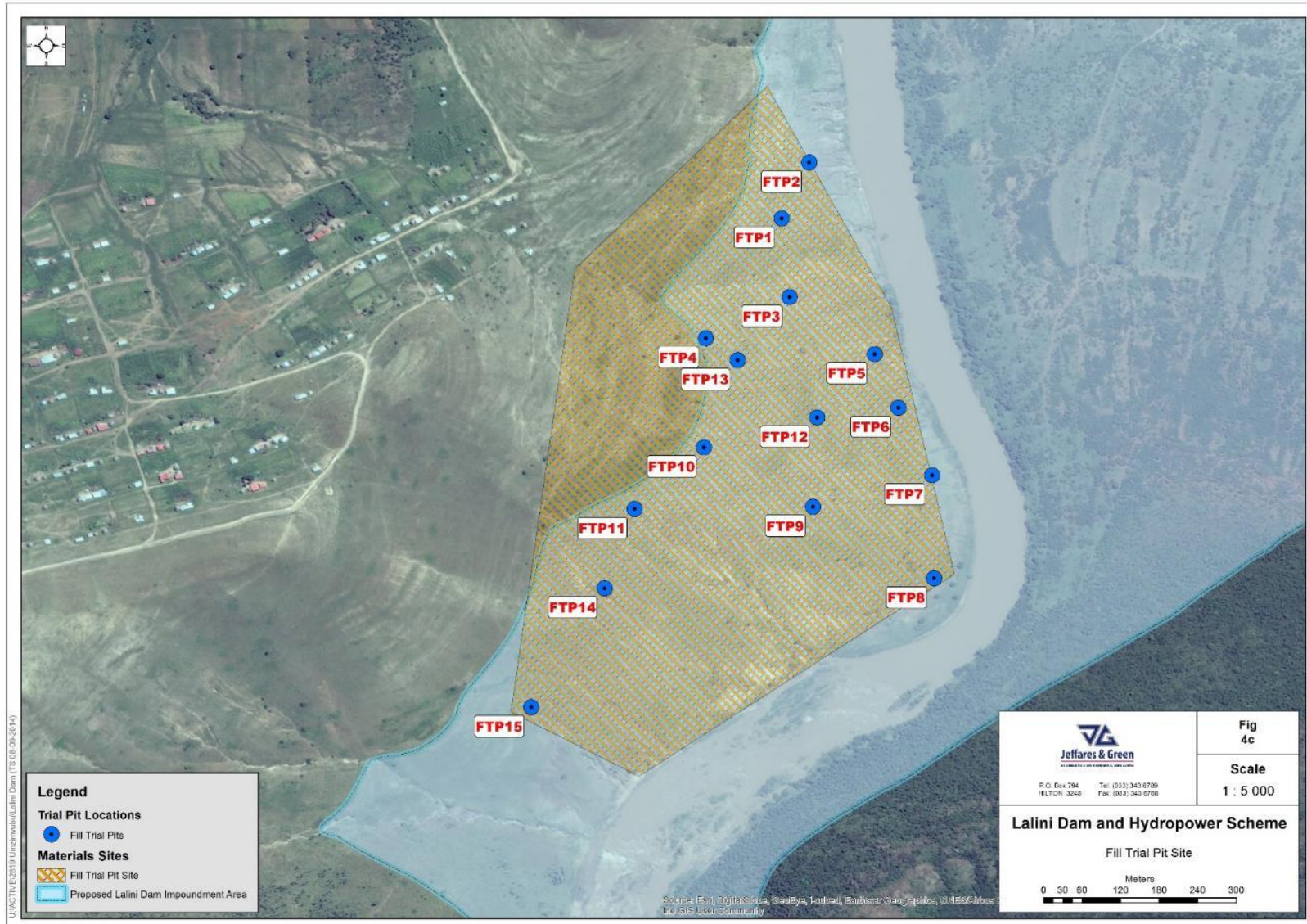


Fig A-4c: Fit Trial Pit Site

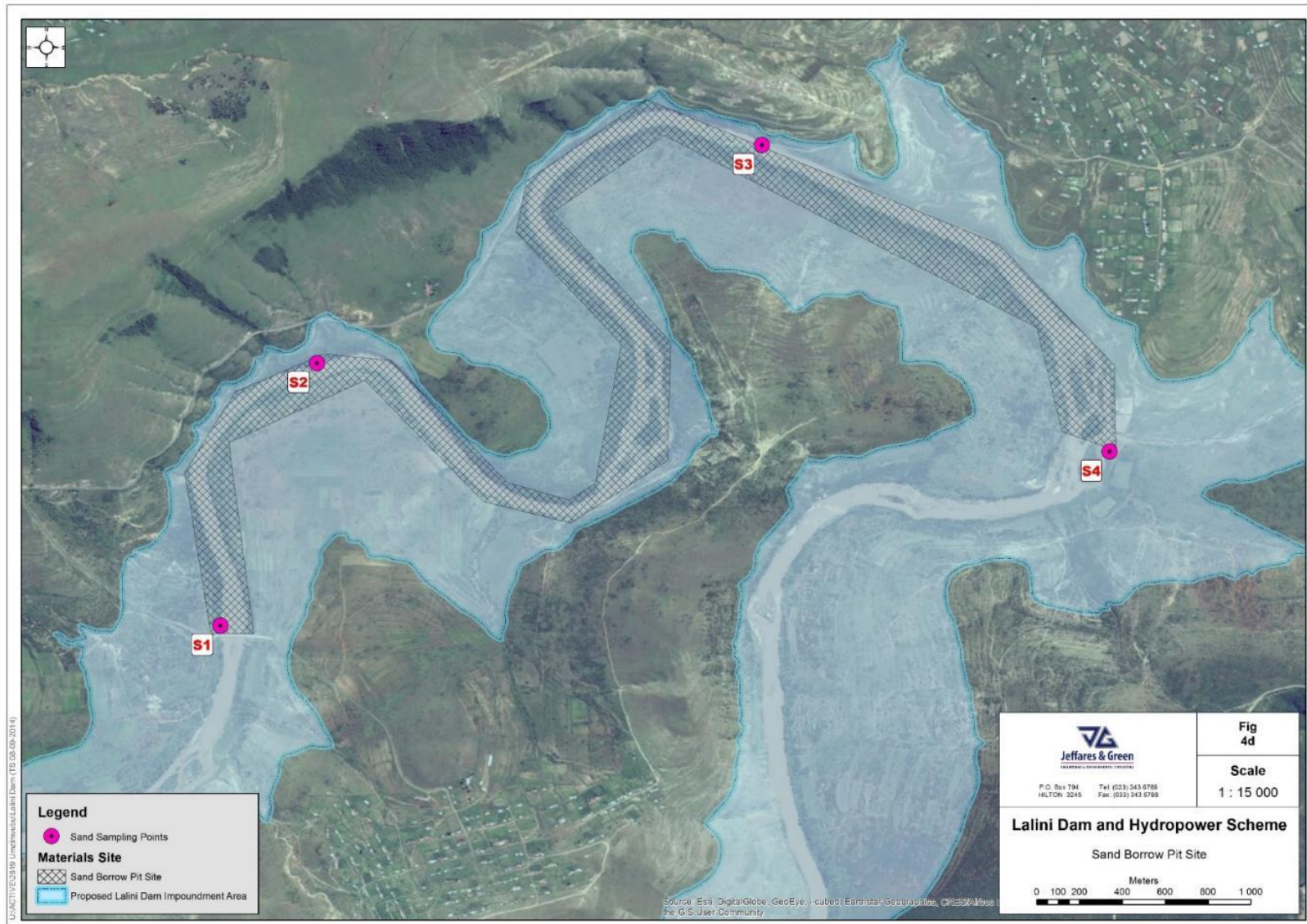


Fig A-4d: Sand Borrow Pit Site



Fig A-5: Pipeline Test Pits

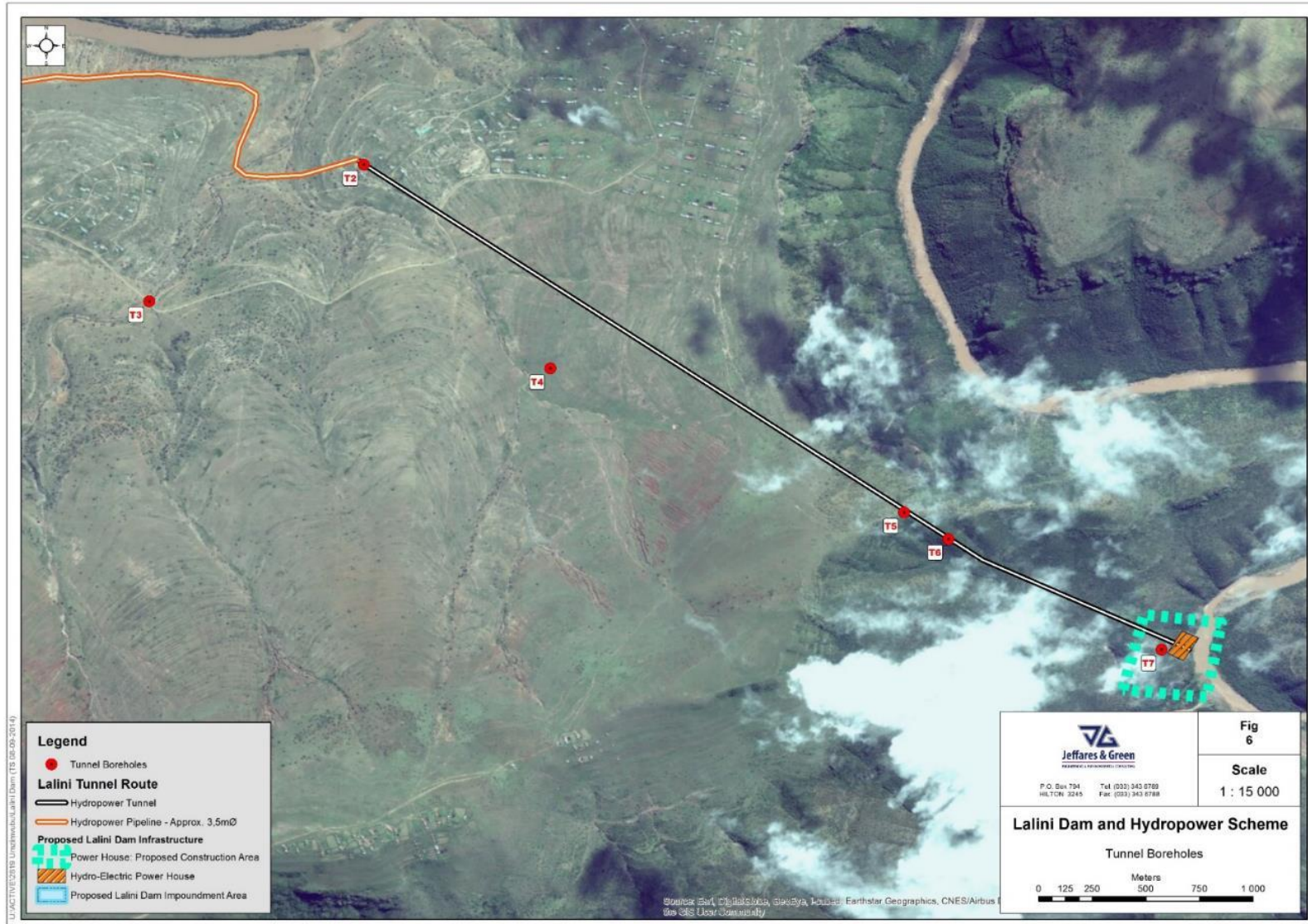


Fig A-6: Tunnel Boreholes

APPENDIX B

DAM BOREHOLE LOGS, PHOTOPGRAPHS AND WATER PRESSURE TESTS

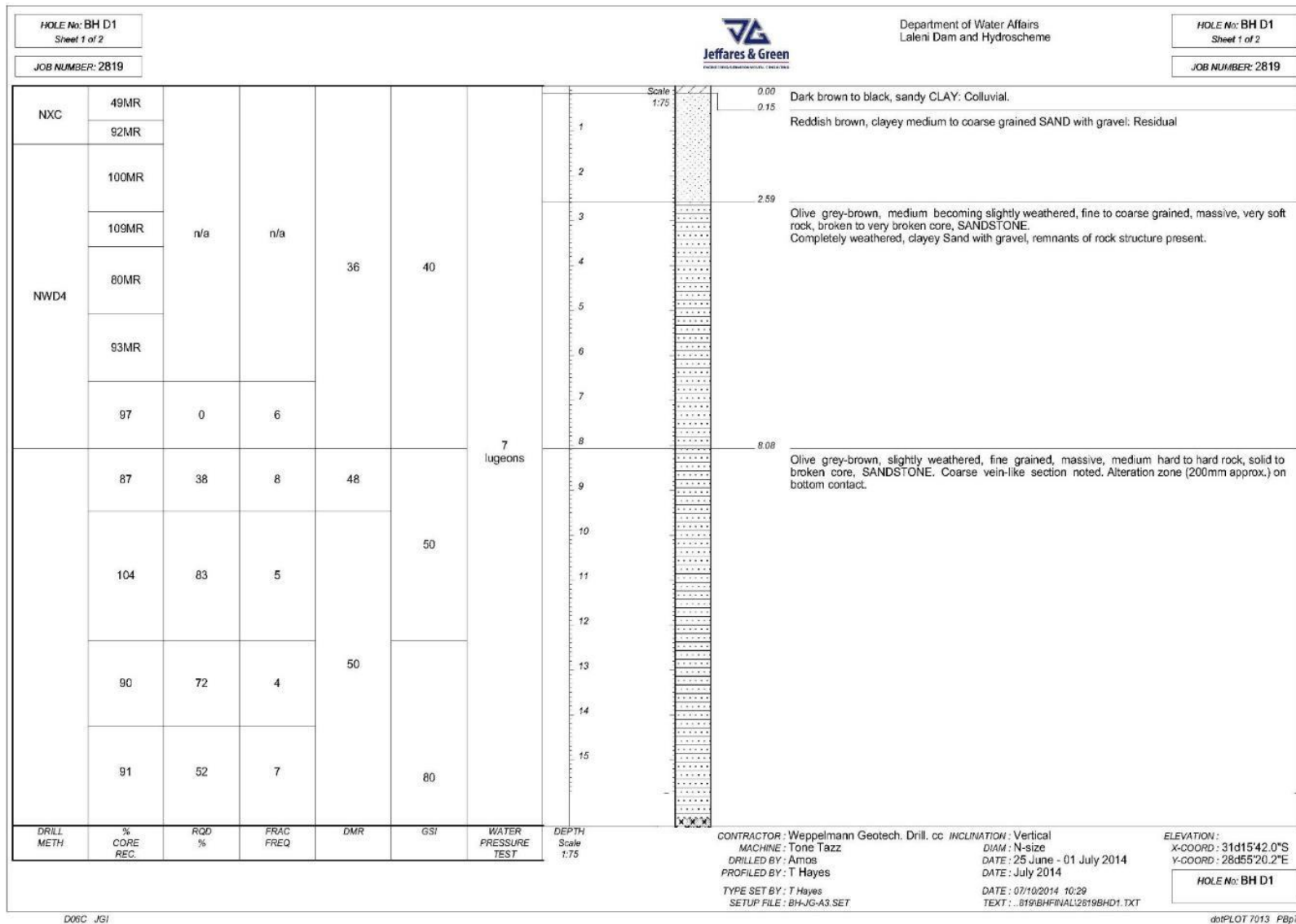


Fig B-1.1: Borehole D1 - Log

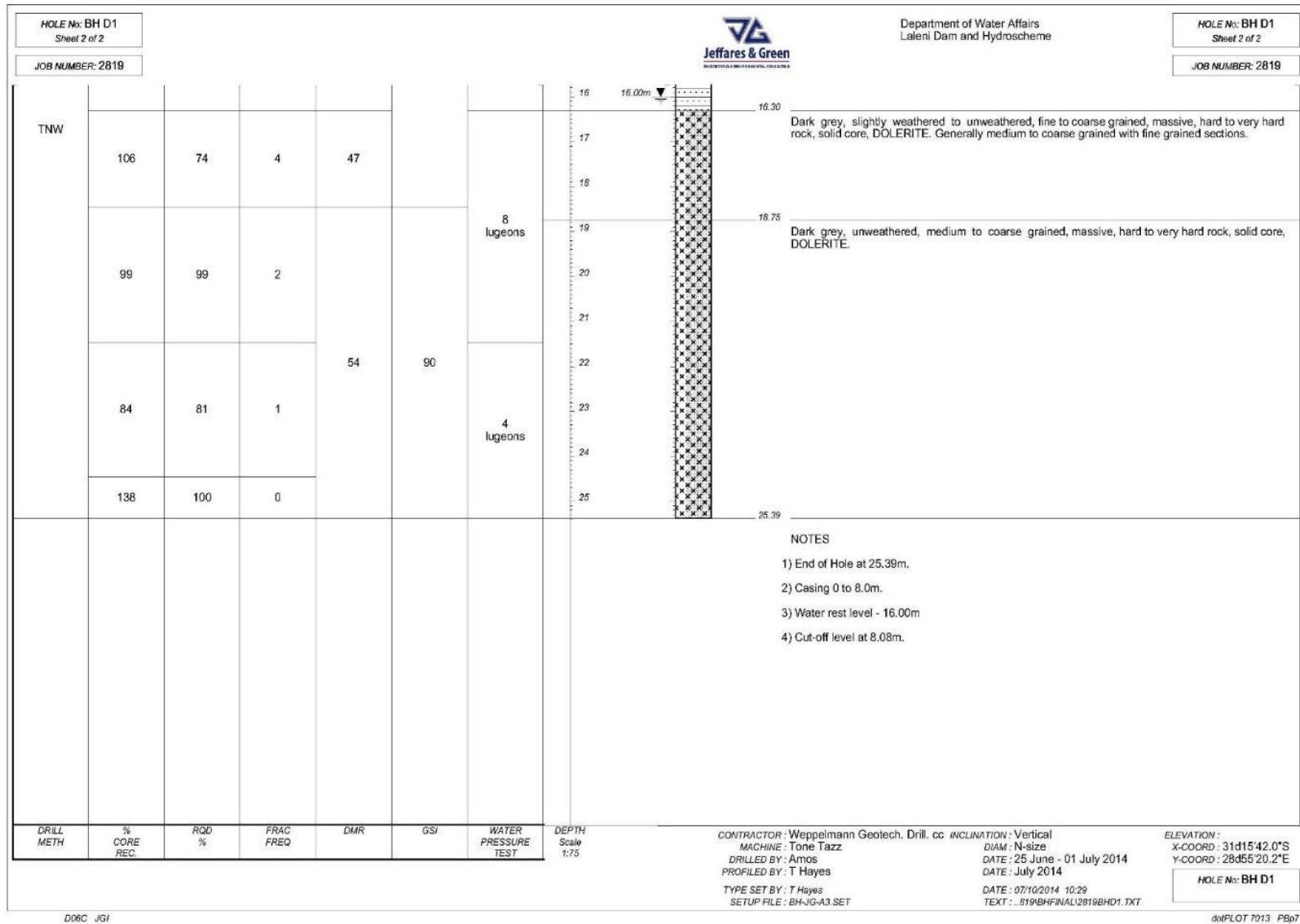


Fig B-1.2: Borehole D1 - Log

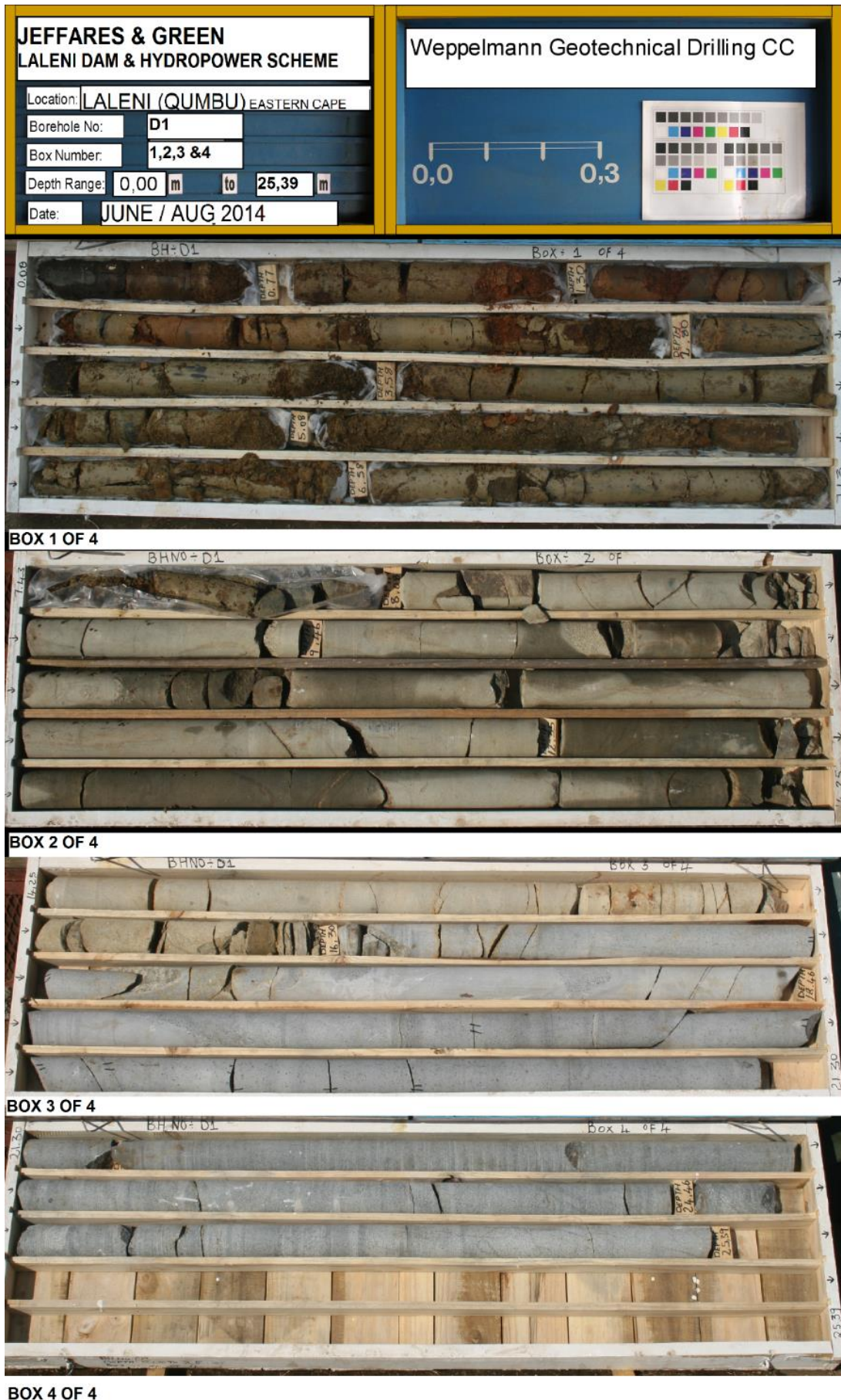


Fig B-2: Borehole D1 – Box 1 to 4 Dry

Guage Pressure (Bars) A	Pressure due to Guage Height (Bars) B	Pressure due to water height above water table (Bars) C	Test Pressure (Bars) A+B+C	Duration (Seconds)	Injected Volume (l)	Top Level	Bottom Level	Length of hole	time in minutes	litres / metre /minute	Lugeon Units	
WPT 10 – 16.3m												
1.5	0	0	1.5	600	82	10	16.3	6.3	10	1.3016	9	
2.6	0	0	2.6	600	120	10	16.3	6.3	10	1.9048	7	
3.75	0	0	3.75	600	122.9	10	16.3	6.3	10	1.9508	5	
2.6	0	0	2.6	600	125.2	10	16.3	6.3	10	1.9873	8	
1.5	0	0	1.5	600	70	10	16.3	6.3	10	1.1111	7	
WPT 16.3- 21.46m												
1.95	0	0	1.95	600	83.5	16.3	21.46	5.16	10	1.6182	8	
3.4	0	0	3.4	600	96.4	16.3	21.46	5.16	10	1.8682	5	
4.85	0	0	4.85	600	183.7	16.3	21.46	5.16	10	3.5601	7	
3.4	0	0	3.4	600	154.8	16.3	21.46	5.16	10	3	9	
1.95	0	0	1.95	600	90.5	16.3	21.46	5.16	10	1.7539	9	
WPT 21.46 – 25.39m												
2.25	0	0	2.25	600	64.6	21.46	25.39	3.93	10	1.6438	7	
3.95	0	0	3.95	600	76.3	21.46	25.39	3.93	10	1.9415	5	
5.65	0	0	5.65	600	98.2	21.46	25.39	3.93	10	2.4987	4	
3.95	0	0	3.95	600	73.6	21.46	25.39	3.93	10	1.8728	5	
2.25	0	0	2.25	600	52.4	21.46	25.39	3.93	10	1.3333	6	
Mzimvubu			Input Data	Depth from top of embankment to water table (m)					0			
Borehole No	D1			Height of guage above top of embankment (m)					0			

Table B-1: Water Pressure Tests – Borehole D1

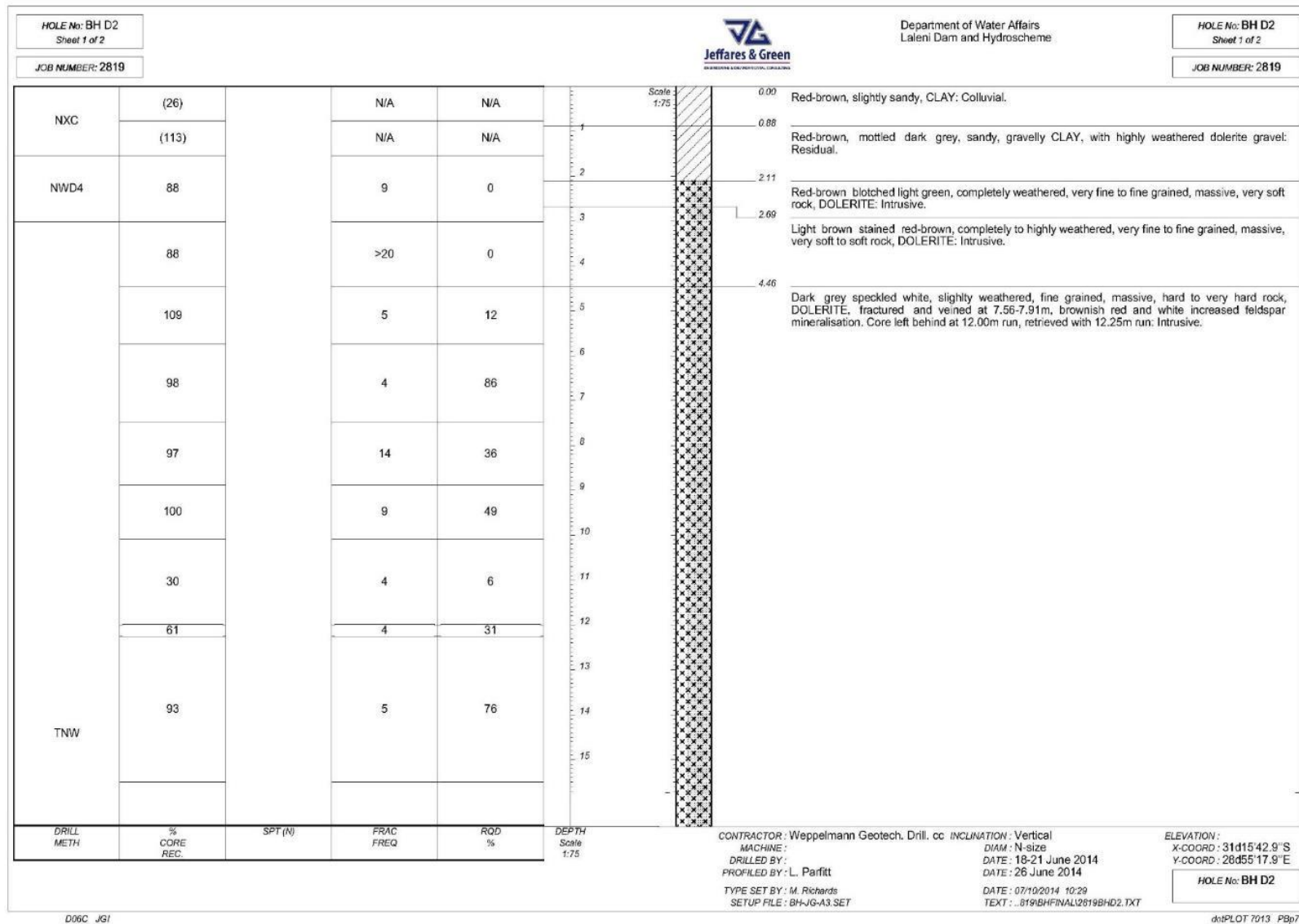


Fig B-3.1: Borehole D2 - Log

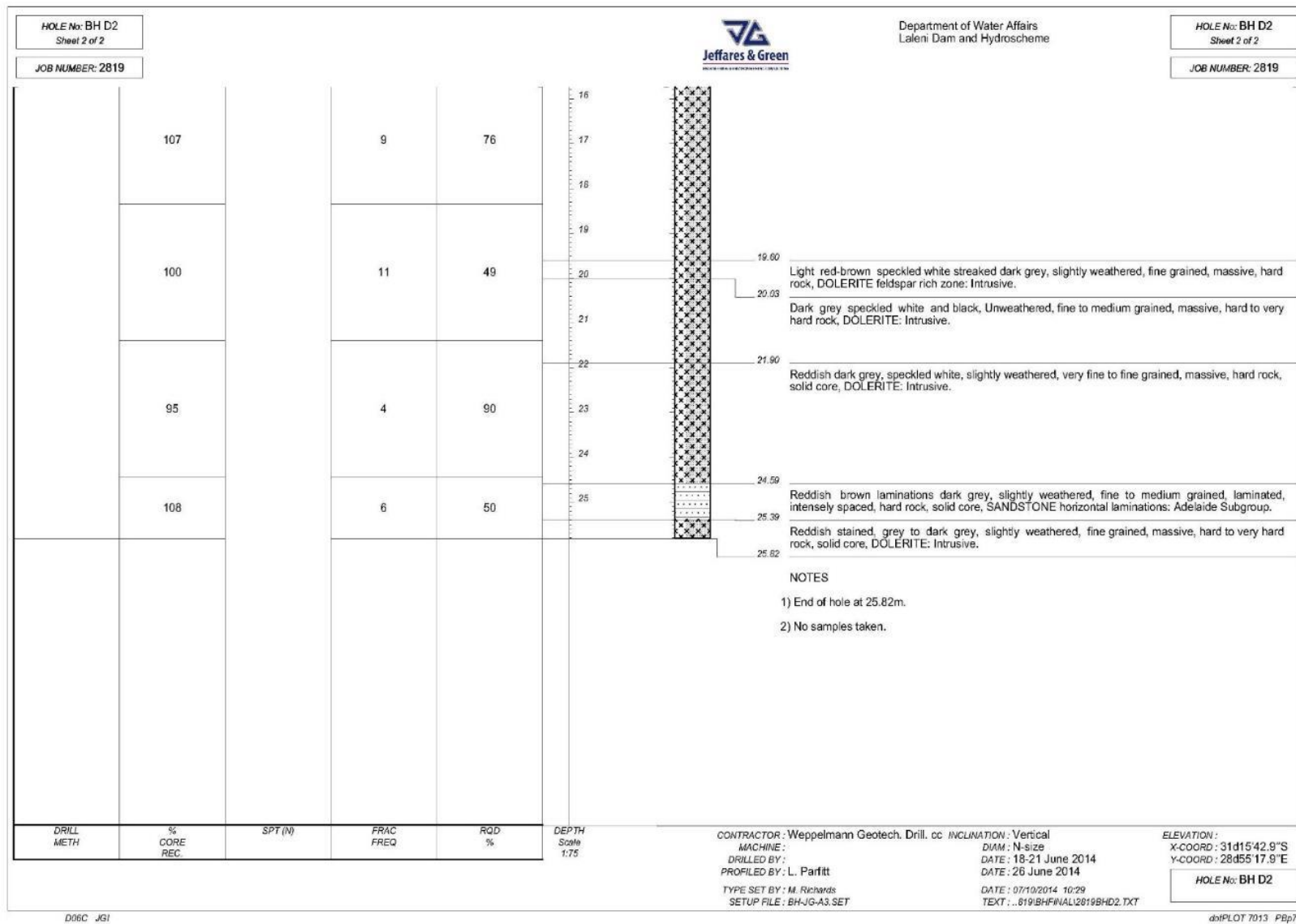


Fig B-3.2: Borehole D2 - Log

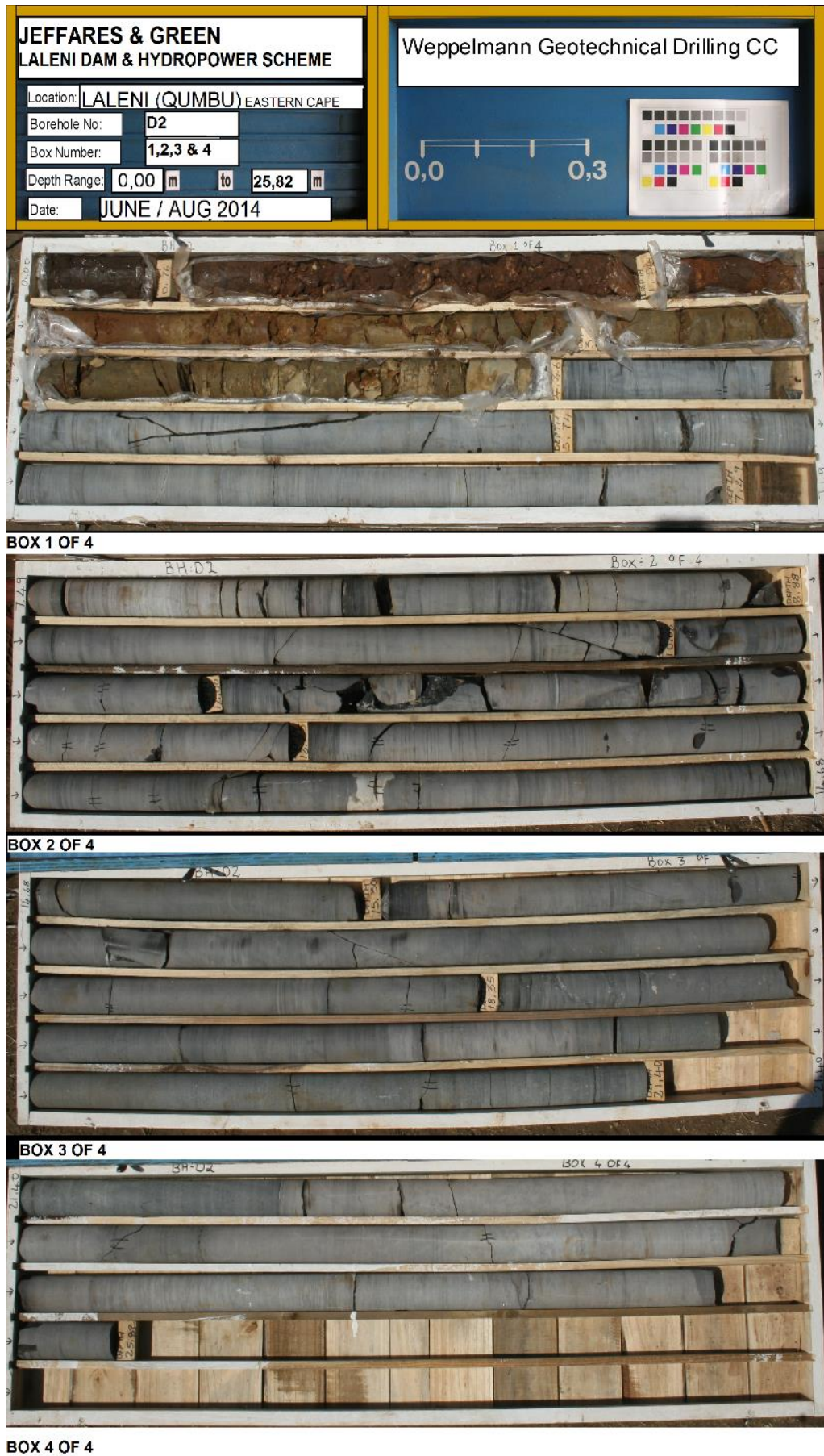


Fig B-4: Borehole D2 – Box 1 to 4 Dry

Guage Pressure (Bars) A	Pressure due to Guage Height (Bars) B	Pressure due to water height above water table (Bars) C	Test Pressure (Bars) A+B+C	Duration (Seconds)	Injected Volume (l)	Top Level	Bottom Level	Length of hole	time in minutes	litres / metre /minute	Lugeon Units	
WPT 7.0 – 12.0m												
1.1	0	0	1.1	600	6	7	12	5	10	0.12	1	
1.9	0	0	1.9	600	11	7	12	5	10	0.22	1	
2	0	0	2	600	25	7	12	5	10	0.5	3	
1.9	0	0	1.9	600	116.2	7	12	5	10	2.324	12	
1.1	0	0	1.1	600	124.8	7	12	5	10	2.496	23	
WPT 12.0- 18.35m												
1.65	0	0	1.65	600	9.1	12	18.35	6.35	10	0.1433	1	
2.85	0	0	2.85	600	48.2	12	18.35	6.35	10	0.7591	3	
4.05	0	0	4.05	600	42.4	12	18.35	6.35	10	0.6677	2	
2.85	0	0	2.85	600	35.1	12	18.35	6.35	10	0.5528	2	
1.65	0	0	1.65	600	25.6	12	18.35	6.35	10	0.4031	2	
WPT 18.5 – 25.82m												
1.4	0	0	1.4	600	114.8	18.5	25.82	7.32	10	1.5683	11	
	0	0	0	600		18.5	25.82	7.32	10	0	0	
	0	0	0	600		18.5	25.82	7.32	10	0	0	
	0	0	0	600		18.5	25.82	7.32	10	0	0	
	0	0	0	600		18.5	25.82	7.32	10	0	0	
Mzimvubu Borehole No	D2		Input Data	Depth from top of embankment to water table (m)					0			
				Height of guage above top of embankment (m)					0			

Table B-2: Water Pressure Tests – Borehole D2

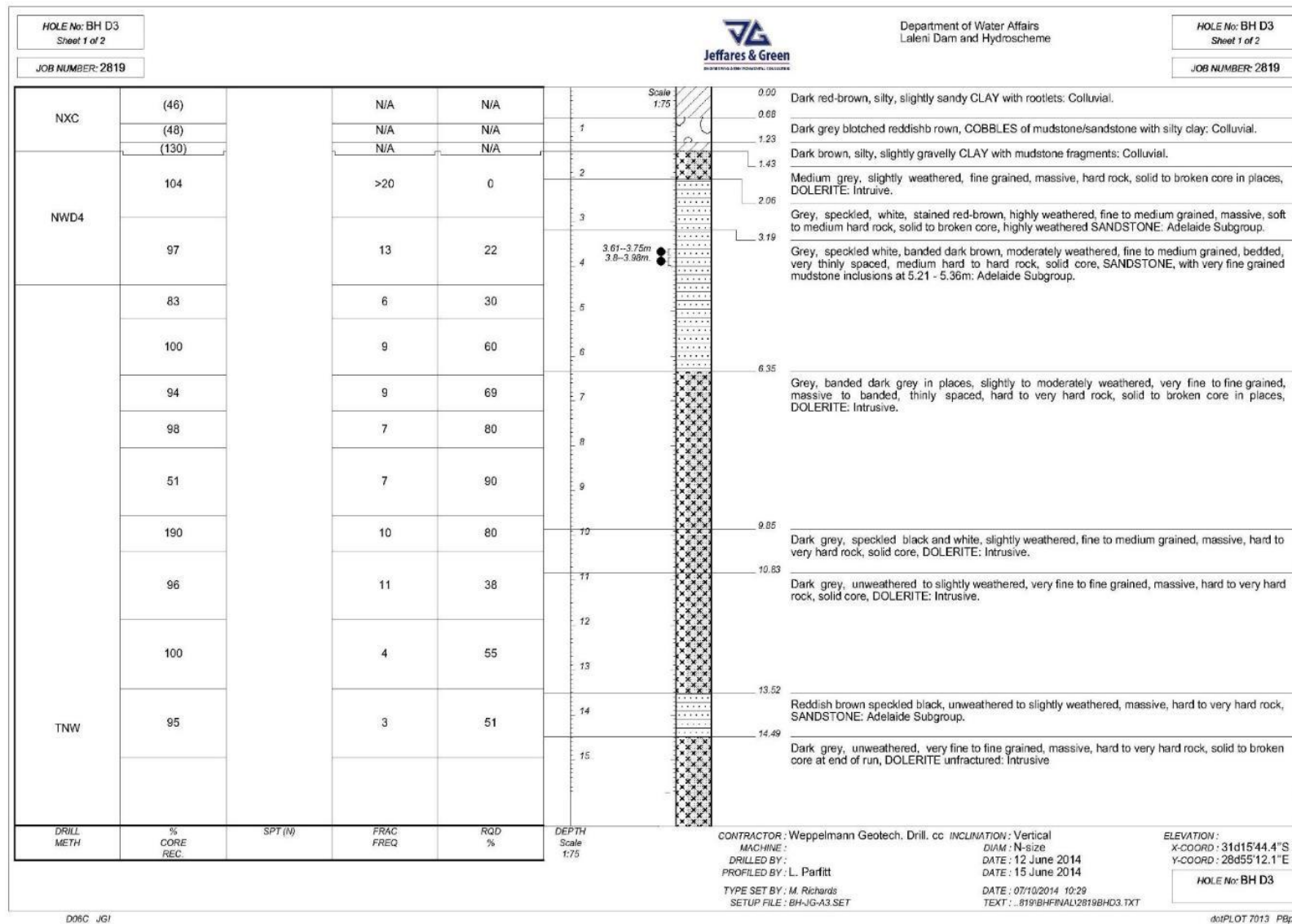


Fig B-5.1: Borehole D3 - Log

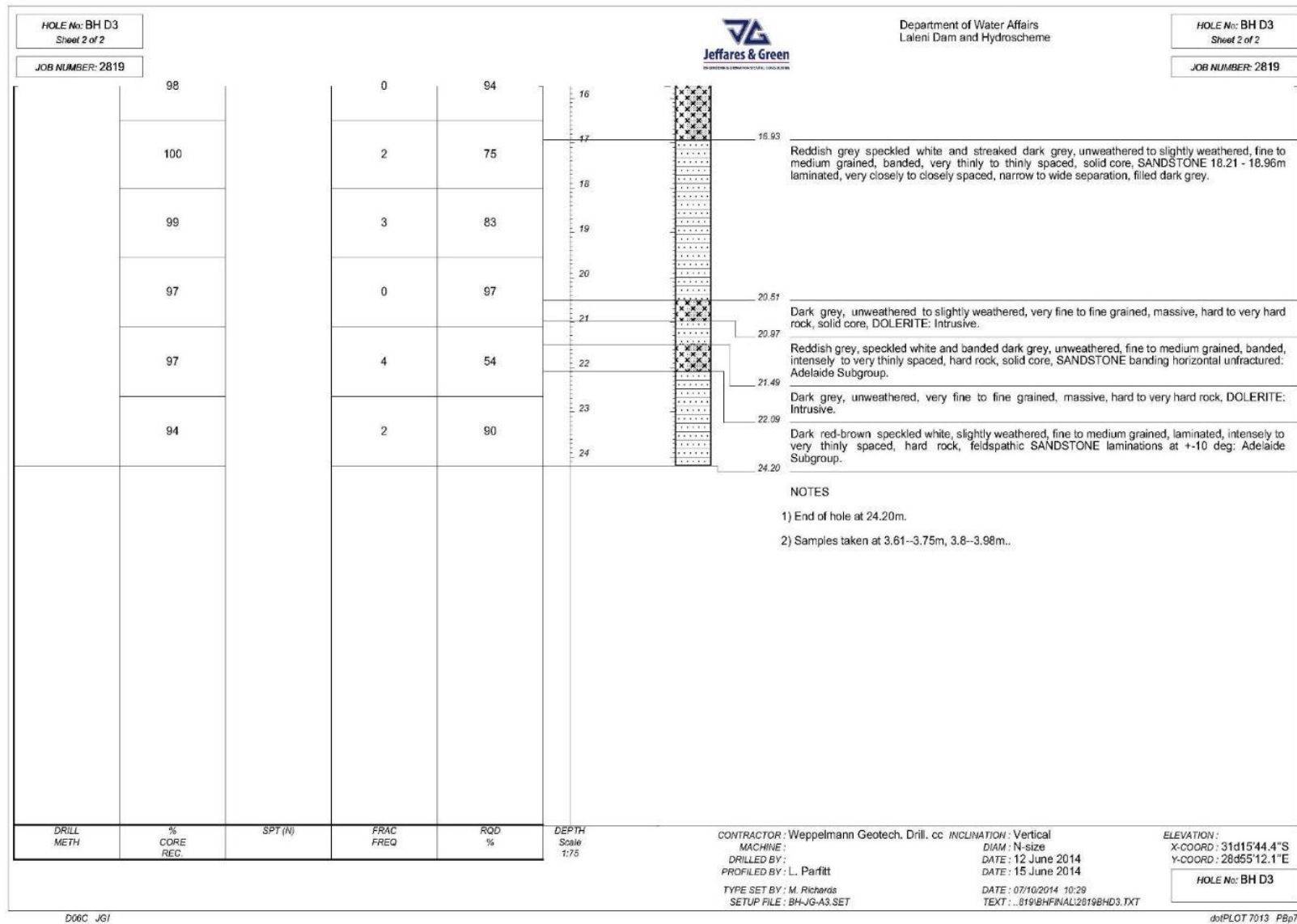


Fig B-5.2: Borehole D3 - Log



Fig B-6: Borehole D3 – Box 1 to 4 Dry

Guage Pressure (Bars) A	Pressure due to Guage Height (Bars) B	Pressure due to water height above water table (Bars) C	Test Pressure (Bars) A+B+C	Duration (Seconds)	Injected Volume (l)	Top Level	Bottom Level	Length of hole	time in minutes	litres / metre /minute	Lugeon Units	
WPT 3.5 – 7.22m												
0.3	0	0	0.3	600	62	3.5	7.22	3.72	10	1.6667	56	
0.55	0	0	0.55	600	49	3.5	7.22	3.72	10	1.3172	24	
0.8	0	0	0.8	600	51.5	3.5	7.22	3.72	10	1.3844	17	
0.55	0	0	0.55	600	43.4	3.5	7.22	3.72	10	1.1667	21	
0.3	0	0	0.3	600	34	3.5	7.22	3.72	10	0.914	30	
WPT 7.5 – 13.5m												
1.2	0	0	1.2	600	90.9	7.5	13.5	6	10	1.515	13	
2.15	0	0	2.15	600	92.4	7.5	13.5	6	10	1.54	7	
3.05	0	0	3.05	600	83.2	7.5	13.5	6	10	1.3867	5	
2.15	0	0	2.15	600	58.2	7.5	13.5	6	10	0.97	5	
1.2	0	0	1.2	600	9.4	7.5	13.5	6	10	0.1567	1	
WPT 13.5 – 24.2m												
2.15	0	0	2.15	600	0	13.5	24.2	10.7	10	0	0	
3.8	0	0	3.8	600	88.1	13.5	24.2	10.7	10	0.8234	2	
5.45	0	0	5.45	600	53.3	13.5	24.2	10.7	10	0.4981	1	
3.8	0	0	3.8	600	86.7	13.5	24.2	10.7	10	0.8103	2	
2.15	0	0	2.15	600	11.5	13.5	24.2	10.7	10	0.1075	0	
Mzimvubu			Input Data	Depth from top of embankment to water table (m)					0			
Borehole No	D3			Height of guage above top of embankment (m)					0			

Table B-3: Water Pressure Tests – Borehole D3

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

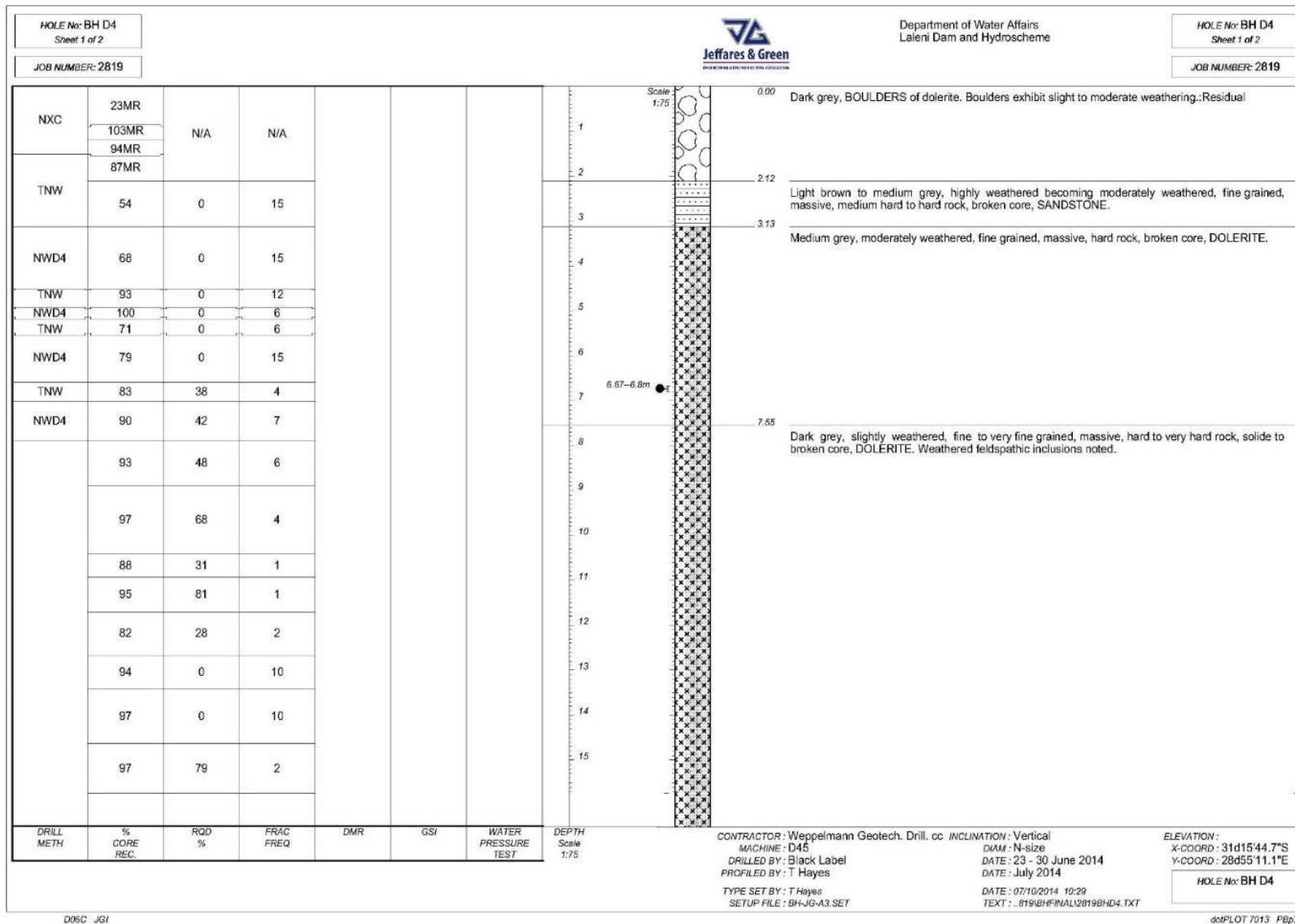


Fig B-7.1: Borehole D4 - Log

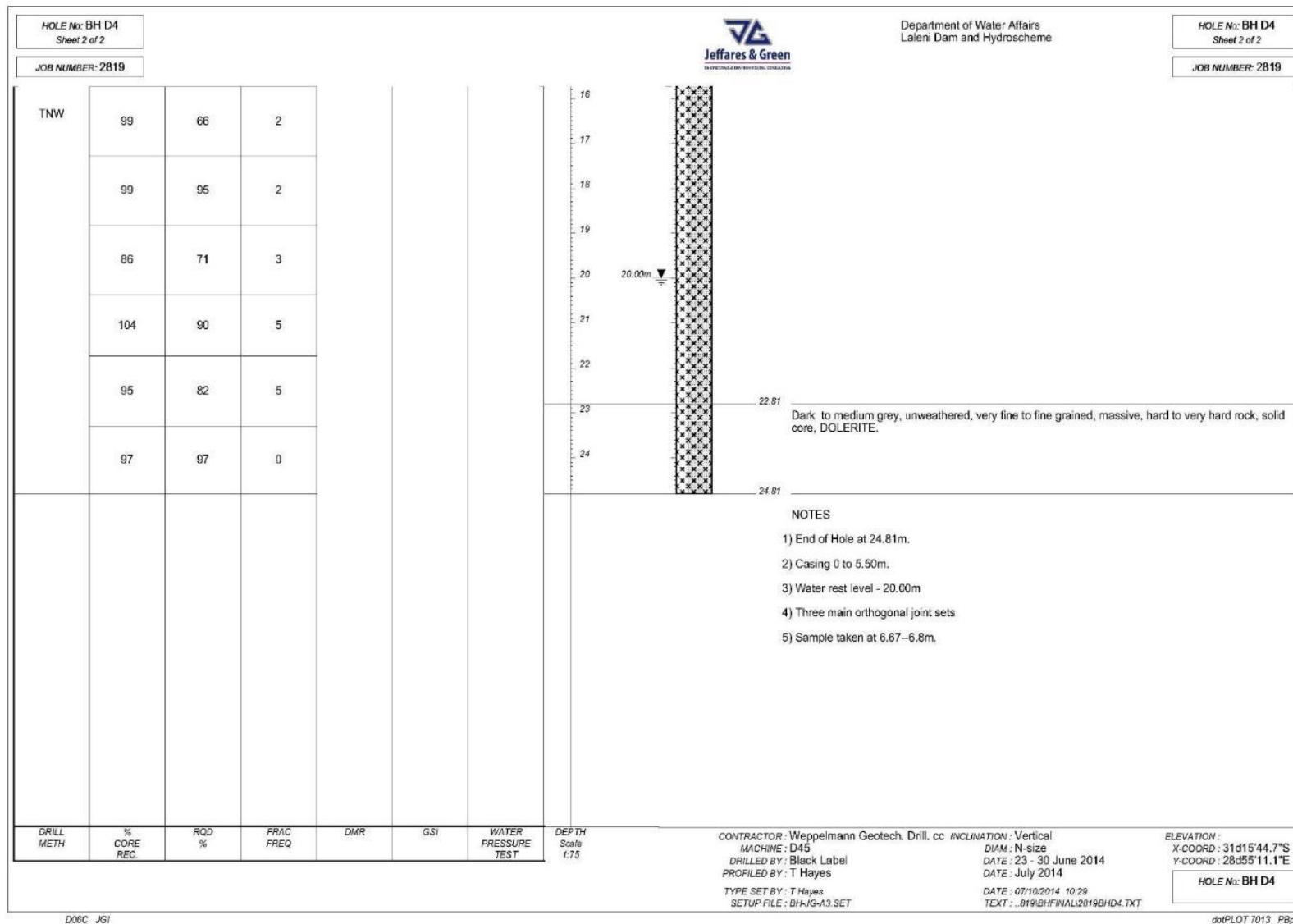


Fig B-7.2: Borehole D4 - Log



Fig B-8: Borehole D4 – Box 1 to 4 Dry

Guage Pressure (Bars) A	Pressure due to Guage Height (Bars) B	Pressure due to water height above water table (Bars) C	Test Pressure (Bars) A+B+C	Duration (Seconds)	Injected Volume (l)	Top Level	Bottom Level	Length of hole	time in minutes	litres / metre /minute	Lugeon Units	
WPT 15.0 – 20.5m												
1.35	0	0	1.35	600	148.4	15	20.5	5.5	10	2.6982	20	
2.4	0	0	2.4	600	169	15	20.5	5.5	10	3.0727	13	
2.4	0	0	2.4	600	92.8	15	20.5	5.5	10	1.6873	7	
2.4	0	0	2.4	600		15	20.5	5.5	10	0	0	
1.35	0	0	1.35	600	150.7	15	20.5	5.5	10	2.74	20	
WPT 20.5 – 24.81m												
1.5	0	0	1.5	600	194.5	20.5	24.81	4.31	10	4.5128	30	
	0	0	0	600		20.5	24.81	4.31	10	0	0	
	0	0	0	600		20.5	24.81	4.31	10	0	0	
	0	0	0	600		20.5	24.81	4.31	10	0	0	
	0	0	0	600		20.5	24.81	4.31	10	0	0	
Mzimvubu			Input Data	Depth from top of embankment to water table (m)					0			
Borehole No	D4			Height of guage above top of embankment (m)					0			

Table B-4: Water Pressure Tests – Borehole D4

APPENDIX C

QUARRY BOREHOLE LOGS AND PHOTOGRAPHS

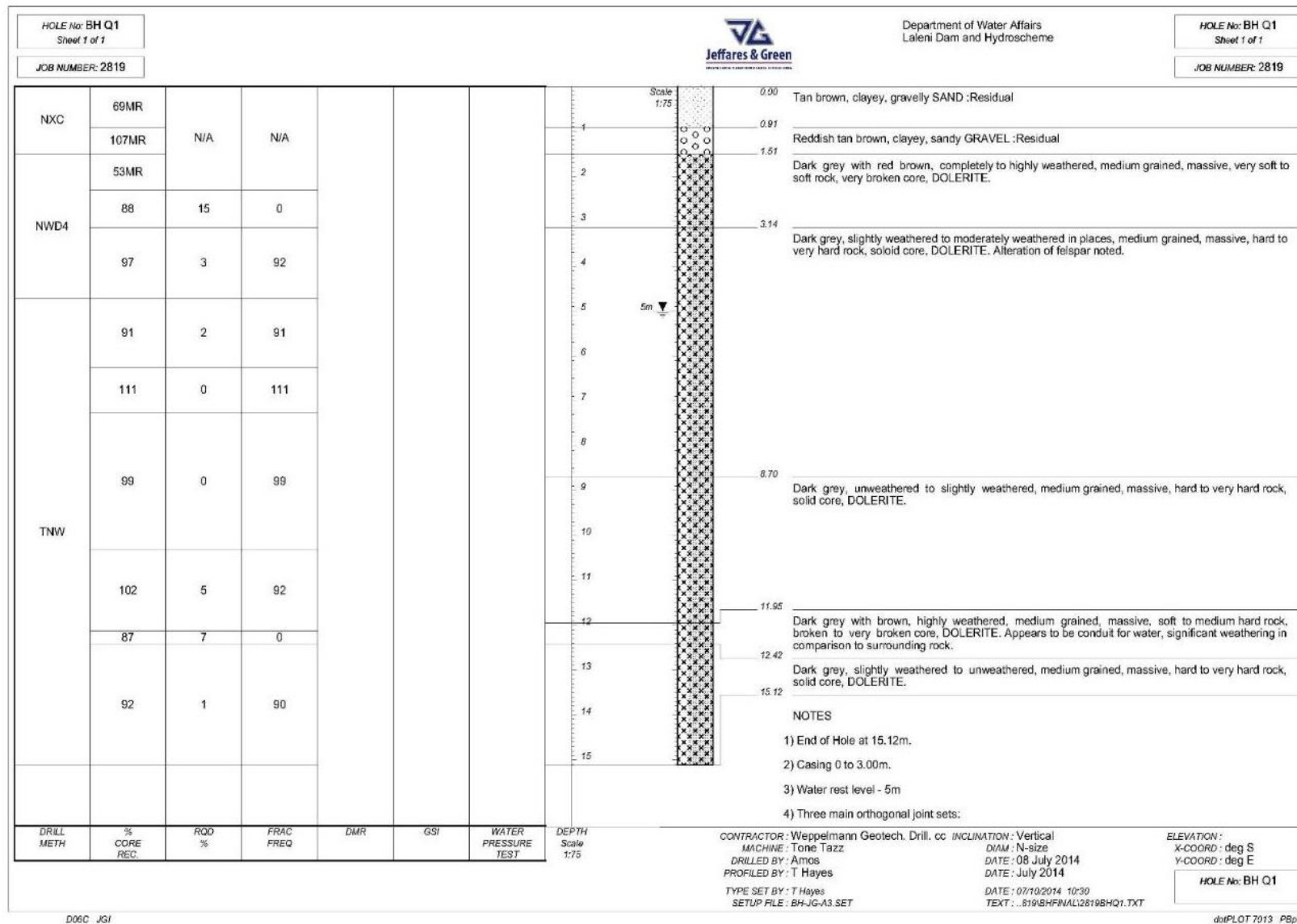


Fig C-1: Borehole Quarry 1 - Log

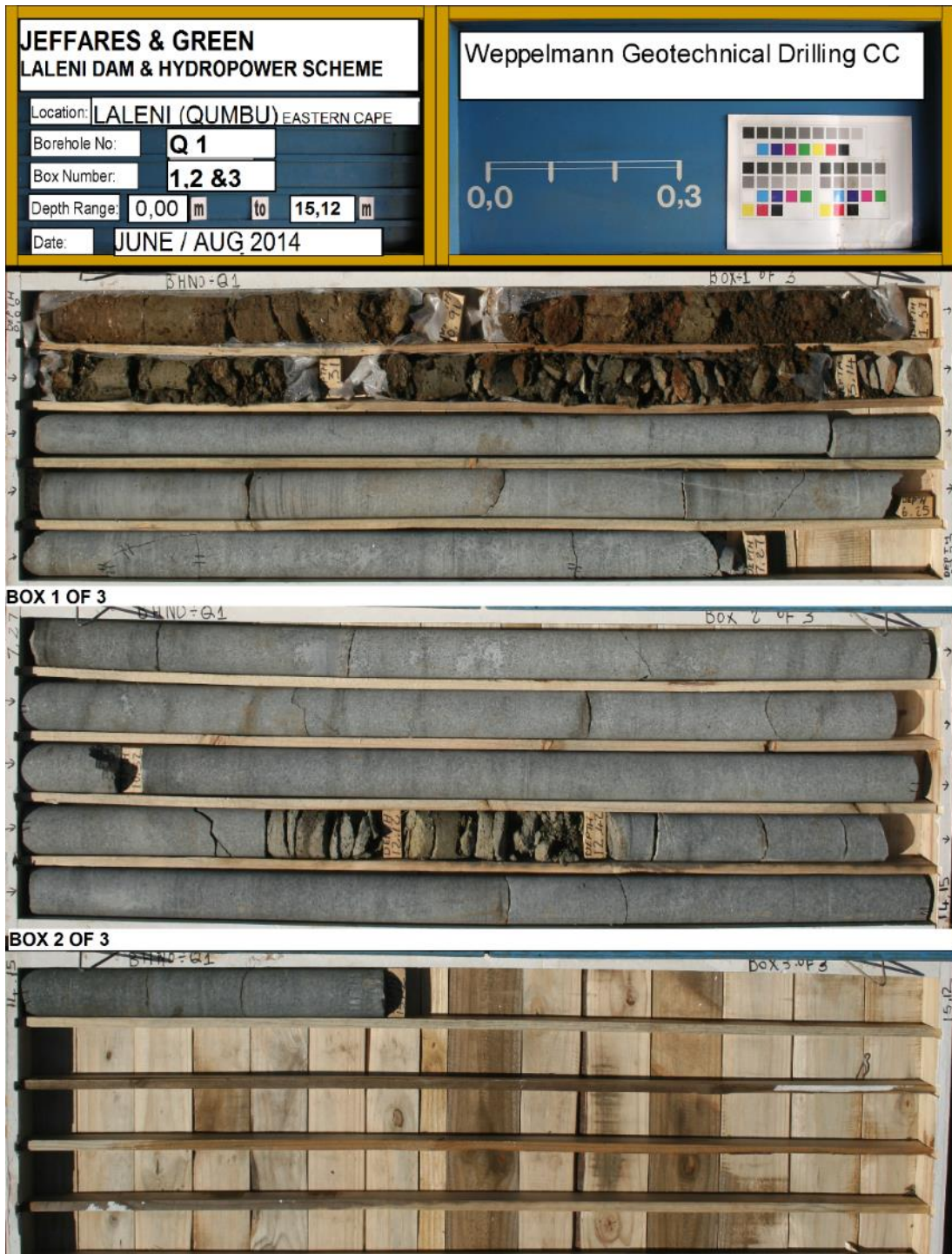


Fig C-2: Borehole Quarry 1 – Box 1 to 3 Dry

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

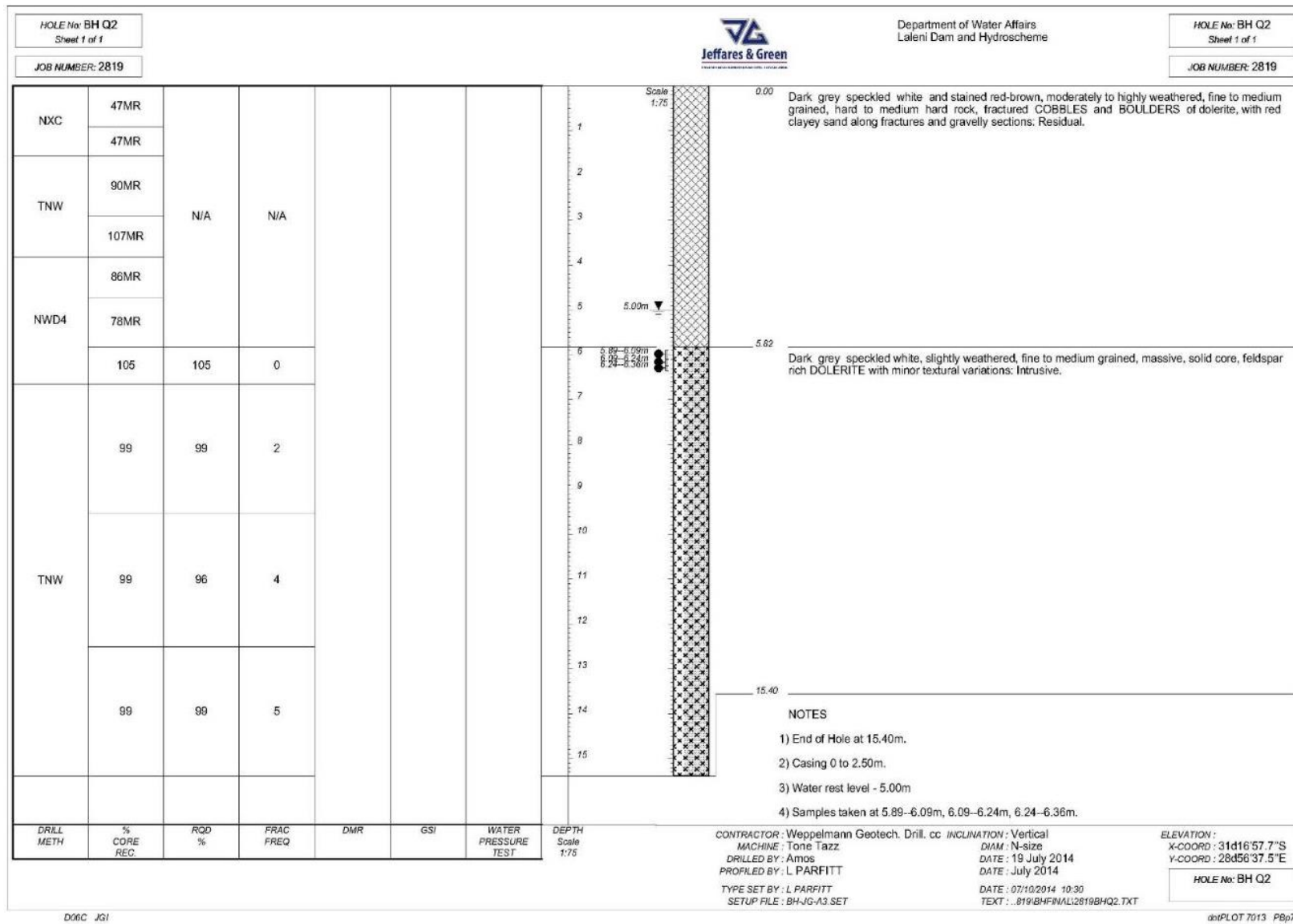


Fig C-3: Borehole Quarry 2 - Log

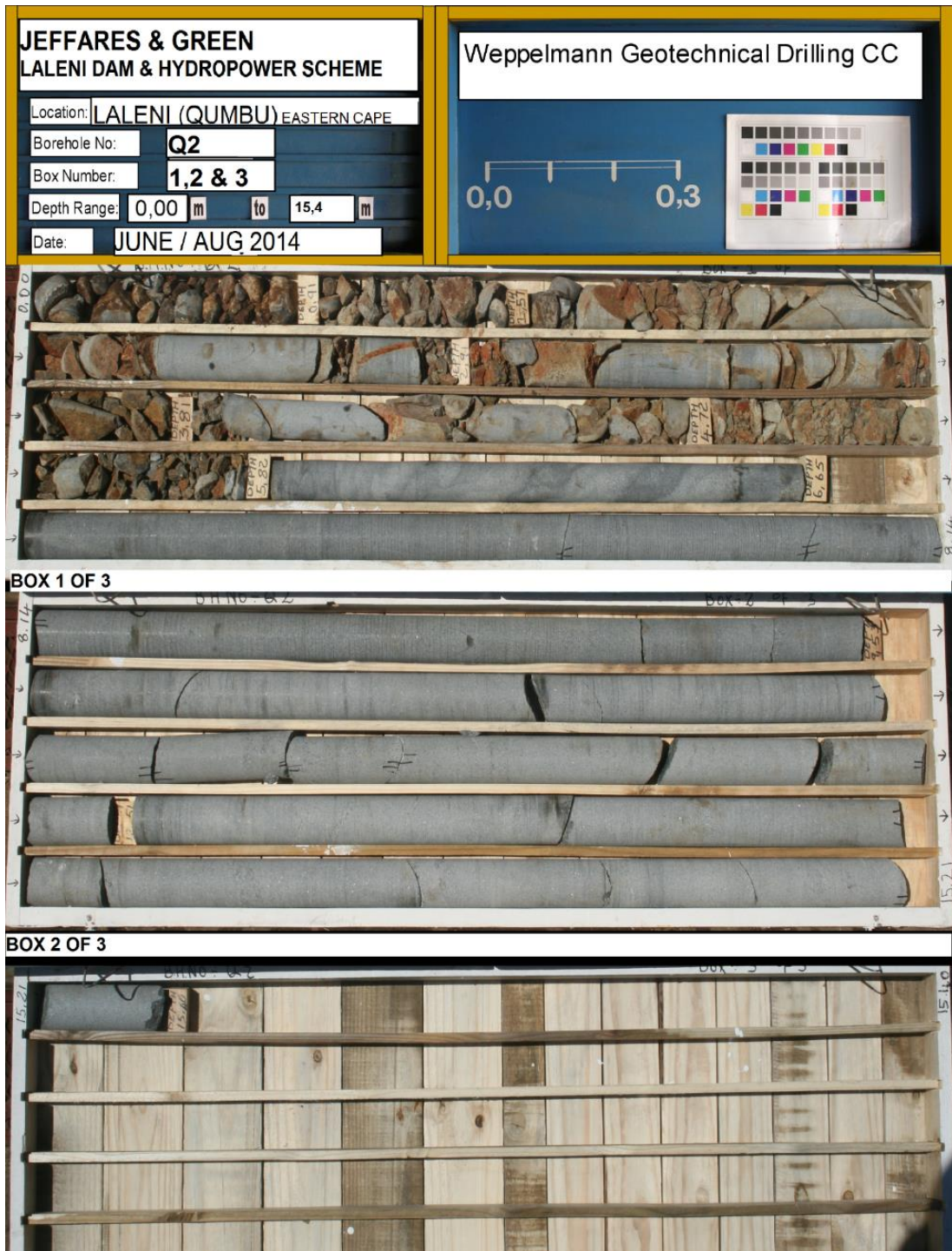


Fig C-4: Borehole Quarry 2 – Box 1 to 3 Dry

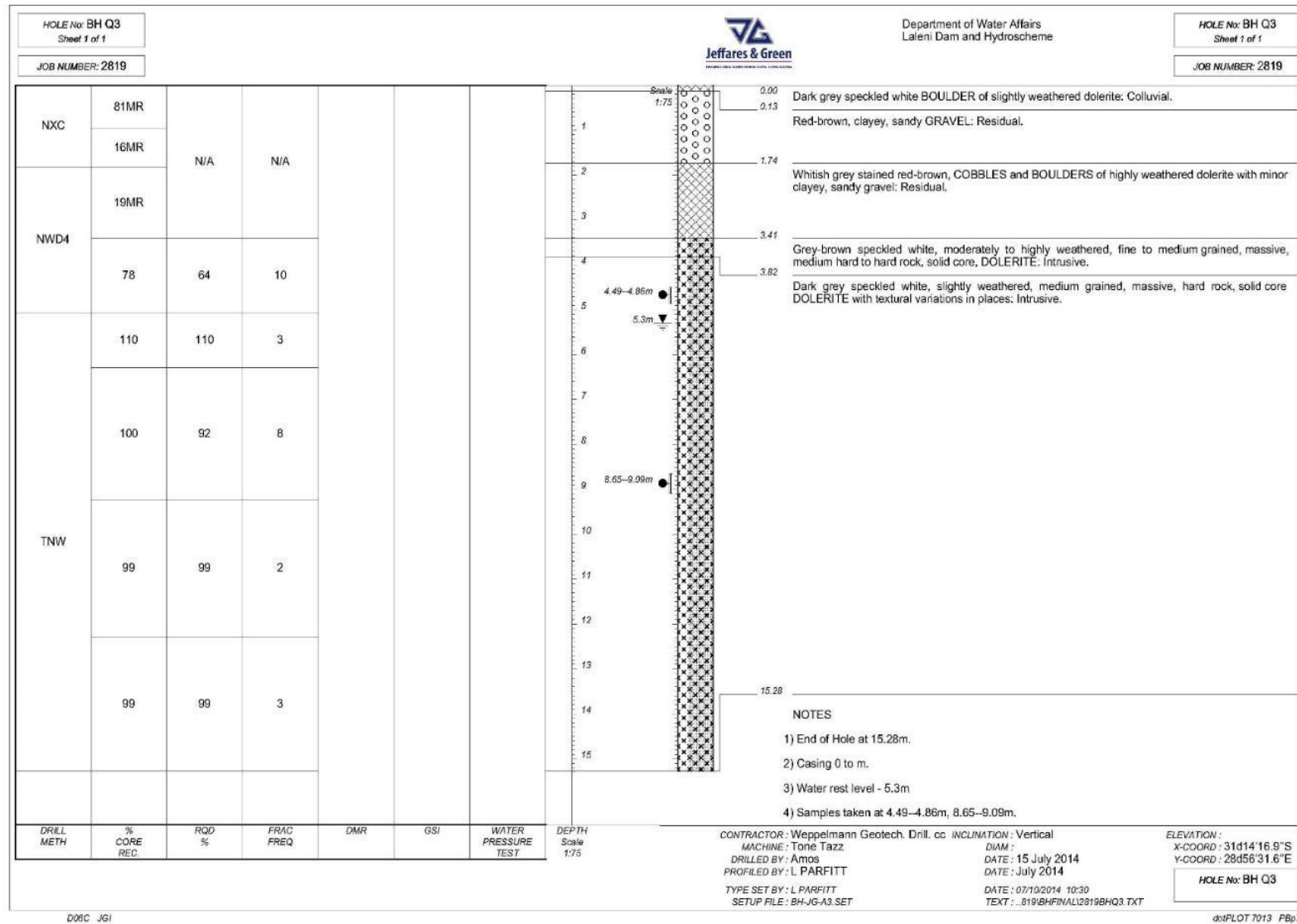


Fig C-5: Borehole Quarry 3 - Log



Fig C-6: Borehole Quarry 3 – Box 1 to 2 Dry

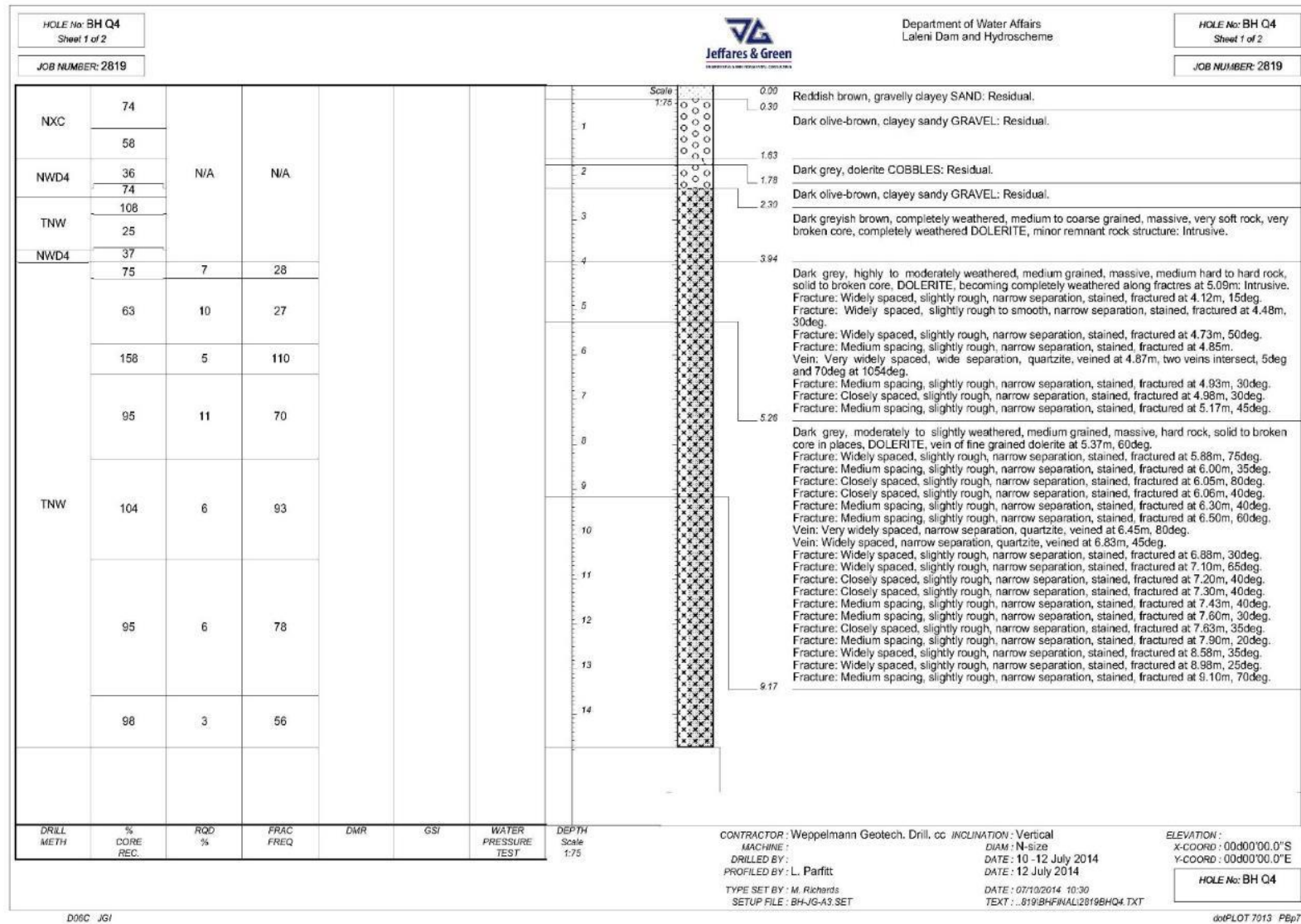


Fig C-7-1: Borehole Quarry 4 - Log

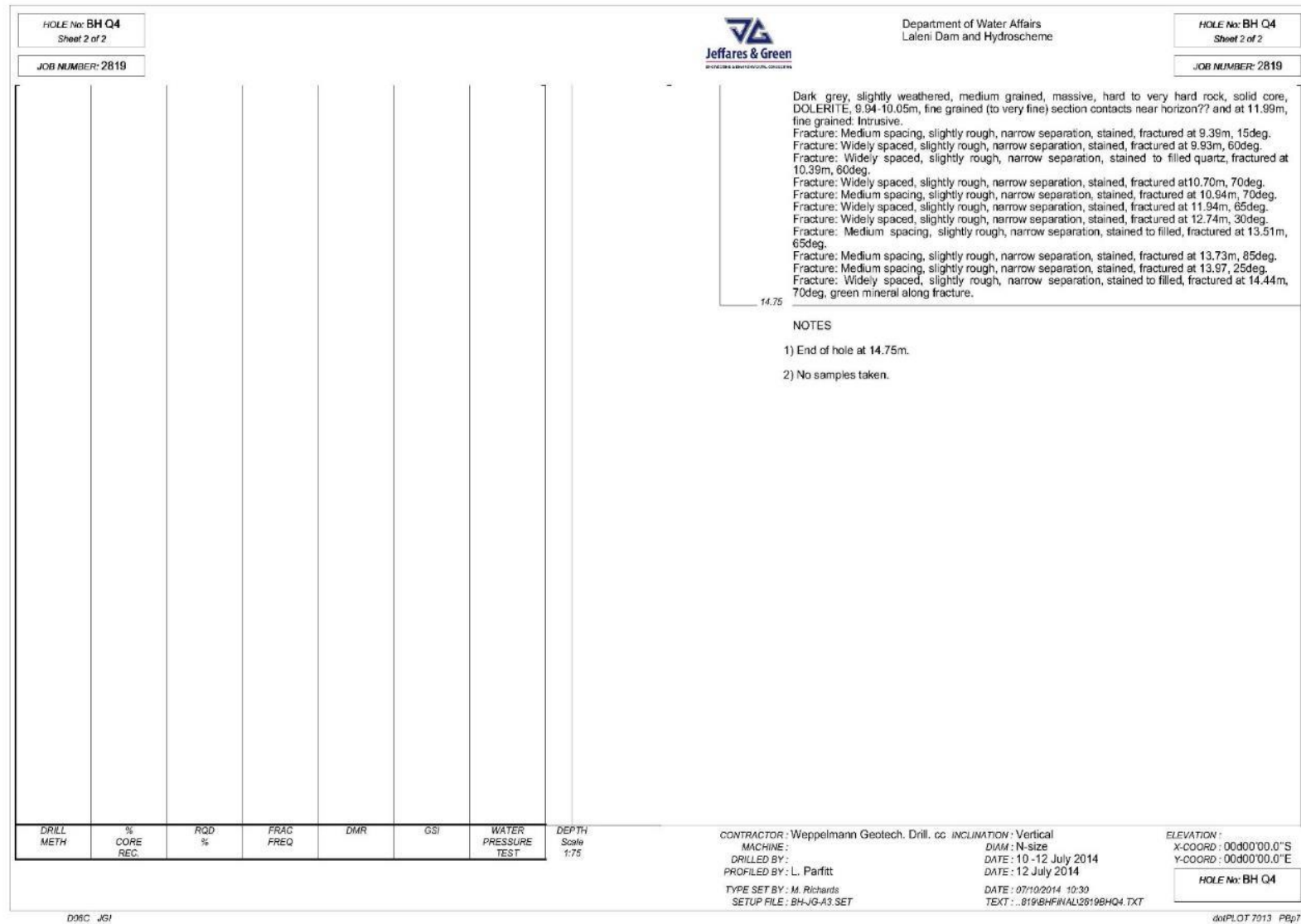


Fig C-7-2: Borehole Quarry 4 - Log



Fig C-8: Borehole Quarry 4 – Box 1 to 2 Dry

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

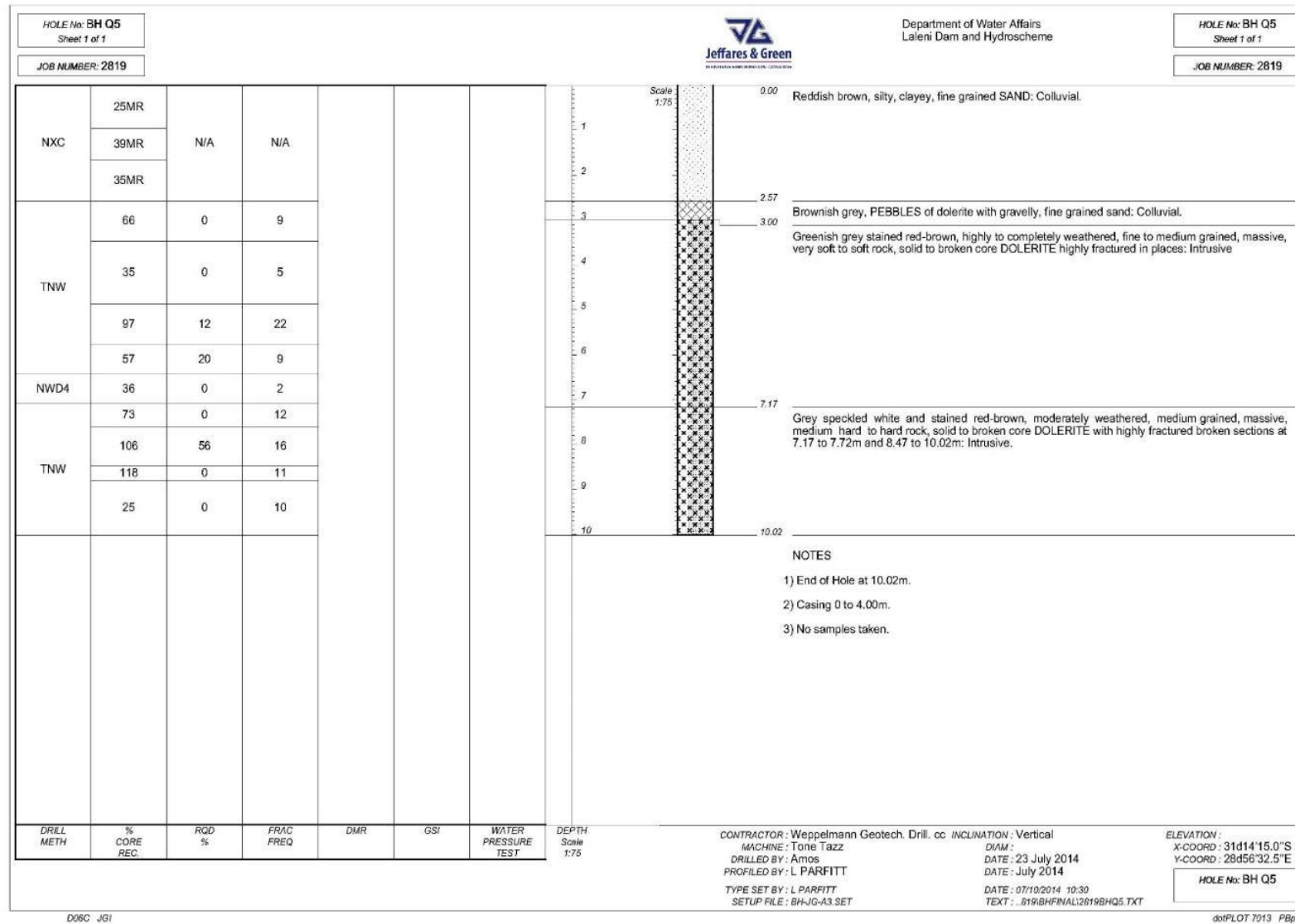


Fig C-8: Borehole Quarry 5 - Log

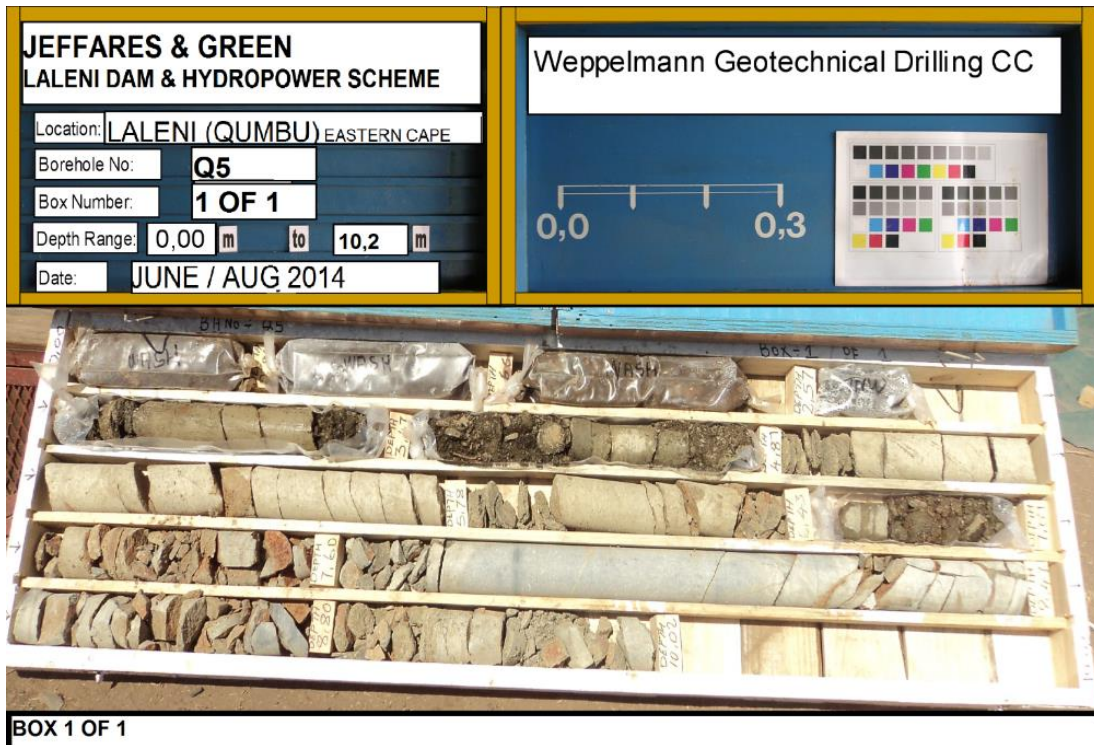


Fig C-9: Borehole Quarry 5 – Box 1 Dry

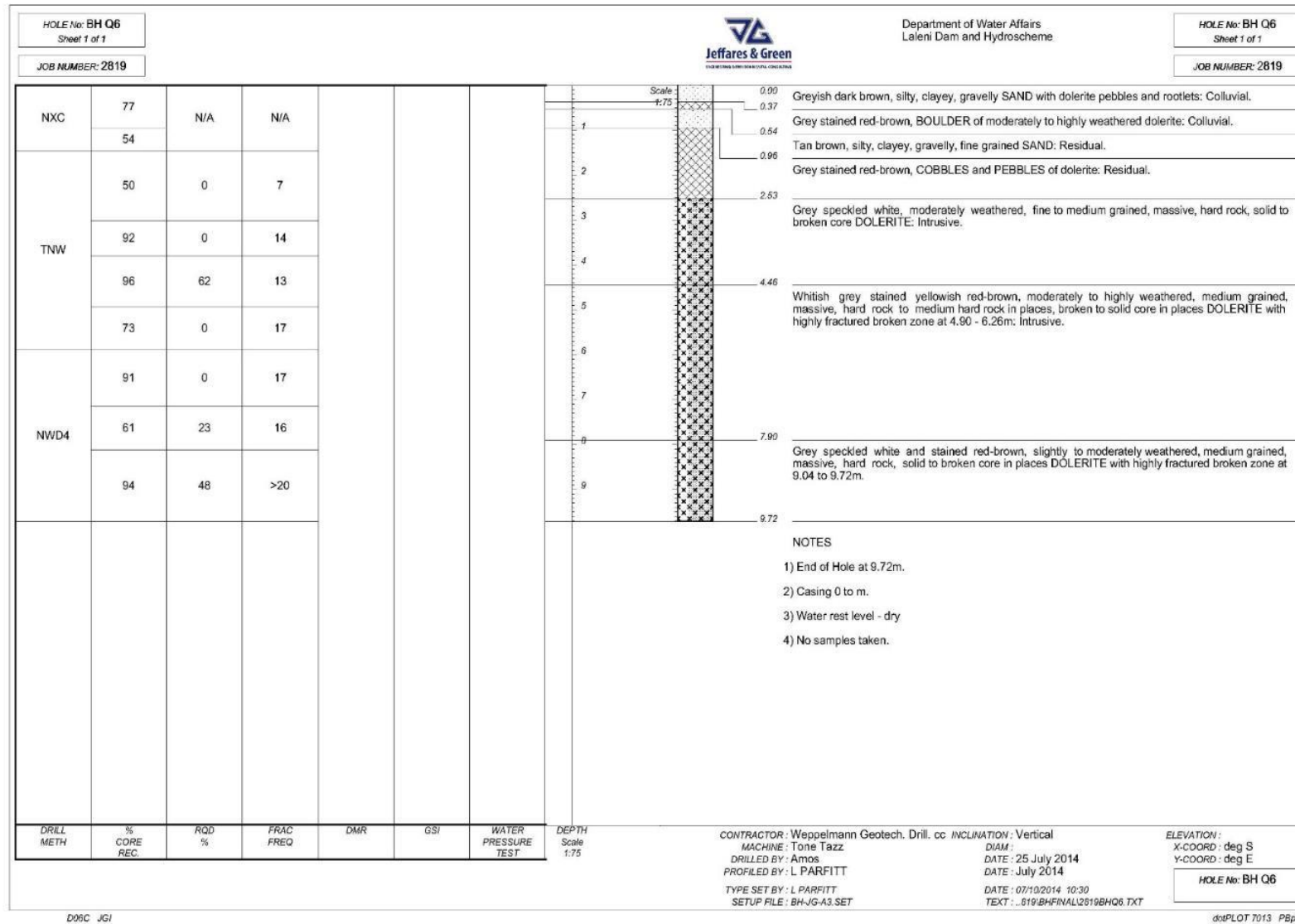


Fig C-10: Borehole Quarry 6 - Log

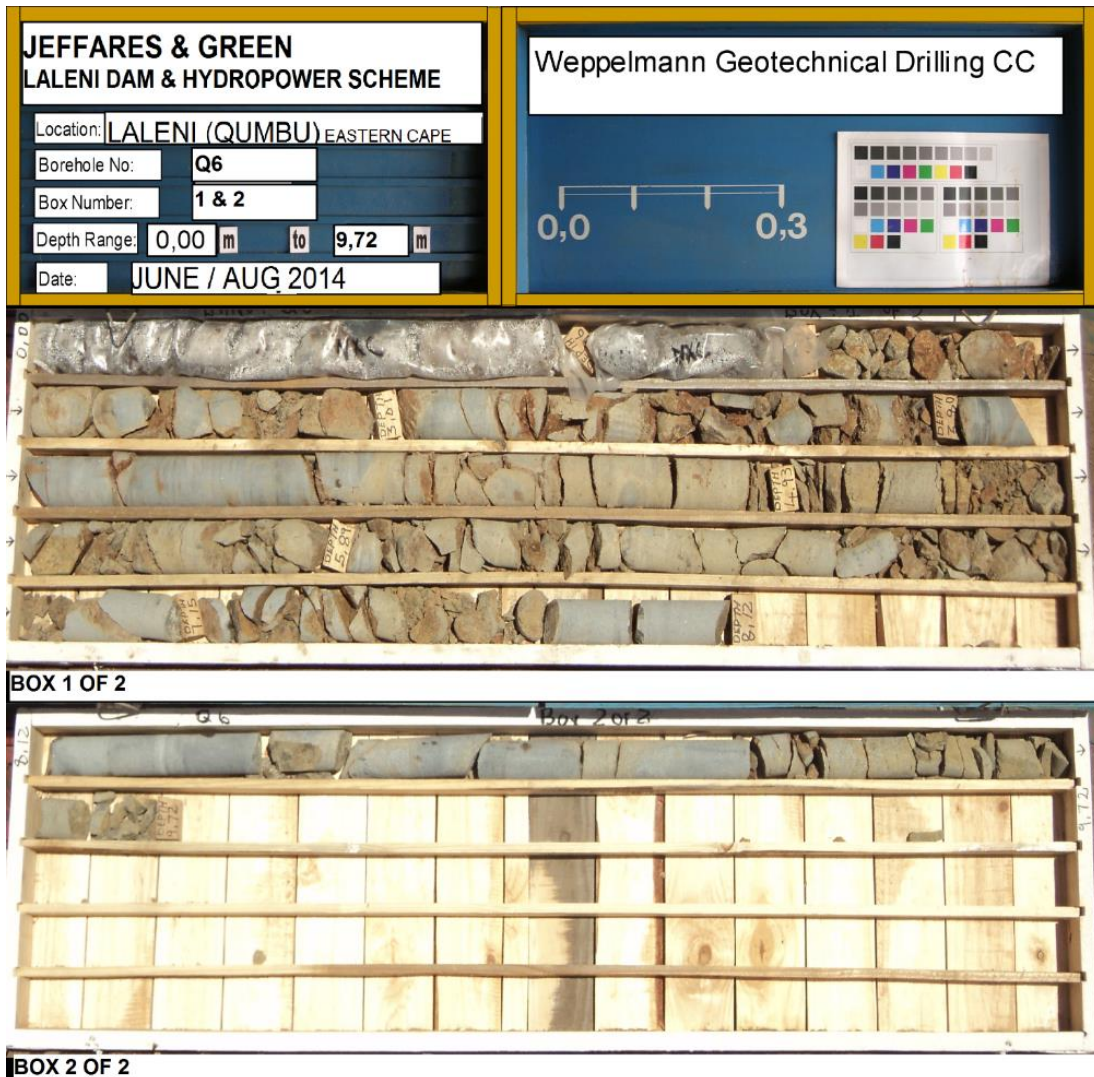


Fig C-11: Borehole Quarry 6 – Box 1 and 2 Dry

APPENDIX D

TUNNEL BOREHOLE LOGS, PHOTOGRAPHS AND WATER PRESSURE TESTS

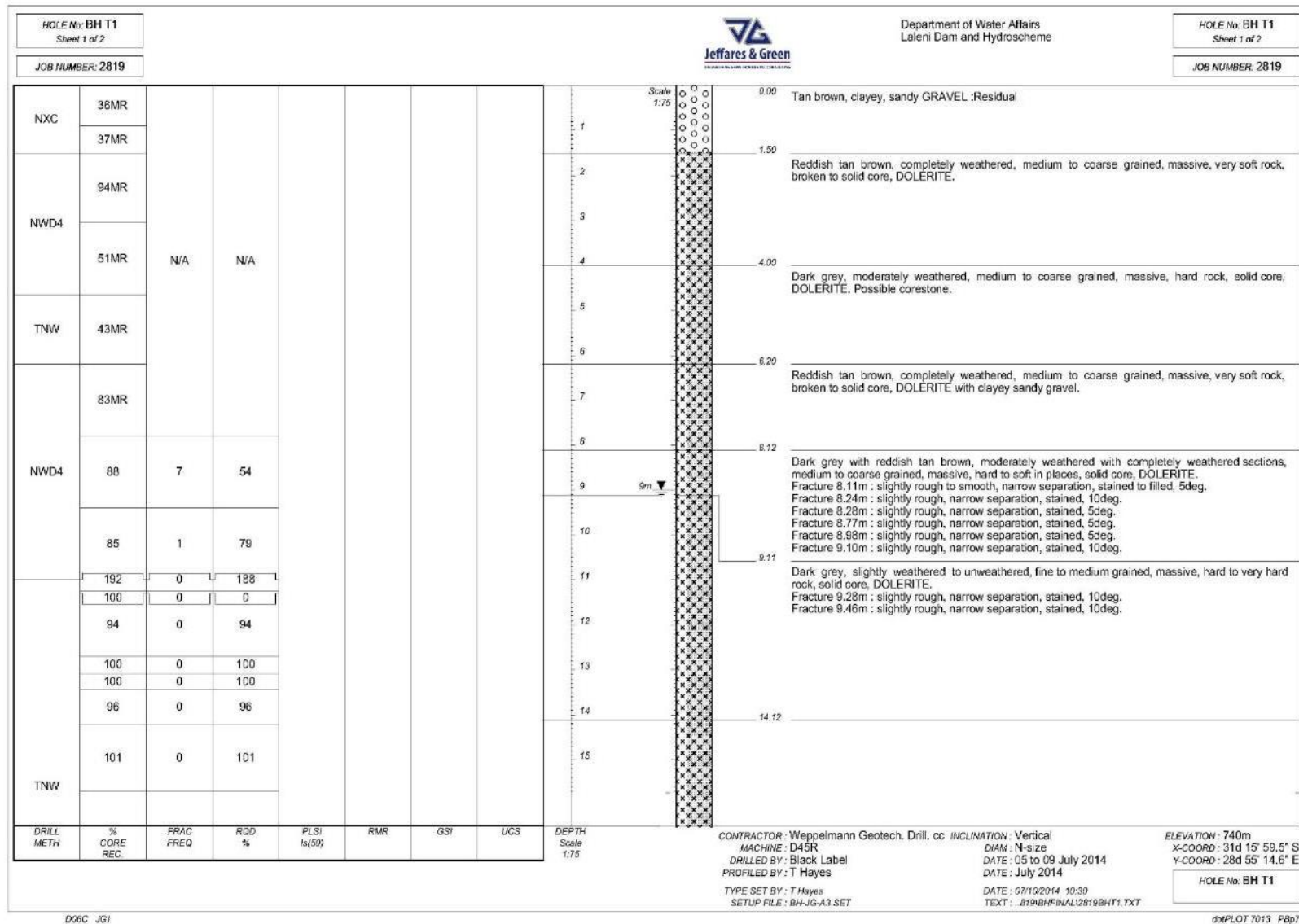


Fig D-1.1: Borehole Tunnel 1 - Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

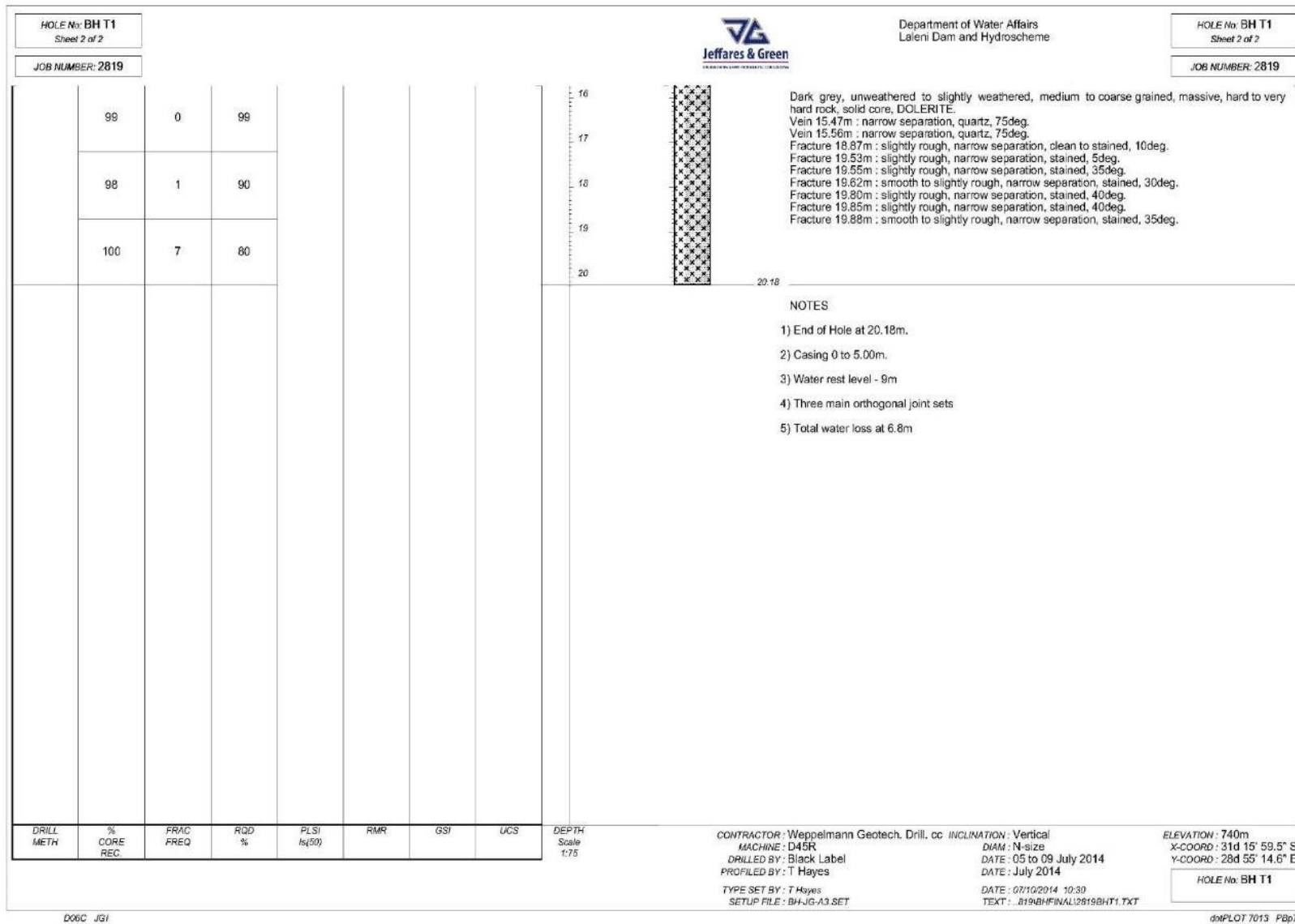


Fig D-1.2: Borehole Tunnel 1 - Log

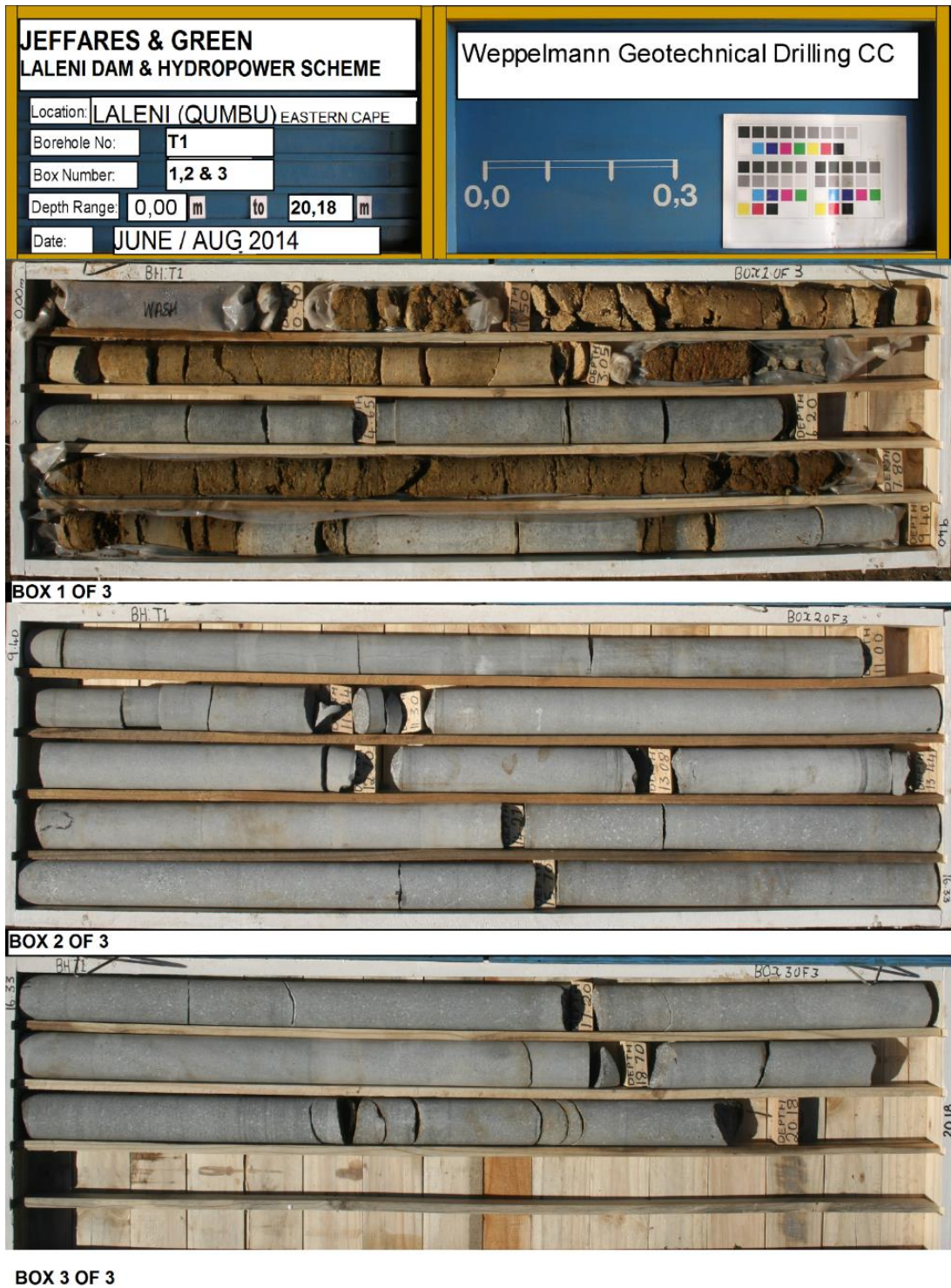


Fig D-2: Borehole Tunnel 1 – Box 1 to 3

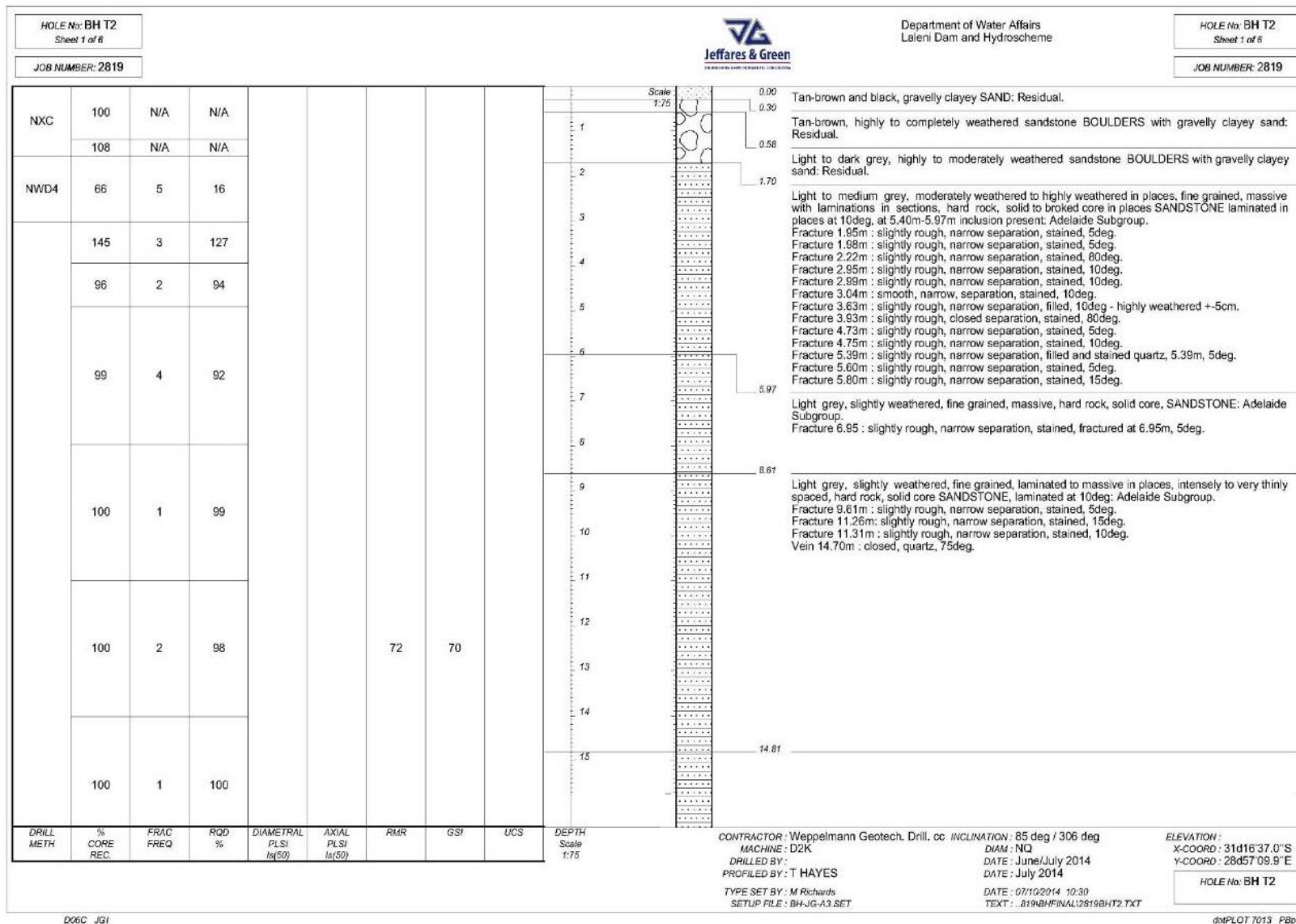


Fig D-3.1: Borehole Tunnel 2 - Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

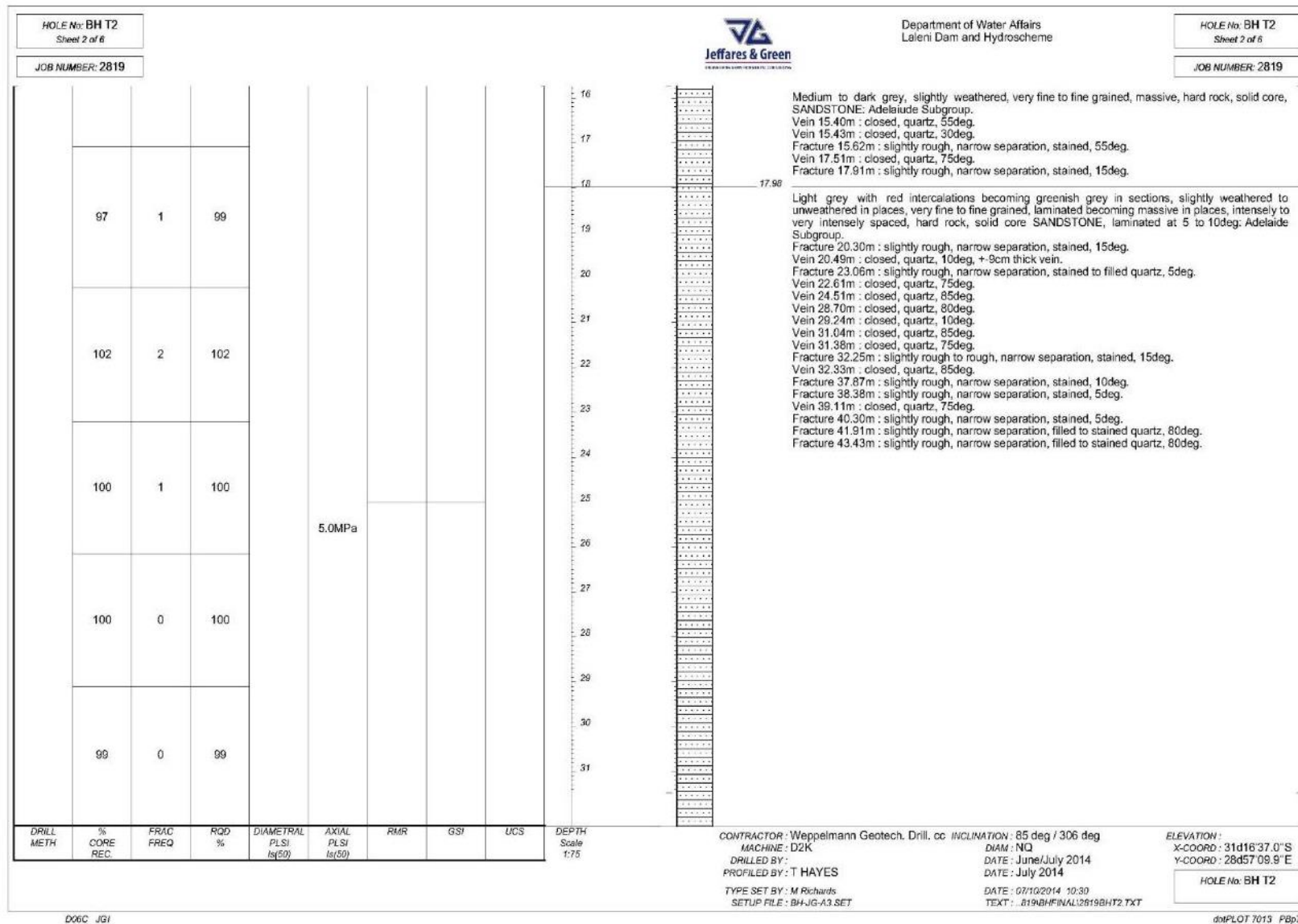


Fig D-3.2: Borehole Tunnel 2 - Log

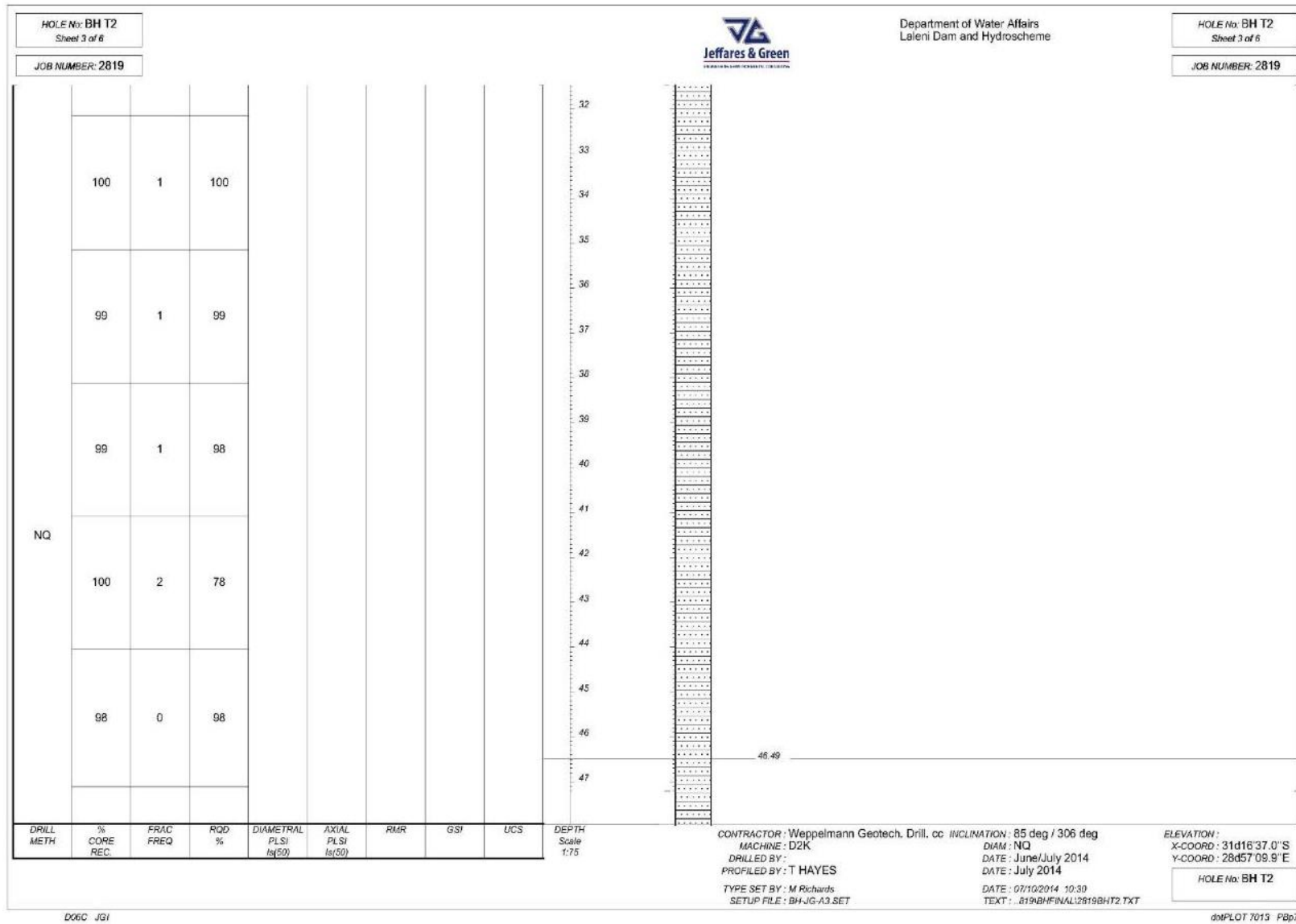


Fig D-3.3: Borehole Tunnel 2 - Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

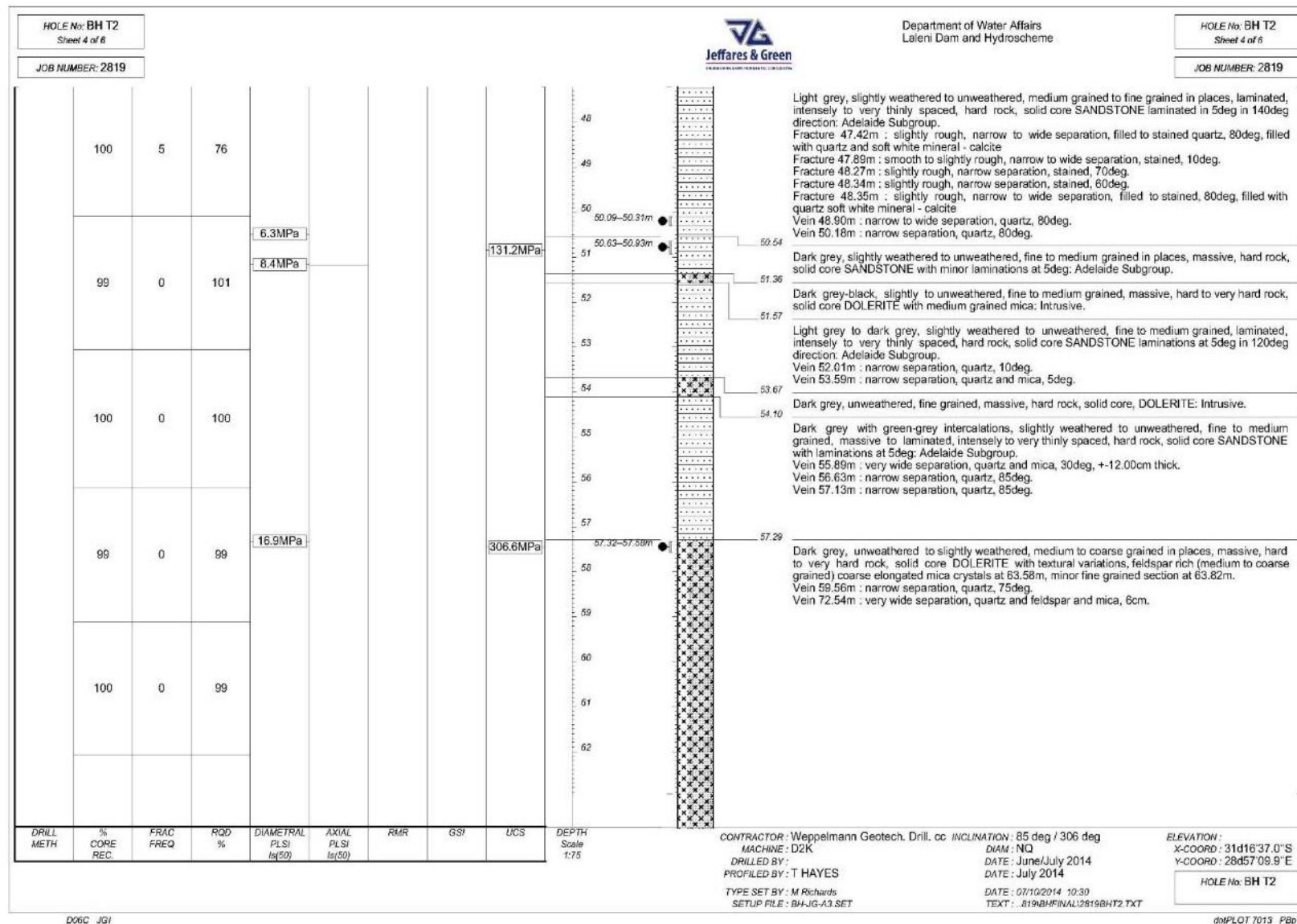


Fig D-3.4: Borehole Tunnel 2 - Log

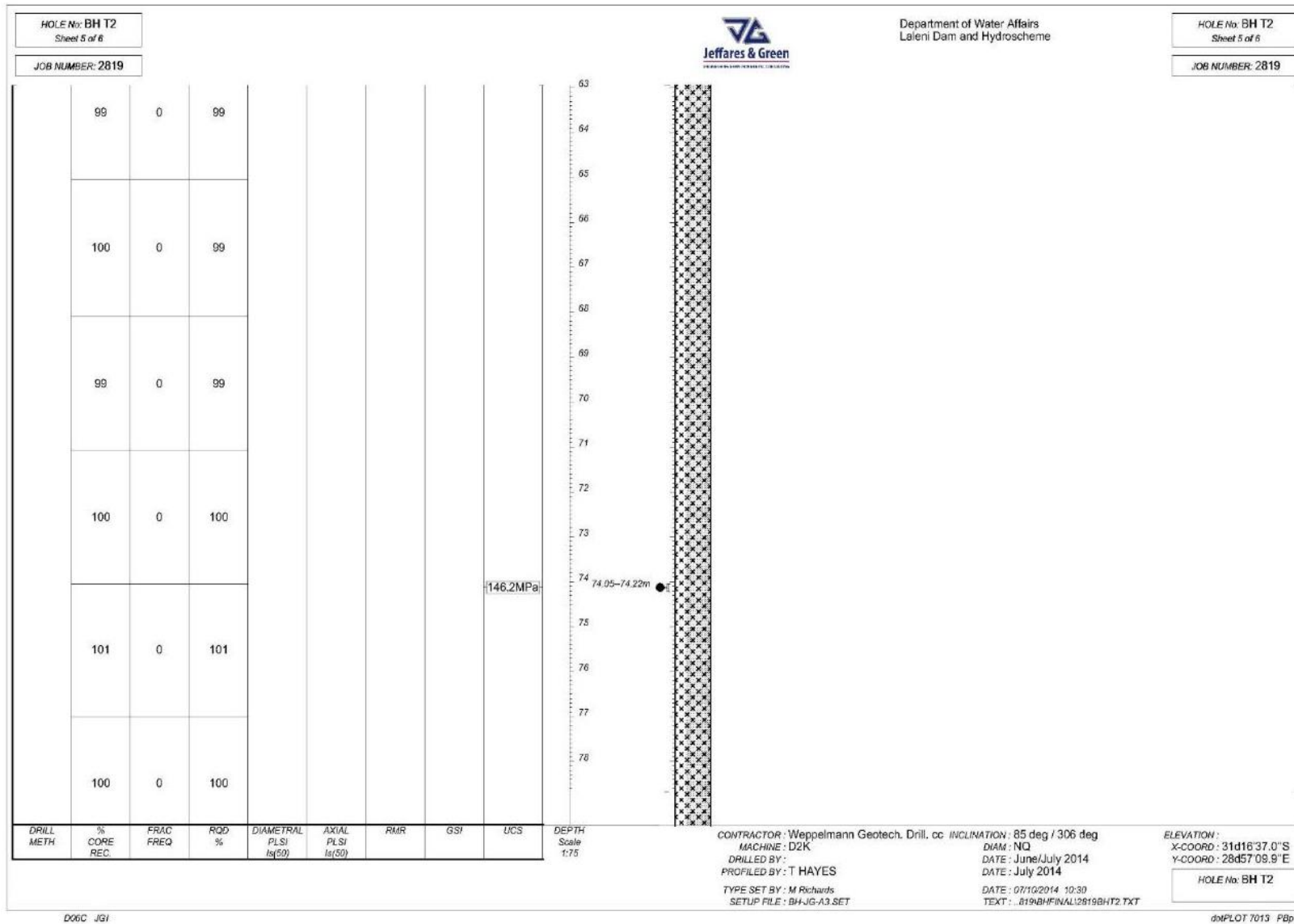


Fig D-3.5: Borehole Tunnel 2 - Log

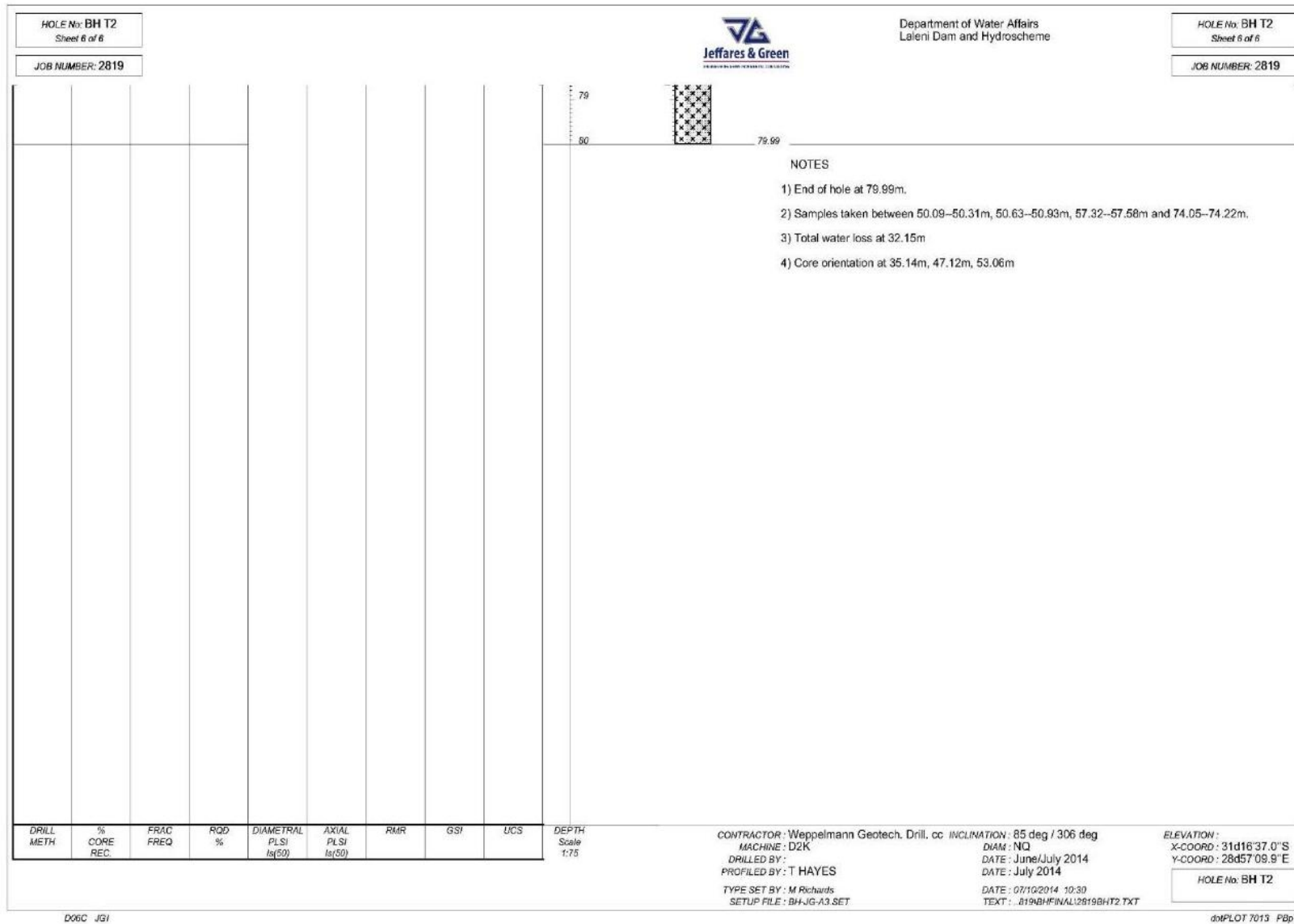


Fig D-3.6: Borehole Tunnel 2 - Log

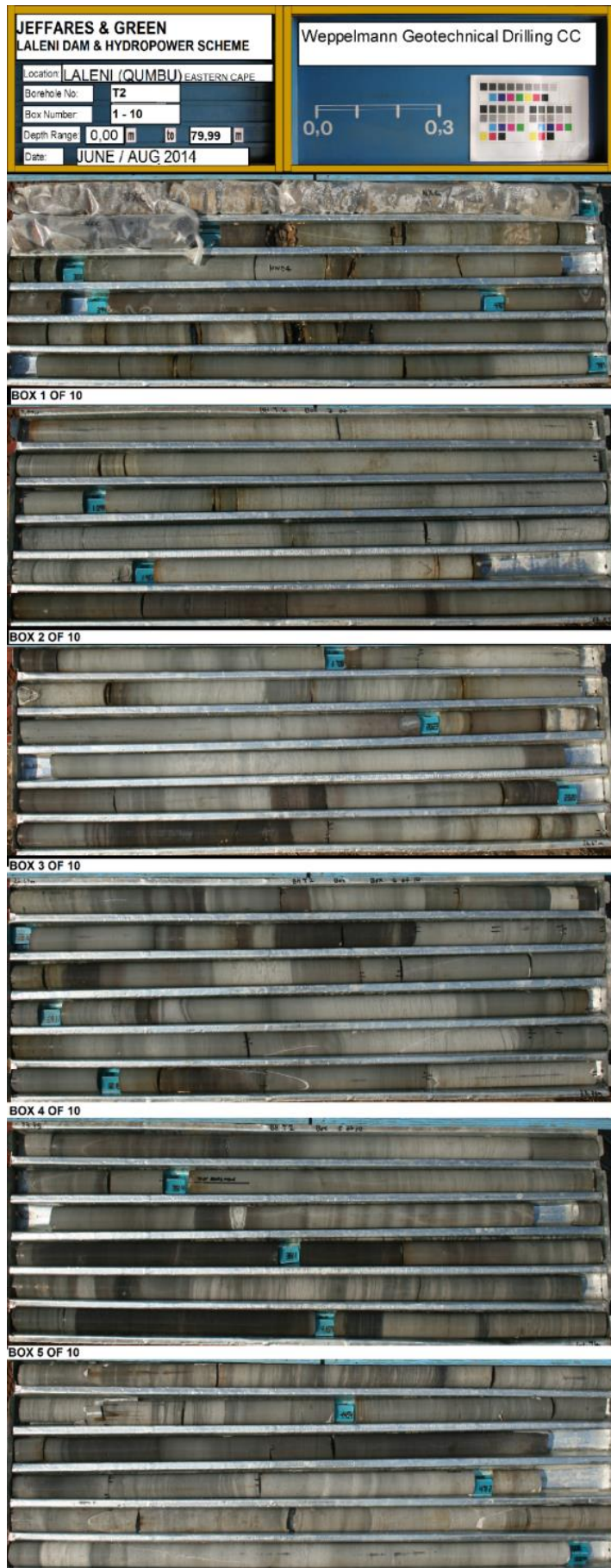


Fig D-4: Borehole Tunnel 1 – Box 1 to 5

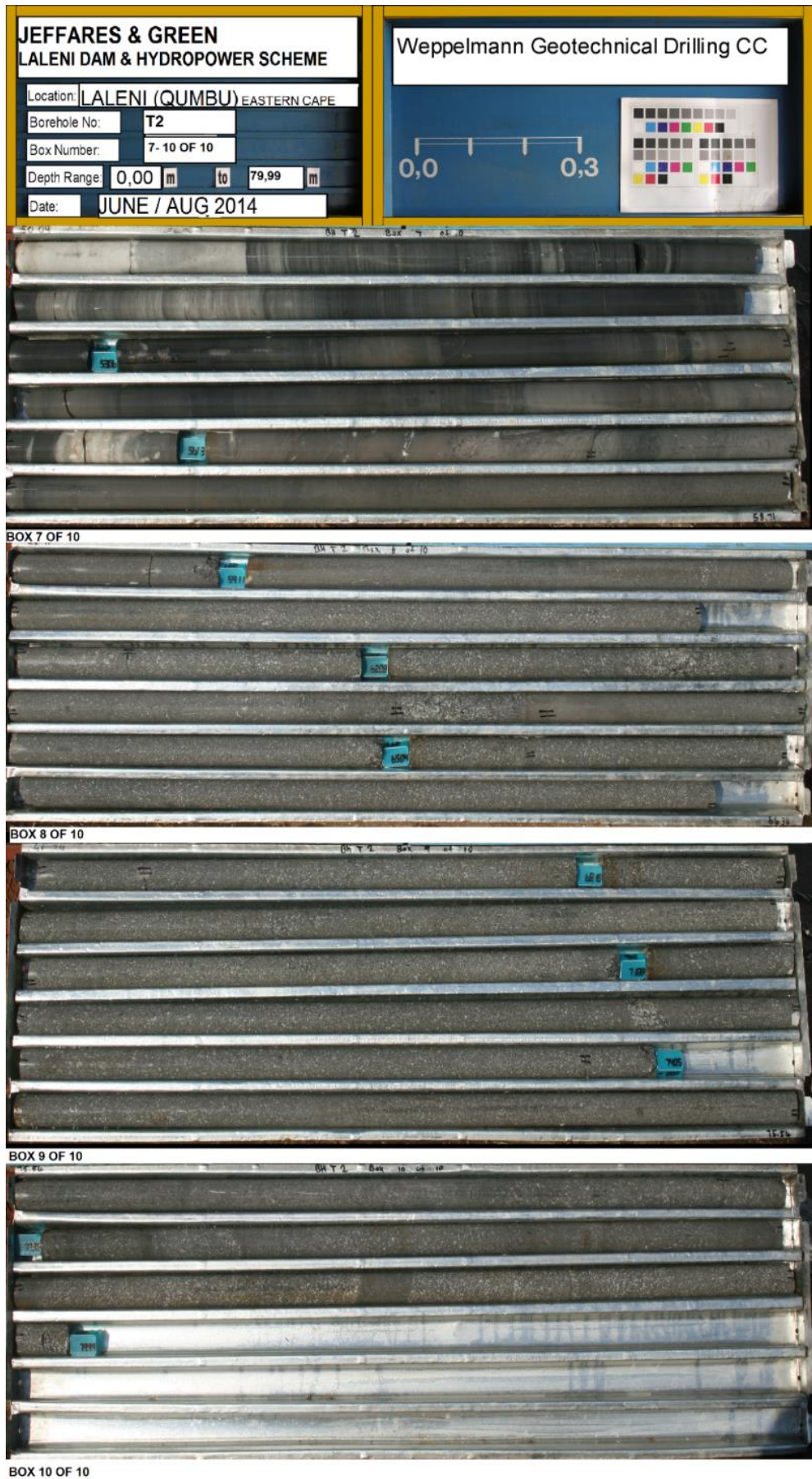


Fig D-4: Borehole Tunnel 1 – Box 6 to 10

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

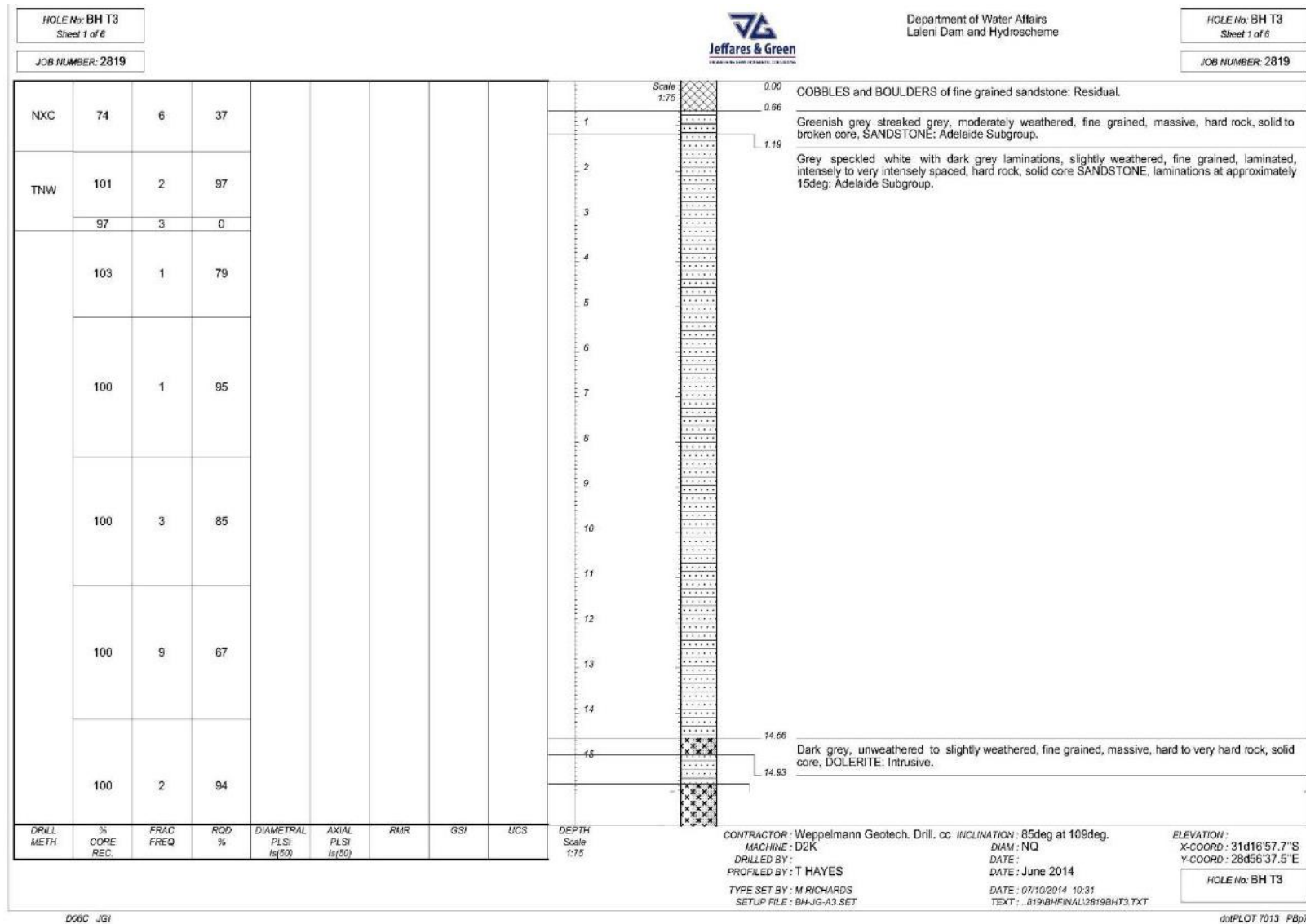


Fig D-5.1: Borehole Tunnel 3 – Log

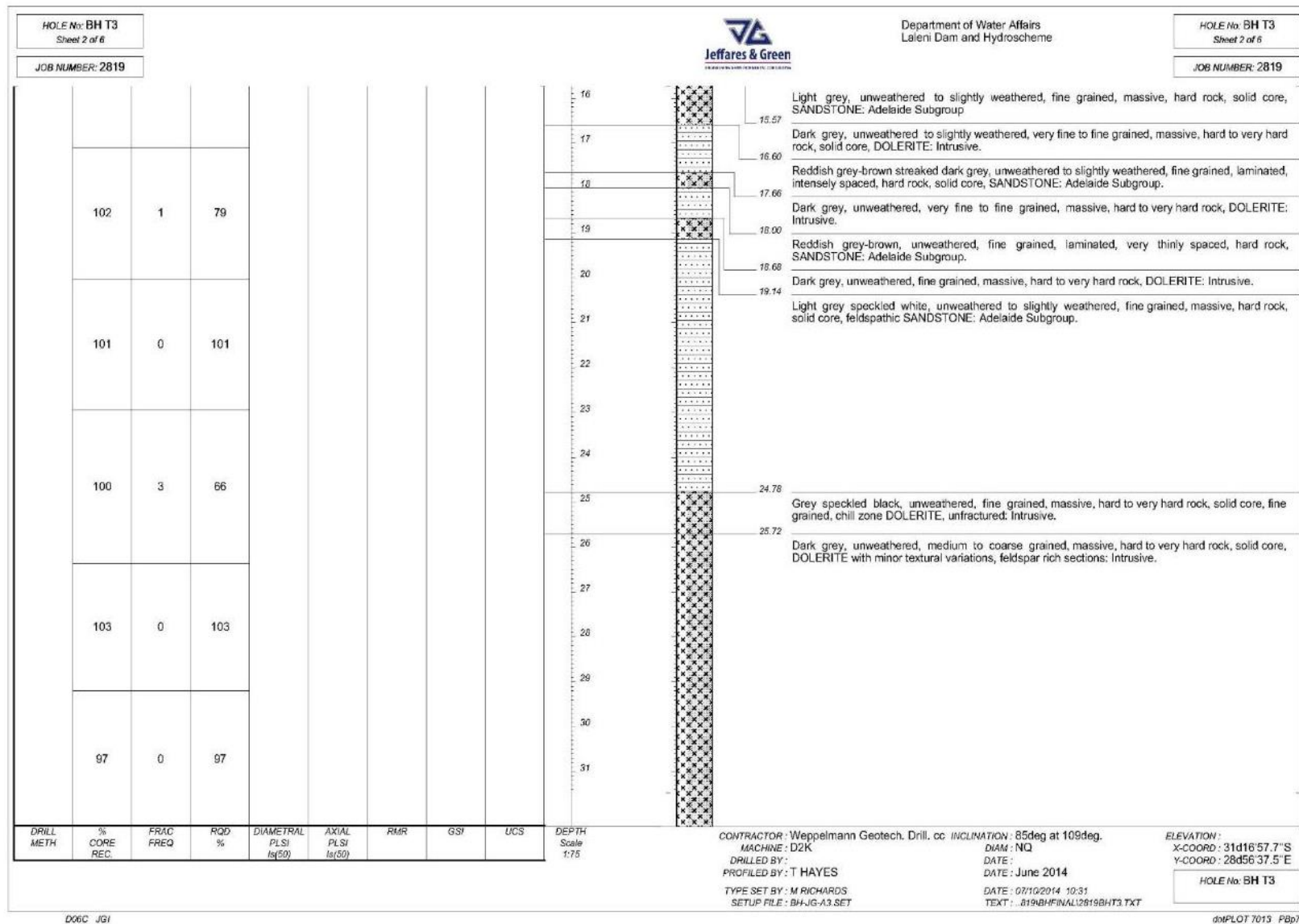


Fig D-5.2: Borehole Tunnel 3 – Log

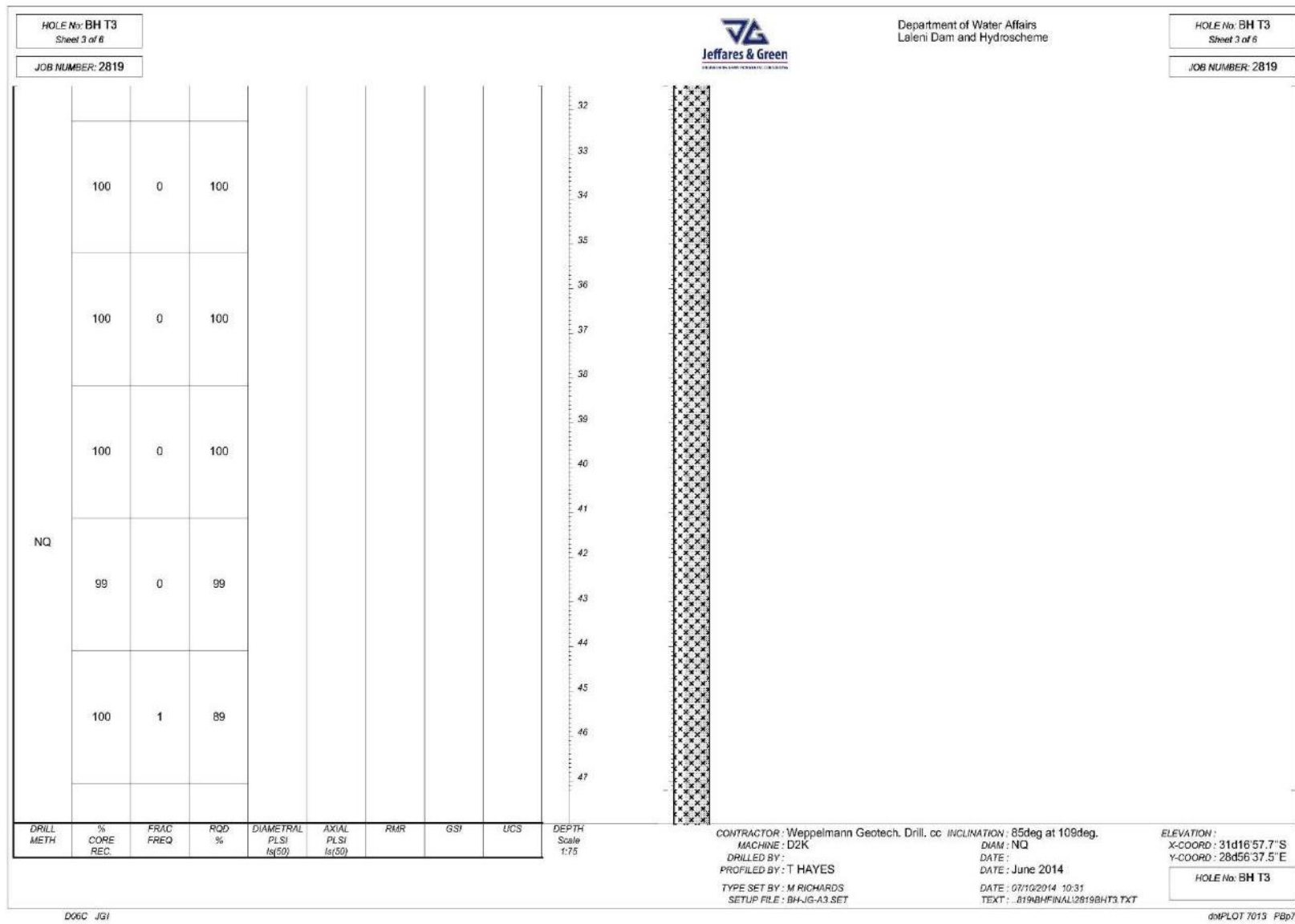


Fig D-5.3: Borehole Tunnel 3 – Log

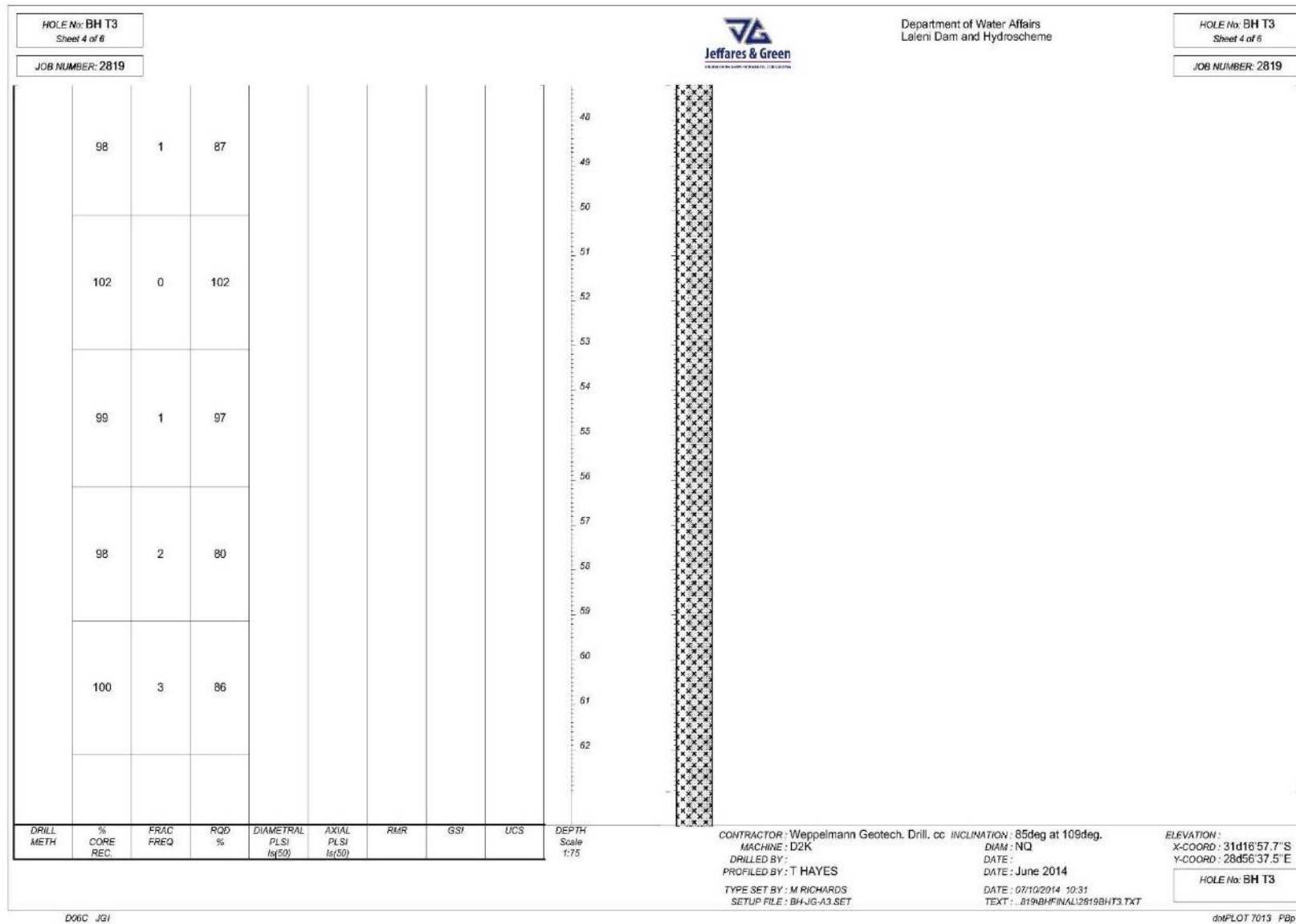


Fig D-5.4: Borehole Tunnel 3 – Log

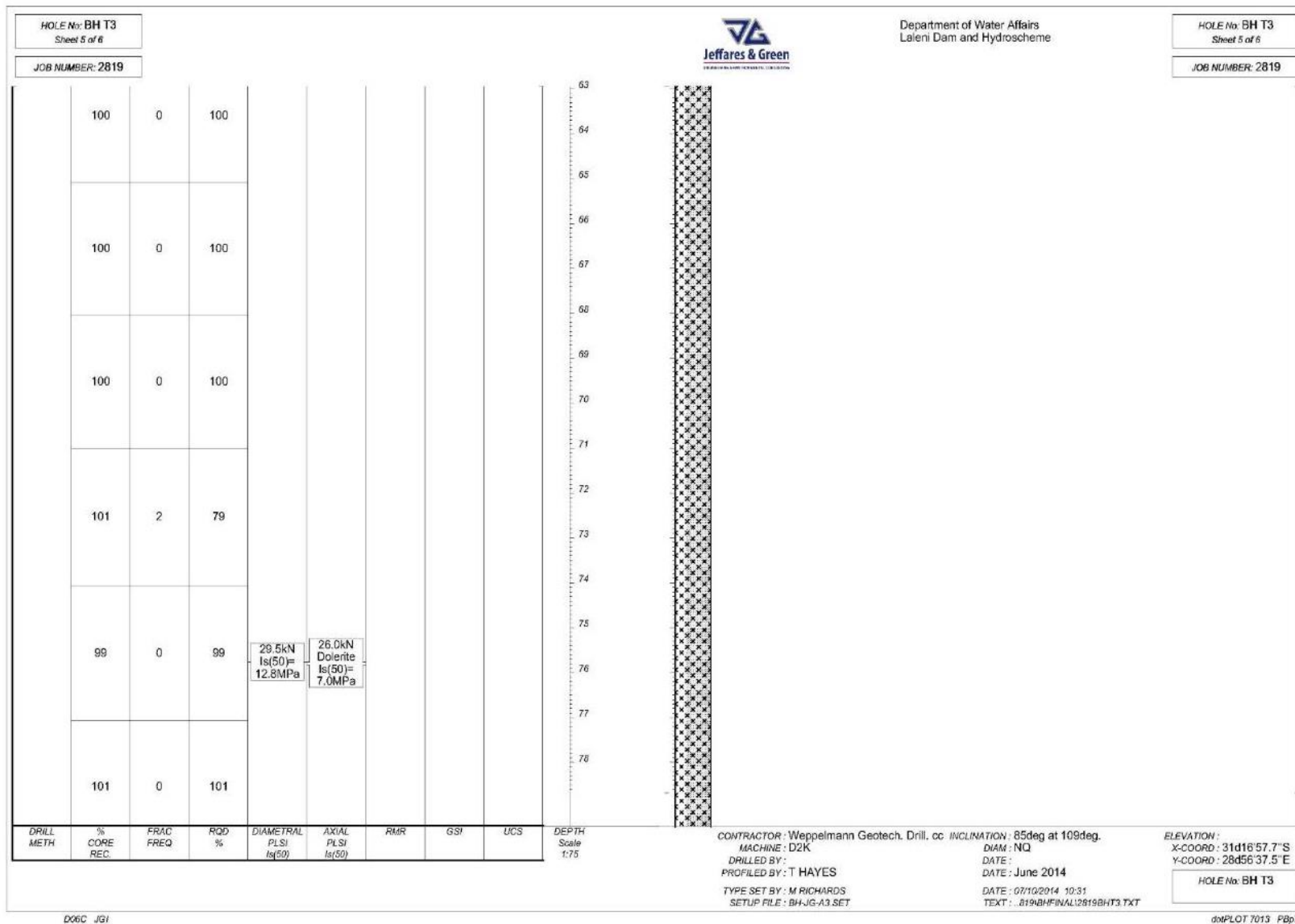


Fig D-5.5: Borehole Tunnel 3 – Log

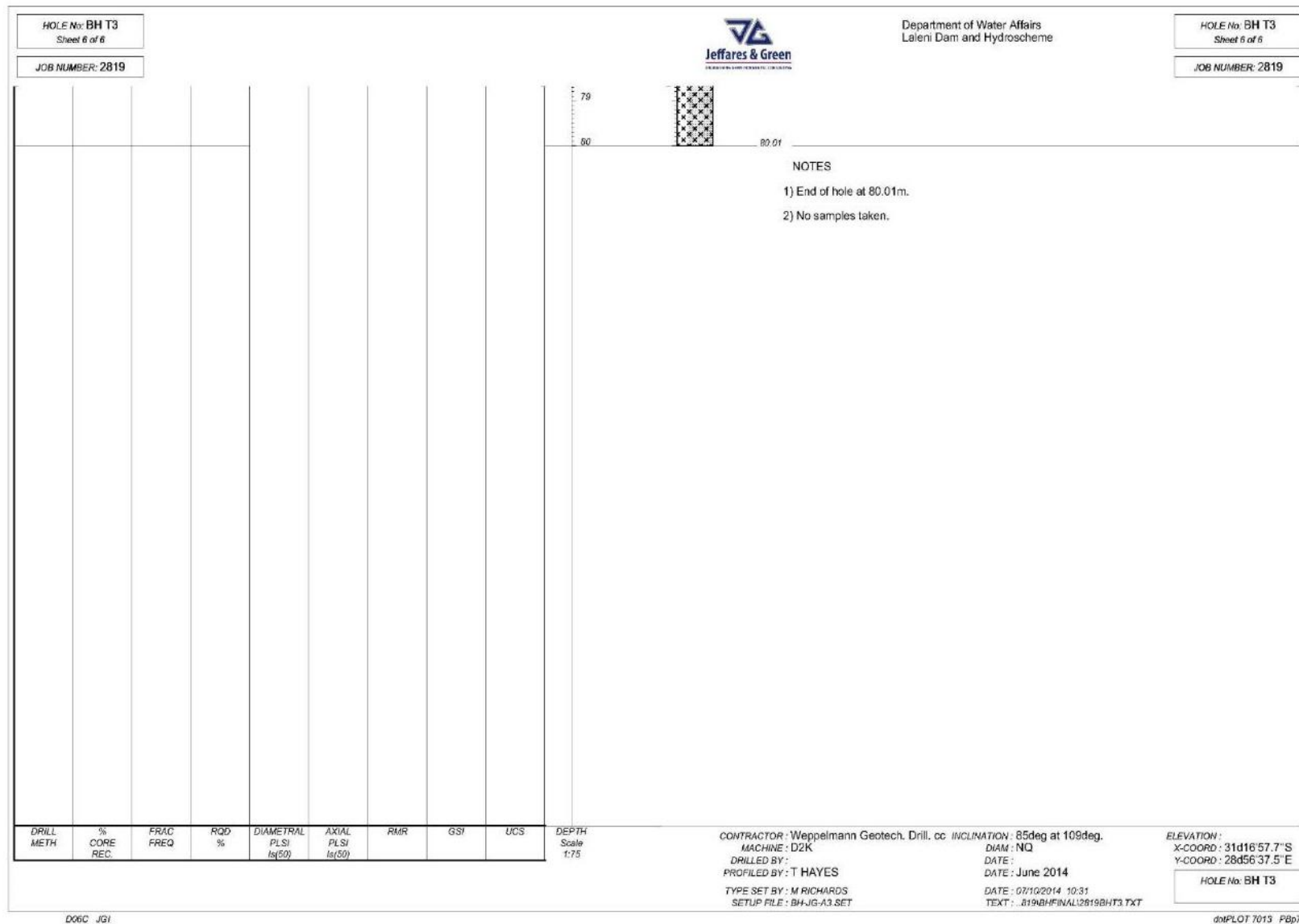


Fig D-5.6: Borehole Tunnel 3 – Log

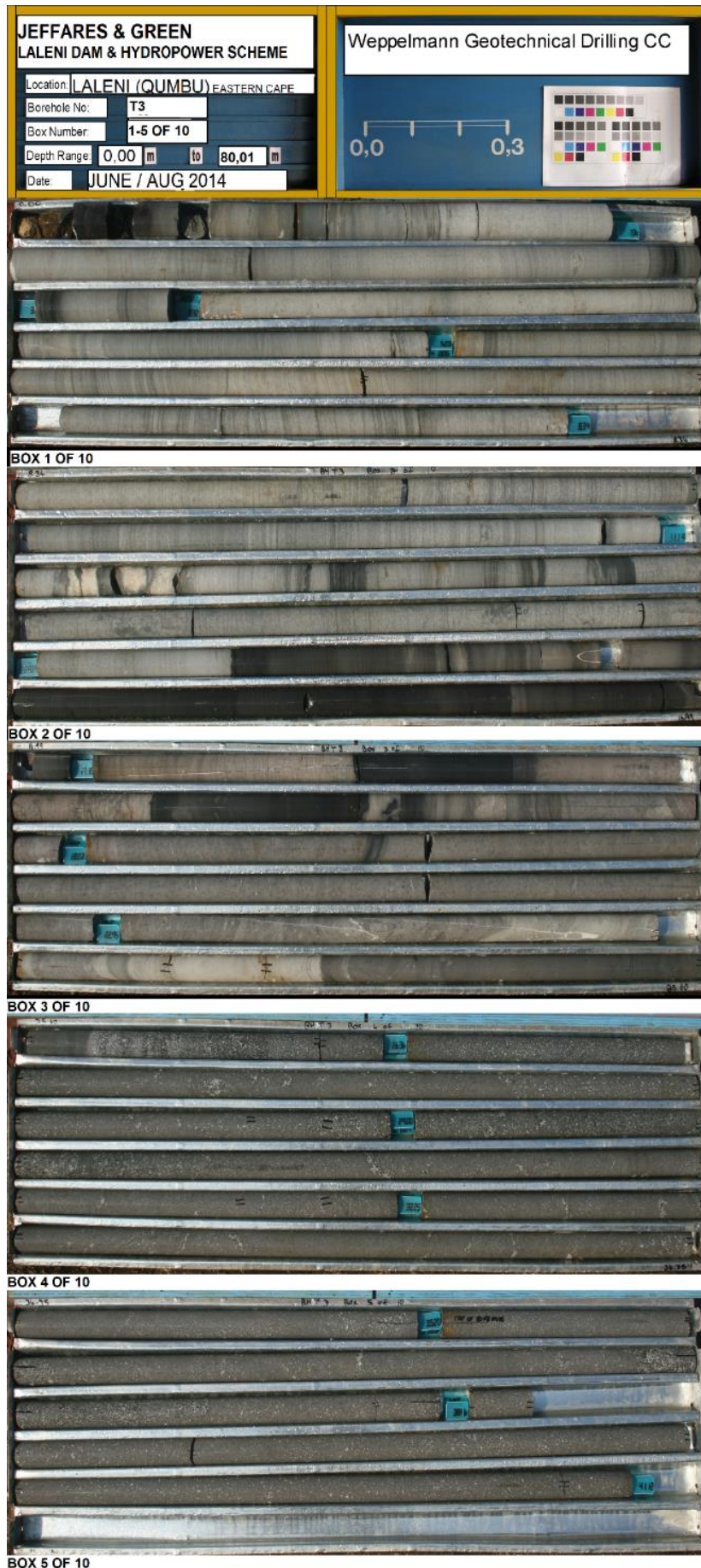


Fig D-6: Borehole Tunnel 3 – Box 1 to 4

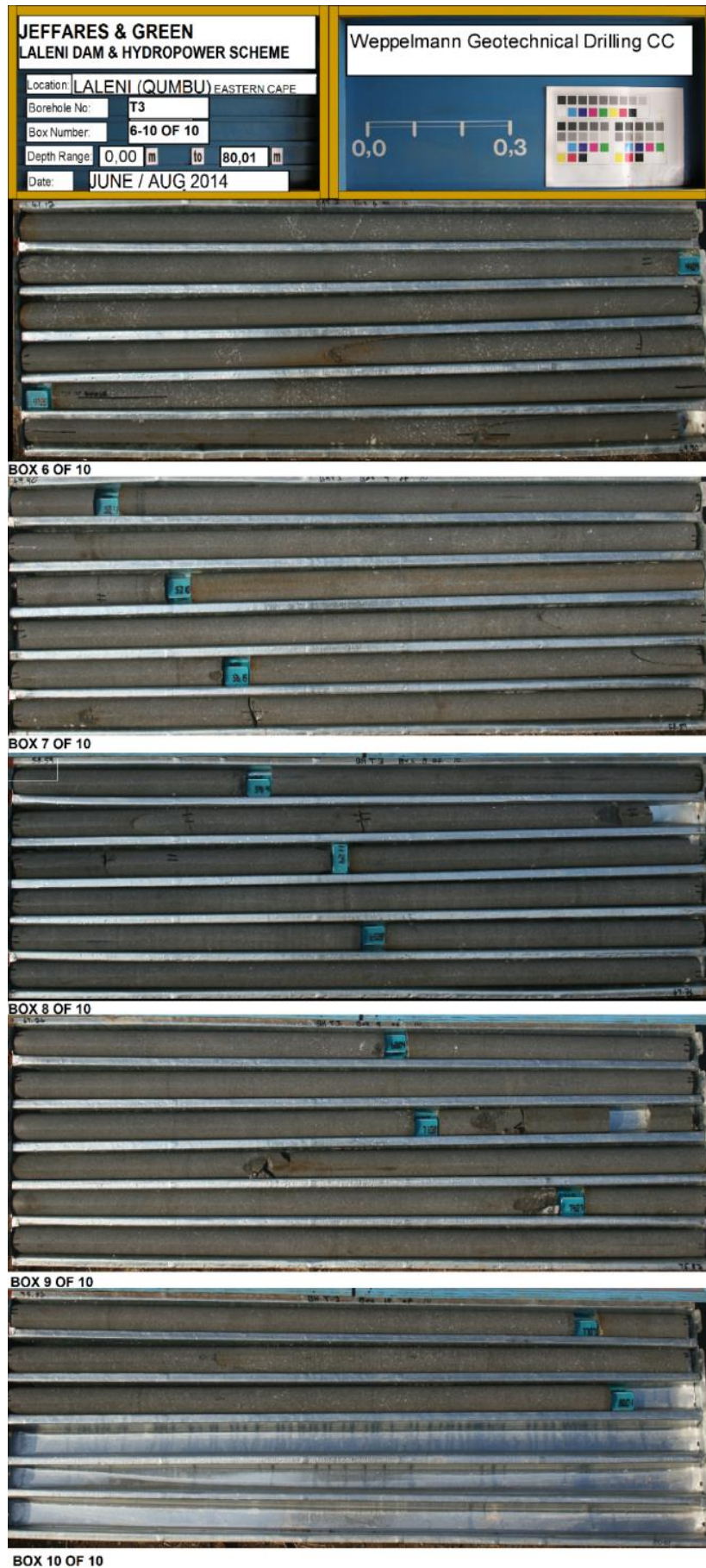


Fig D-6: Borehole Tunnel 3 – Box 5 to 10

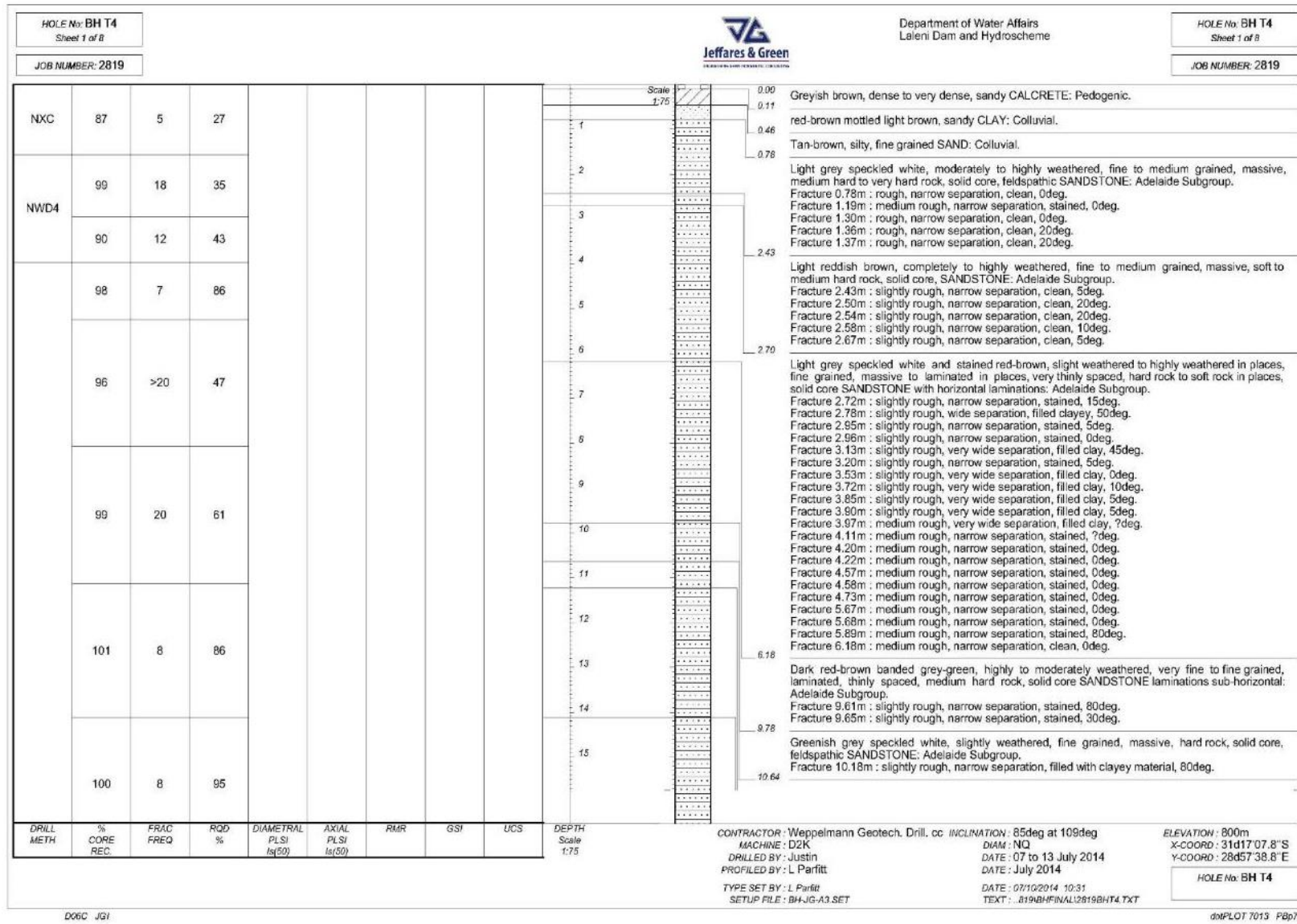


Fig D-7.1: Borehole Tunnel 4 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

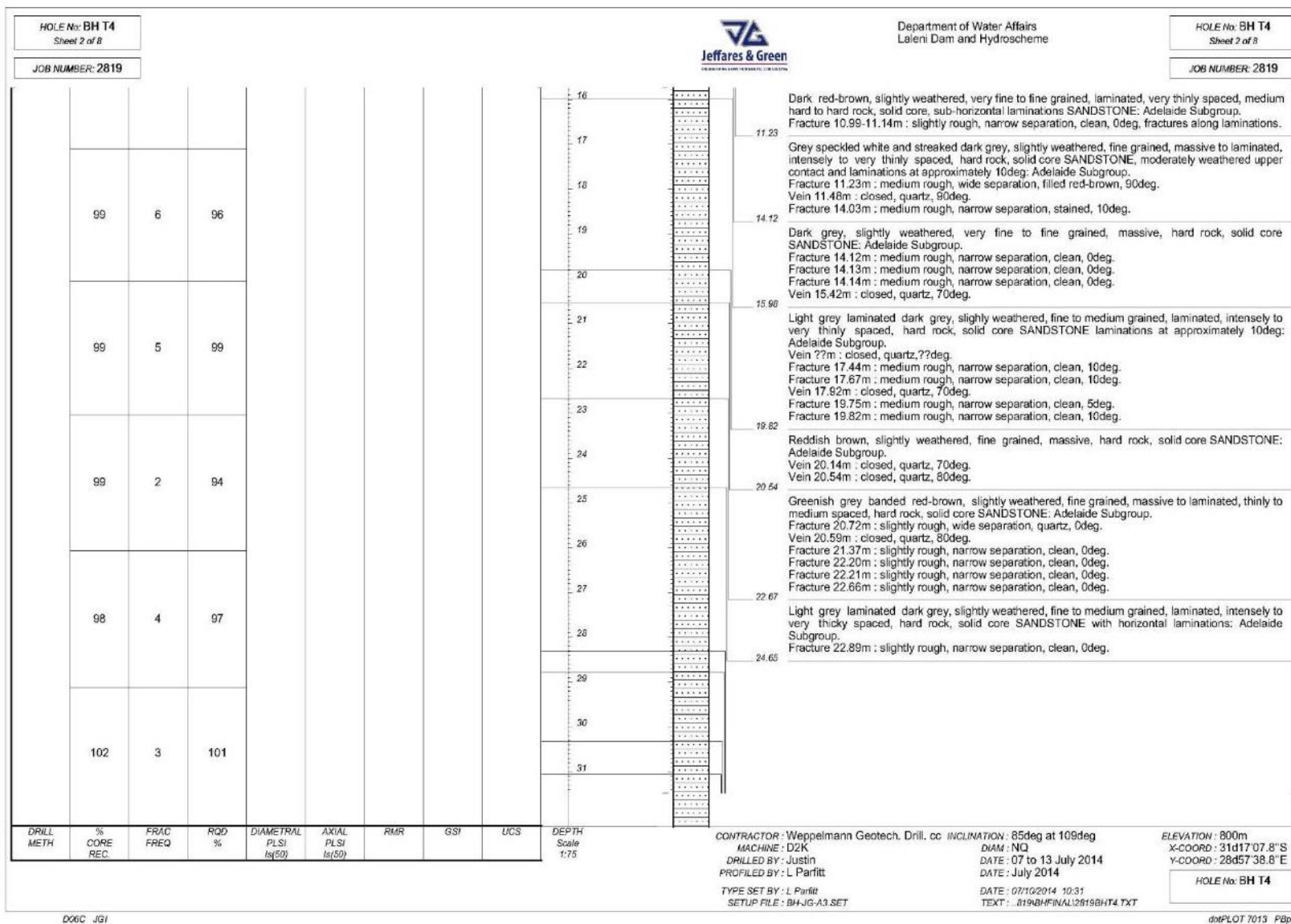


Fig D-7.2: Borehole Tunnel 4 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

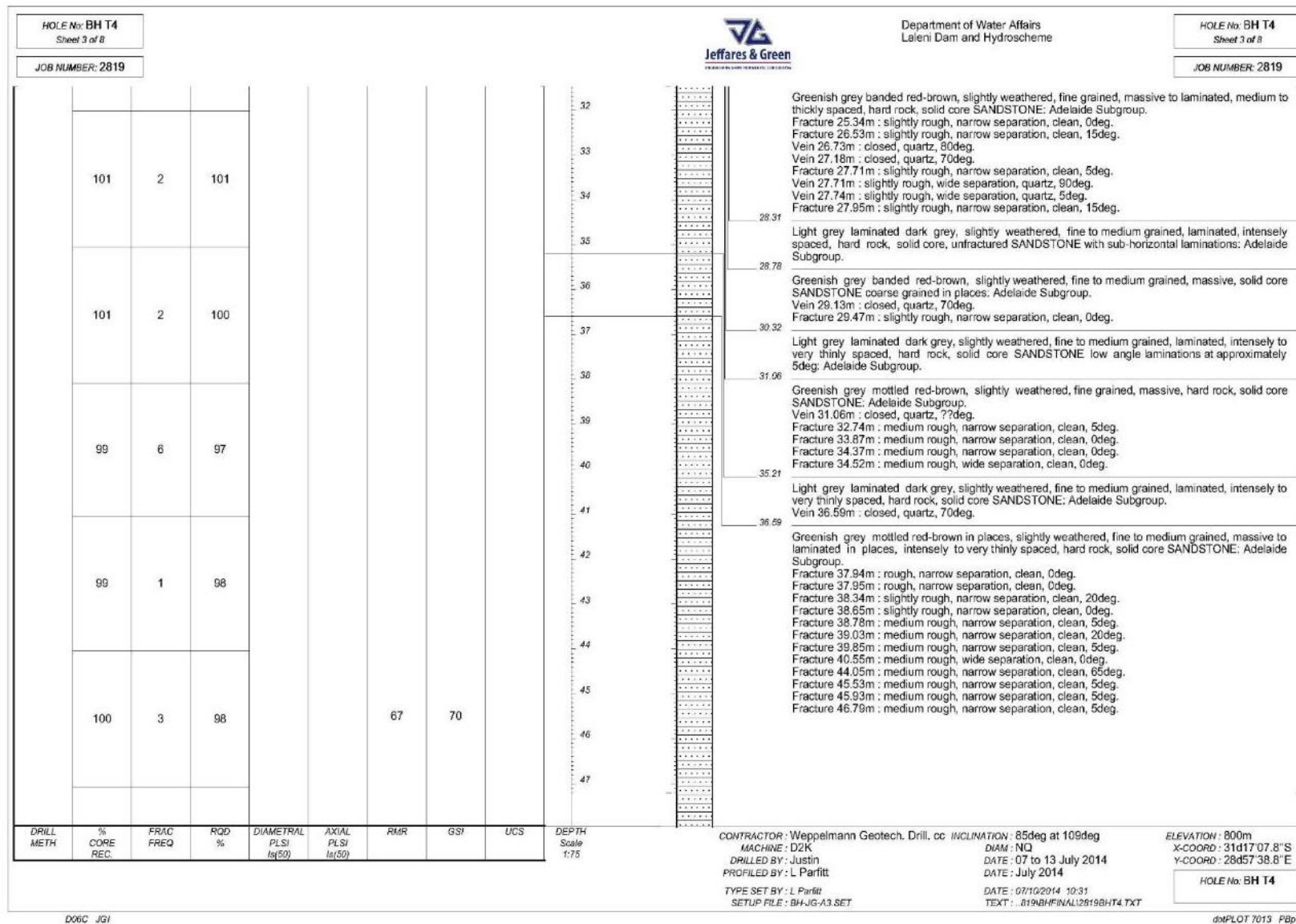


Fig D-7.3: Borehole Tunnel 4 – Log

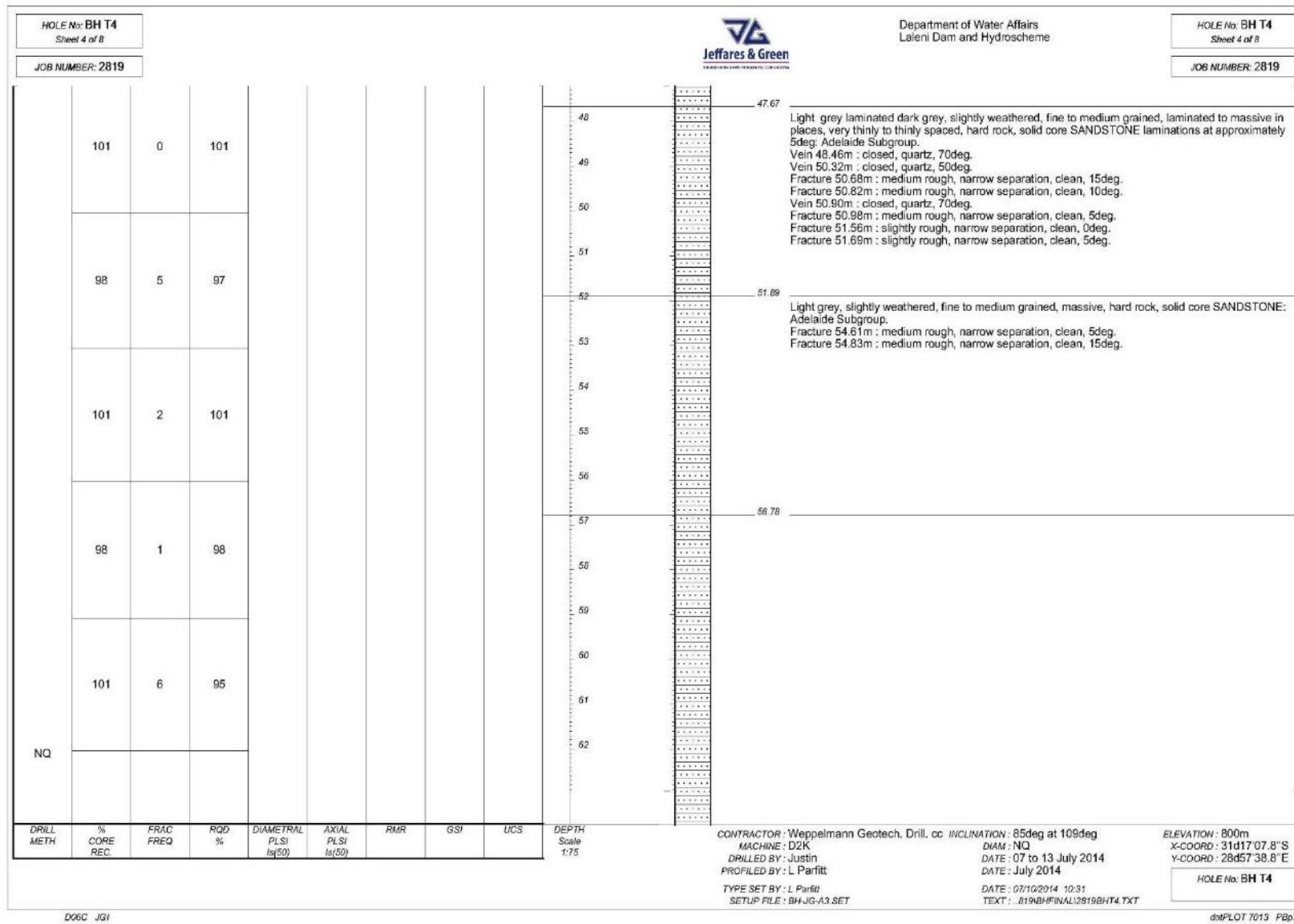


Fig D-7.4: Borehole Tunnel 4 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

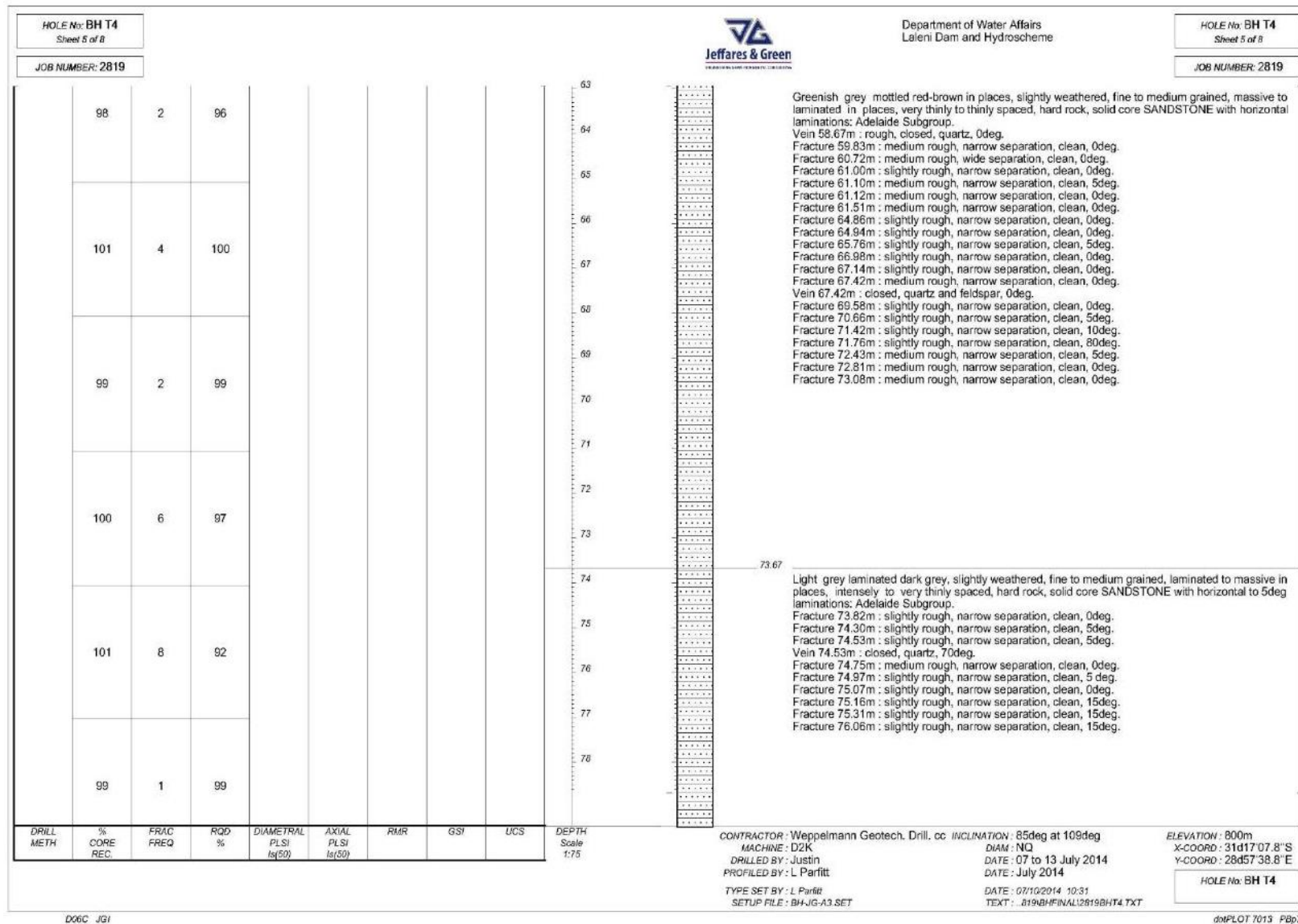


Fig D-7.5: Borehole Tunnel 4 – Log

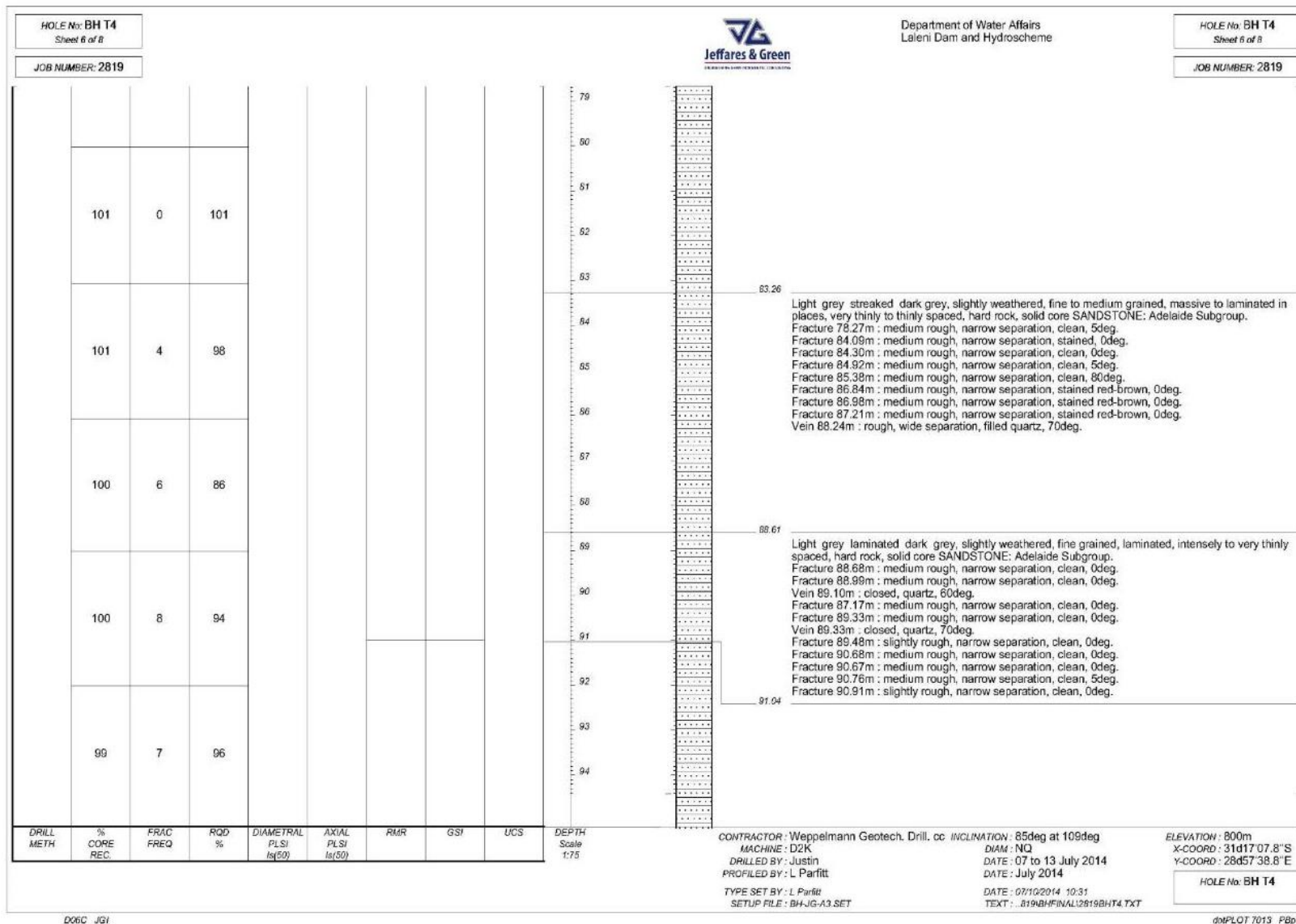


Fig D-7.6: Borehole Tunnel 4 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

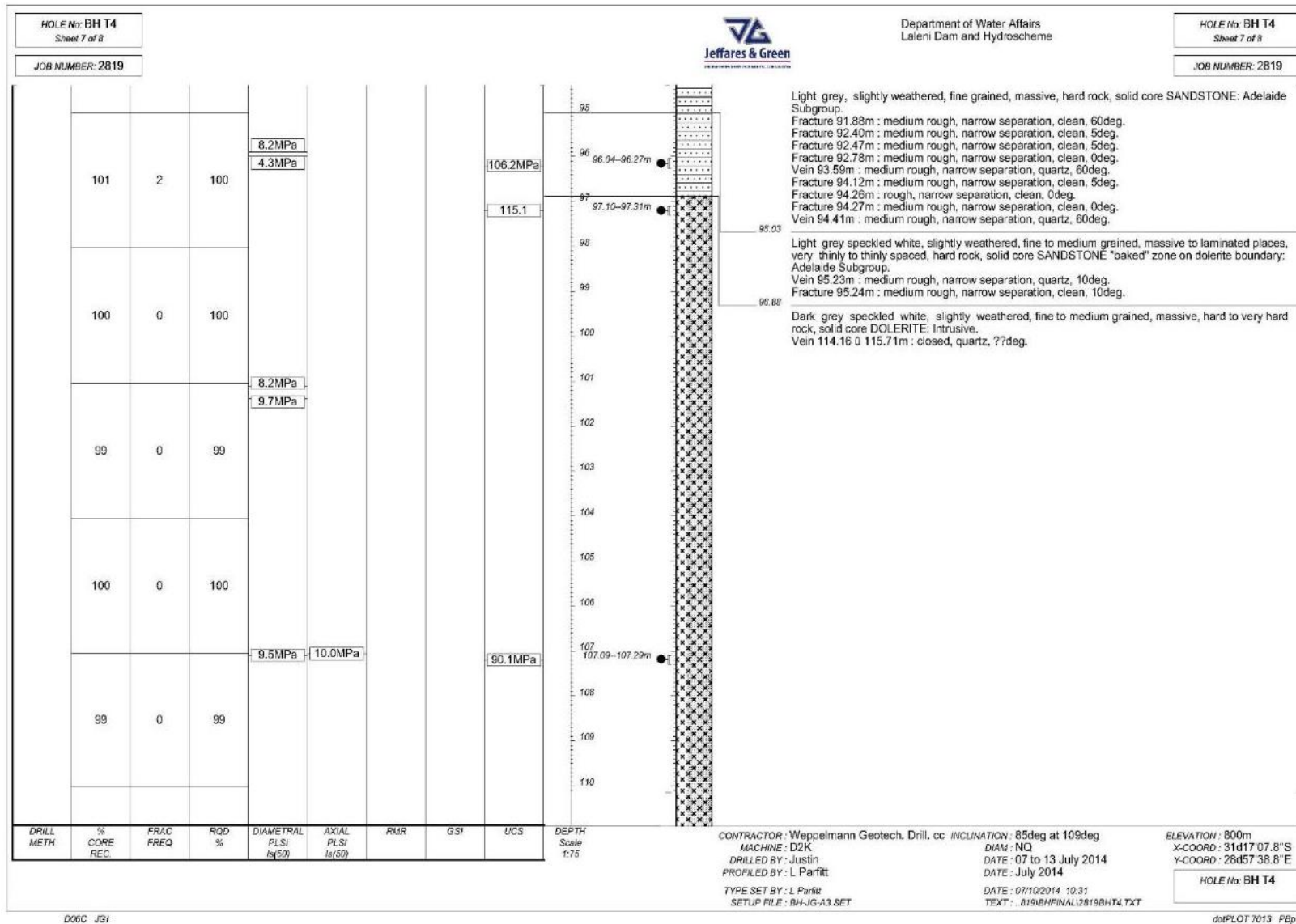


Fig D-7.7: Borehole Tunnel 4 – Log

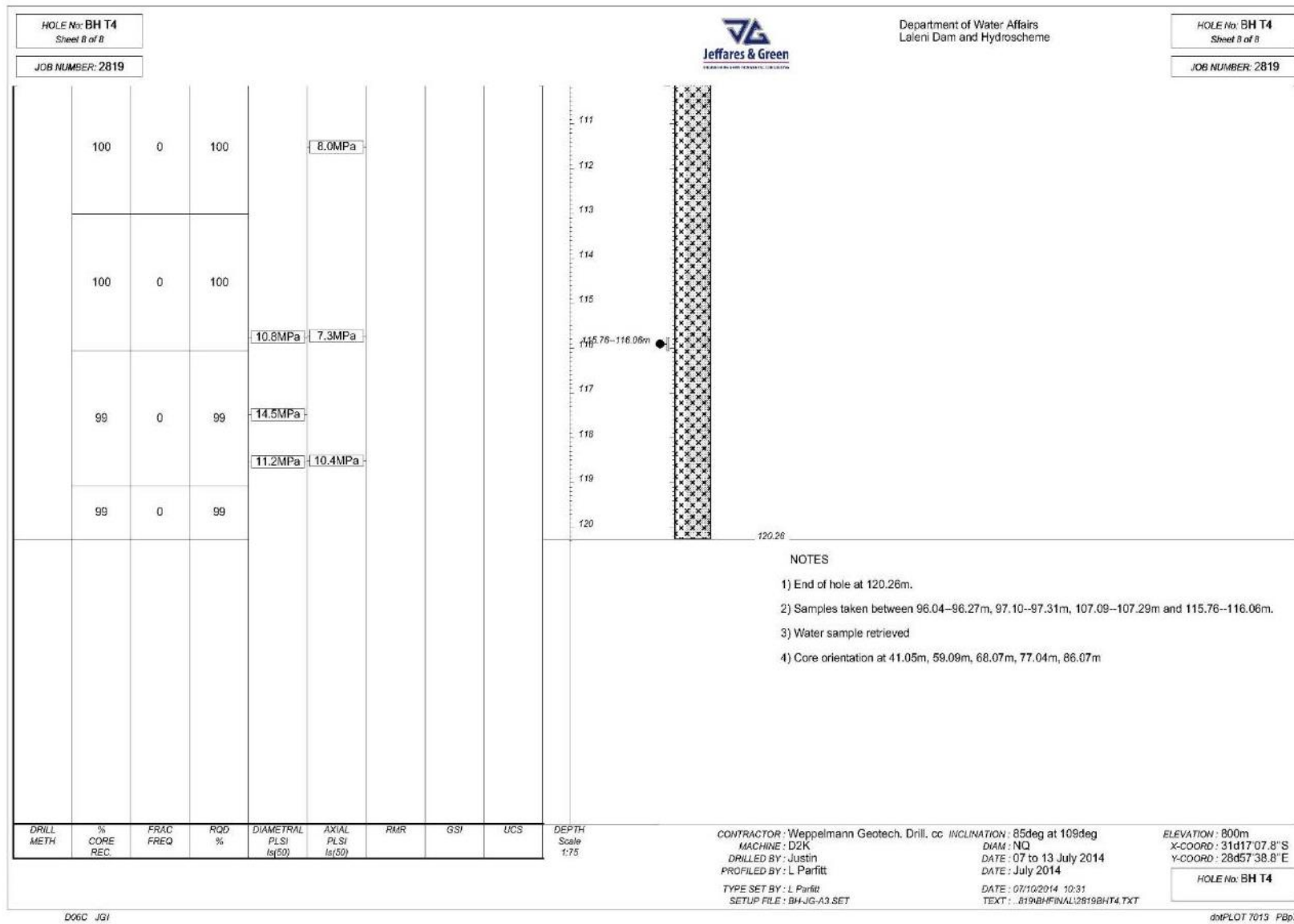


Fig D-7.1: Borehole Tunnel 4 – Log

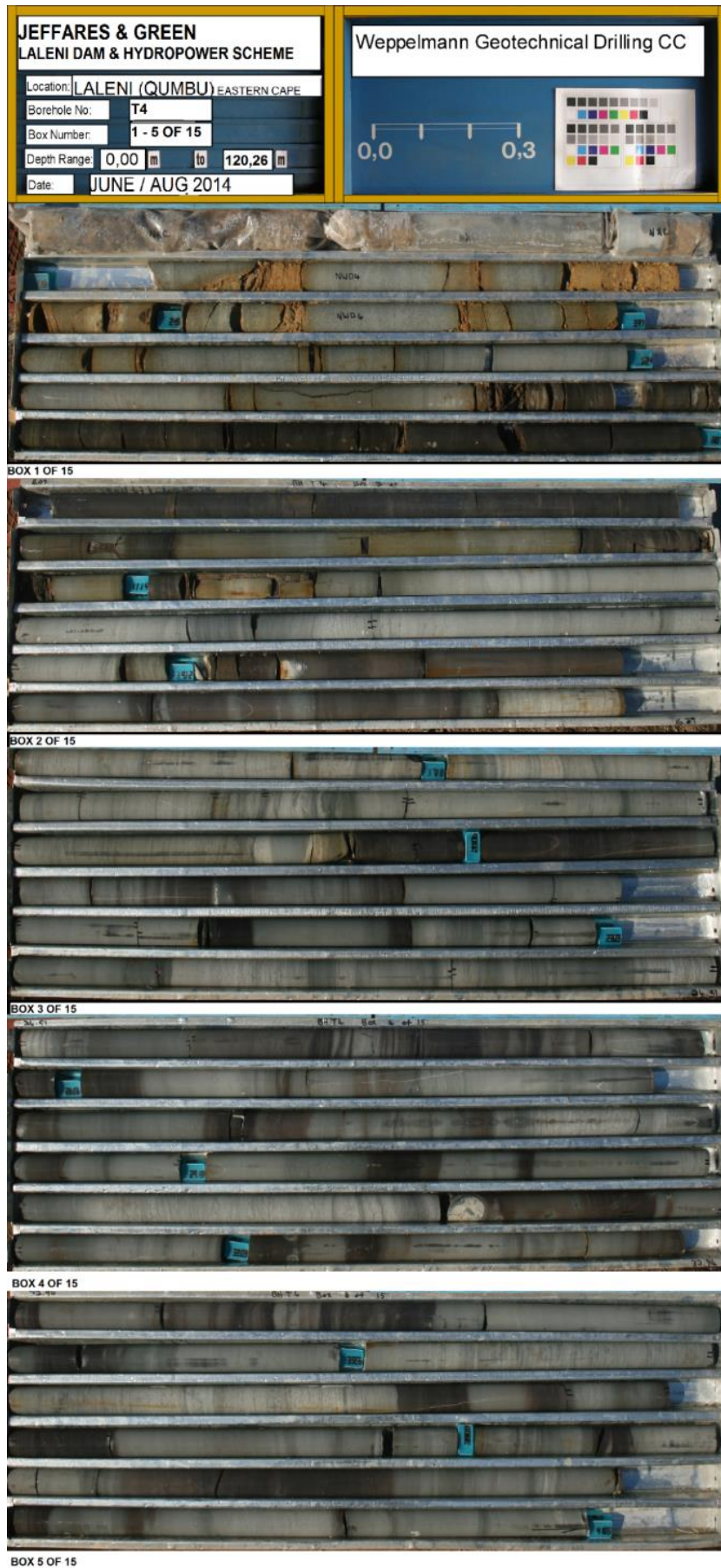


Fig D-8: Borehole Tunnel 4 – Box 1 to 5

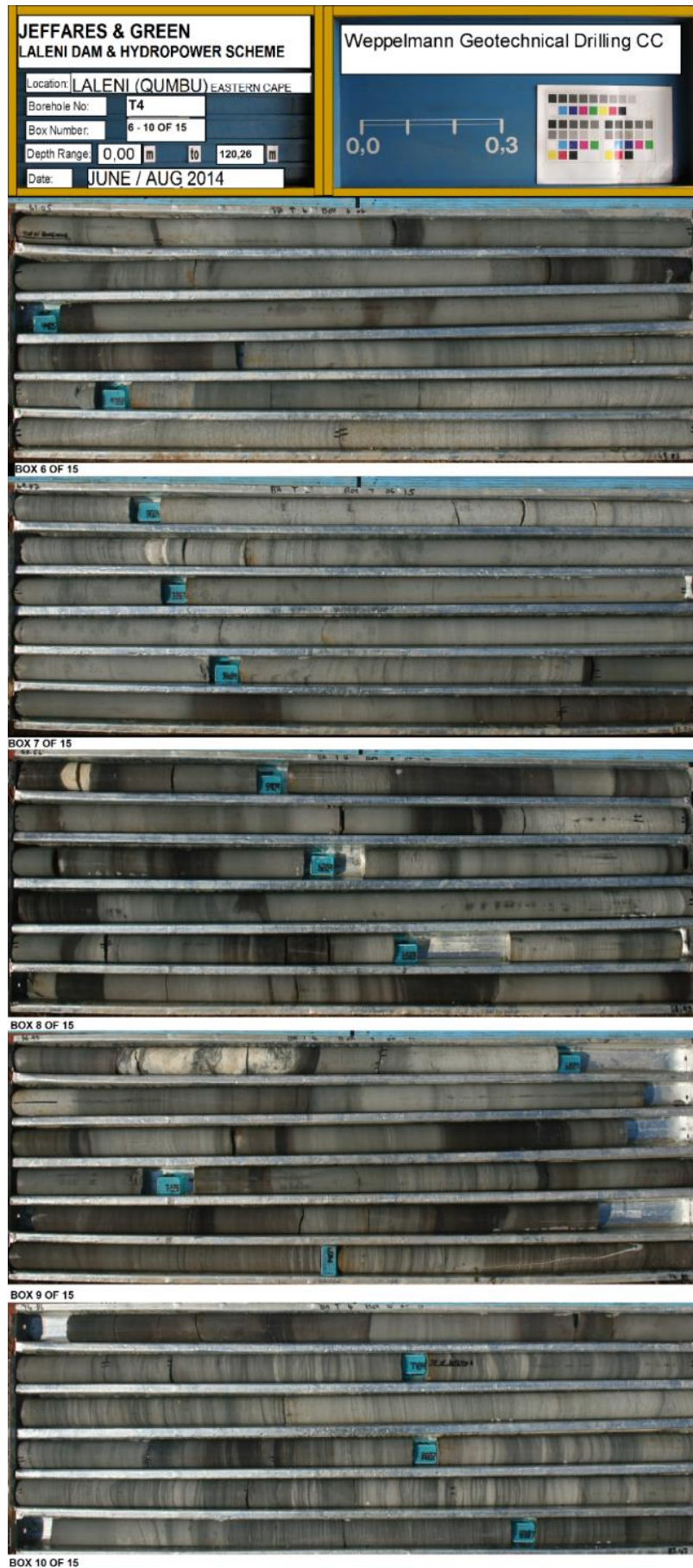


Fig D-8: Borehole Tunnel 4 – Box 6 to 10

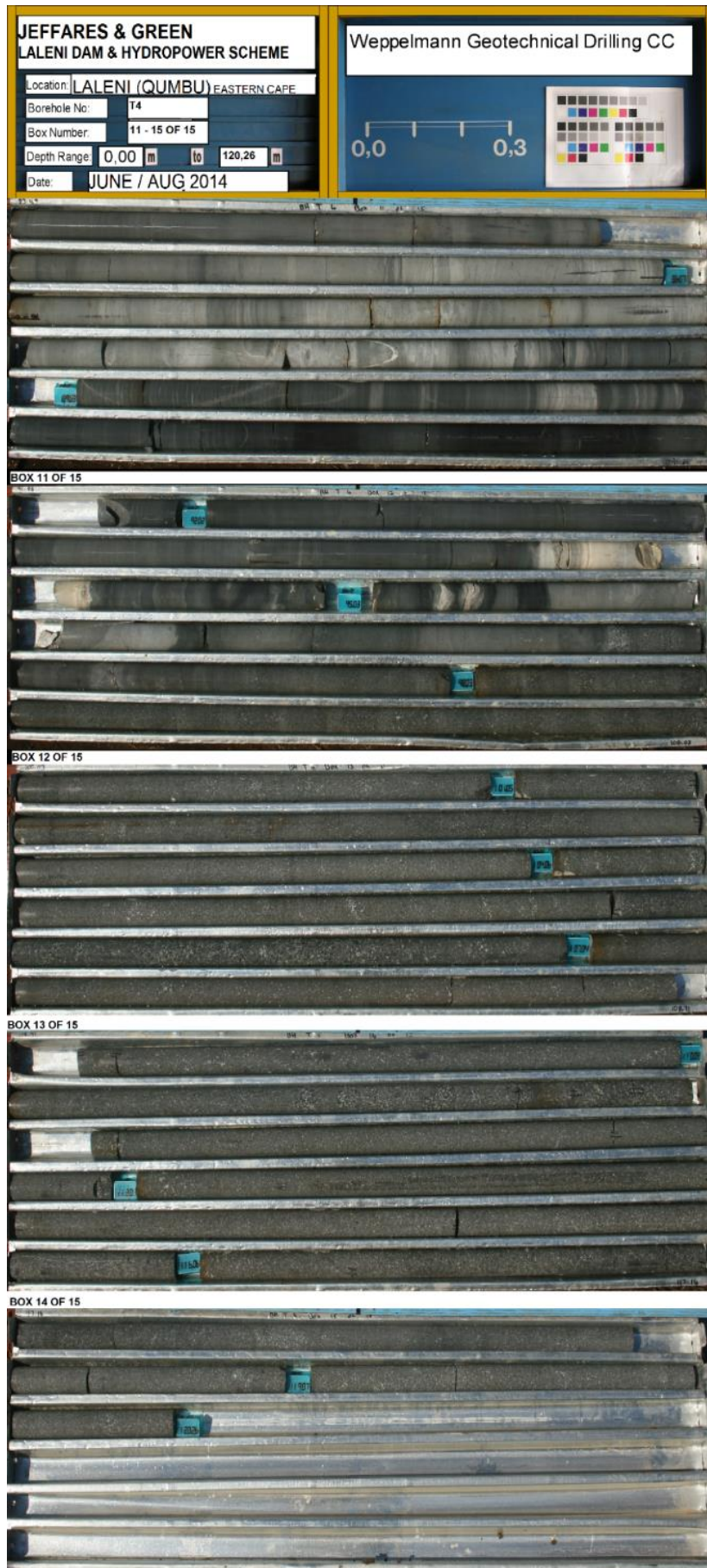


Fig D-8: Borehole Tunnel 4 – Box 11 to 15

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

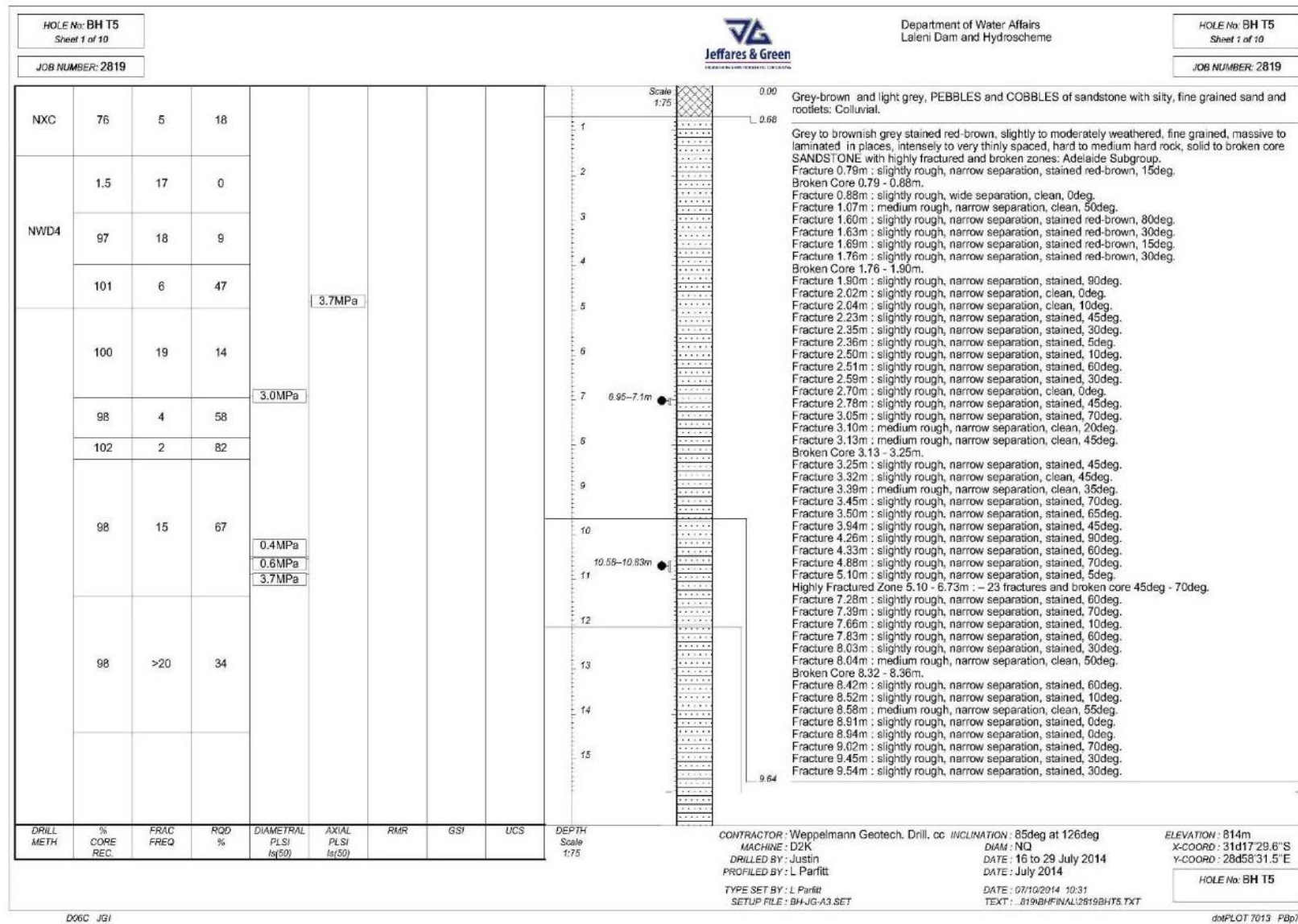


Fig D-9.1: Borehole Tunnel 5 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

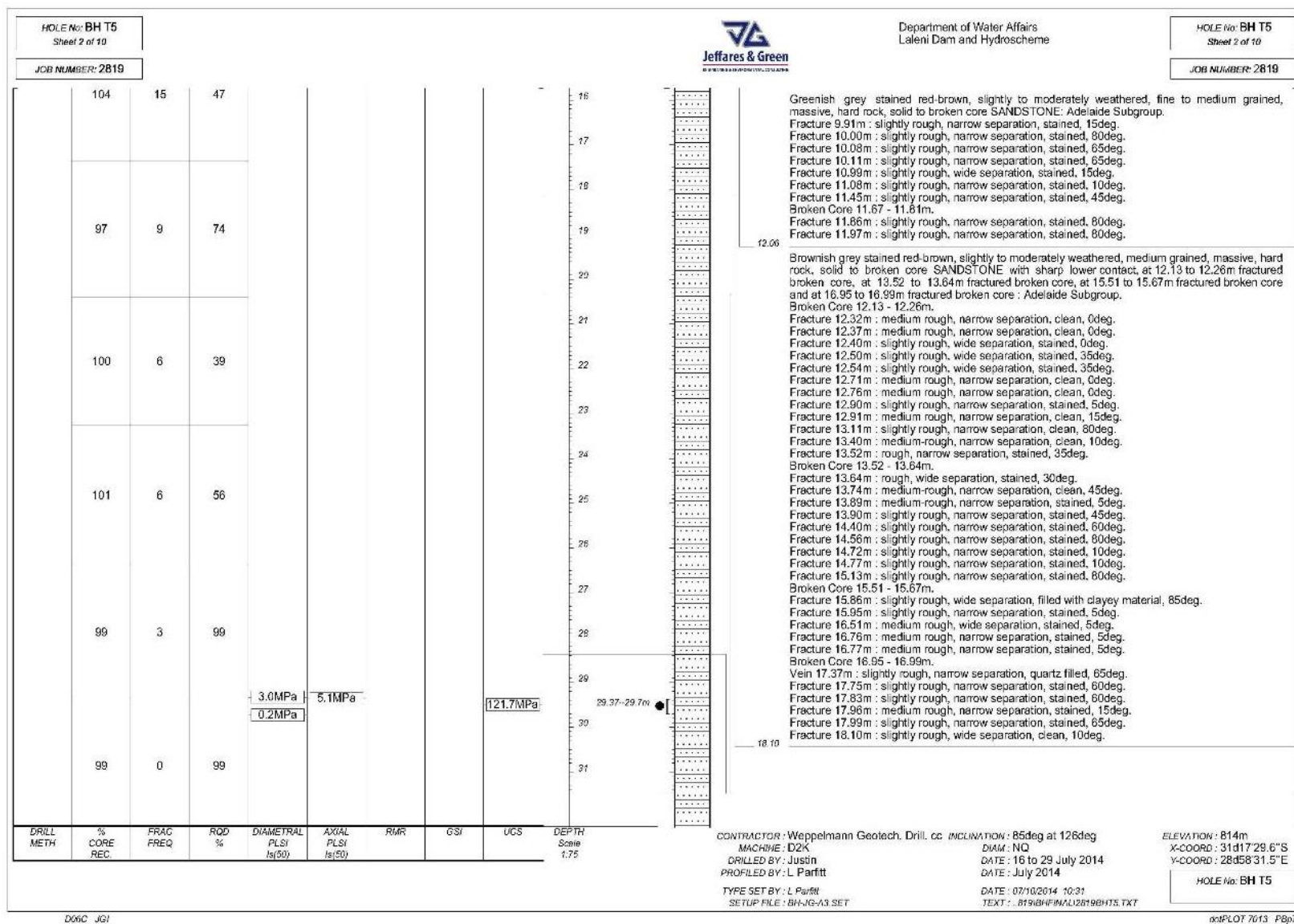


Fig D-9.2: Borehole Tunnel 5 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

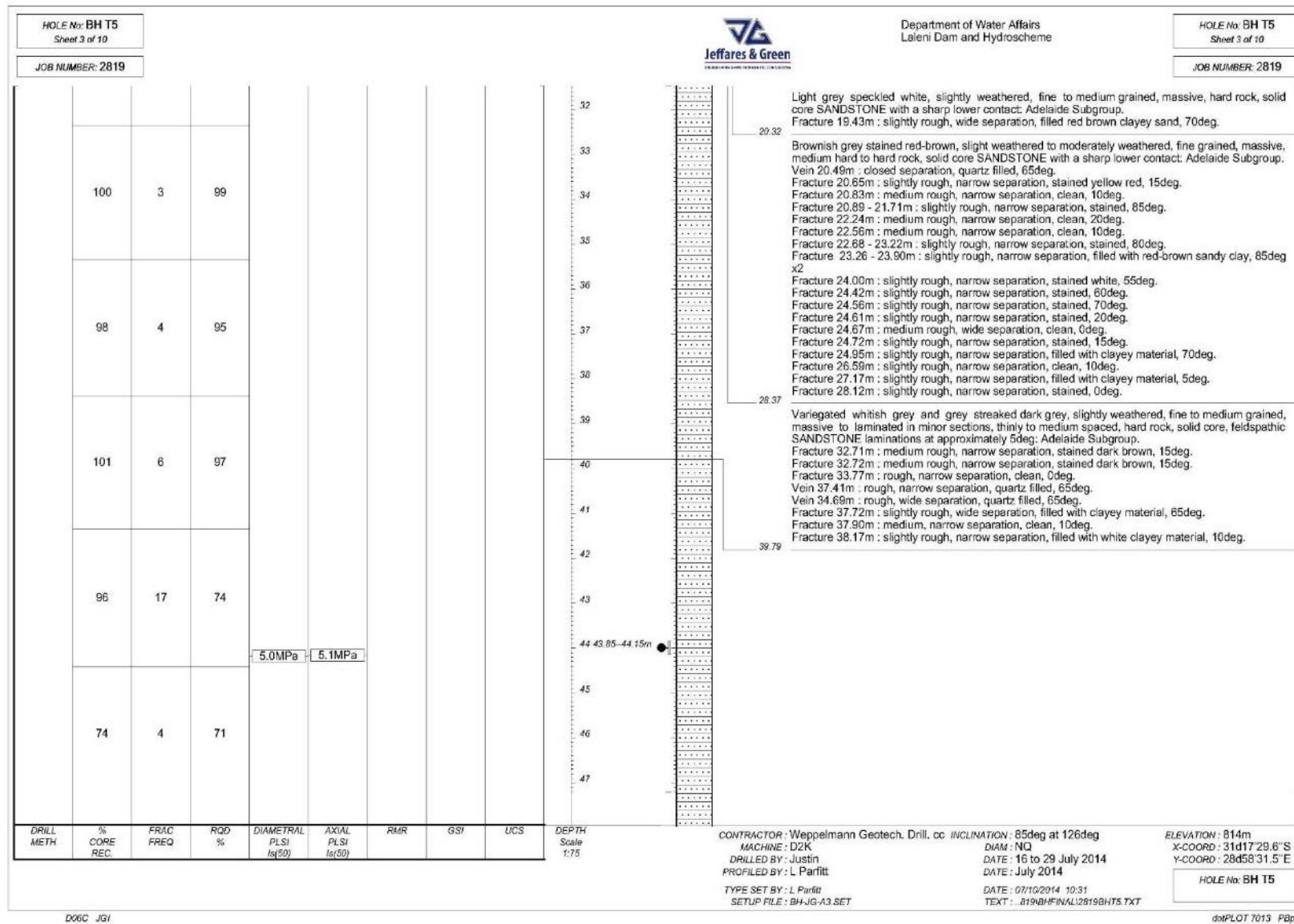


Fig D-9.3: Borehole Tunnel 5 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

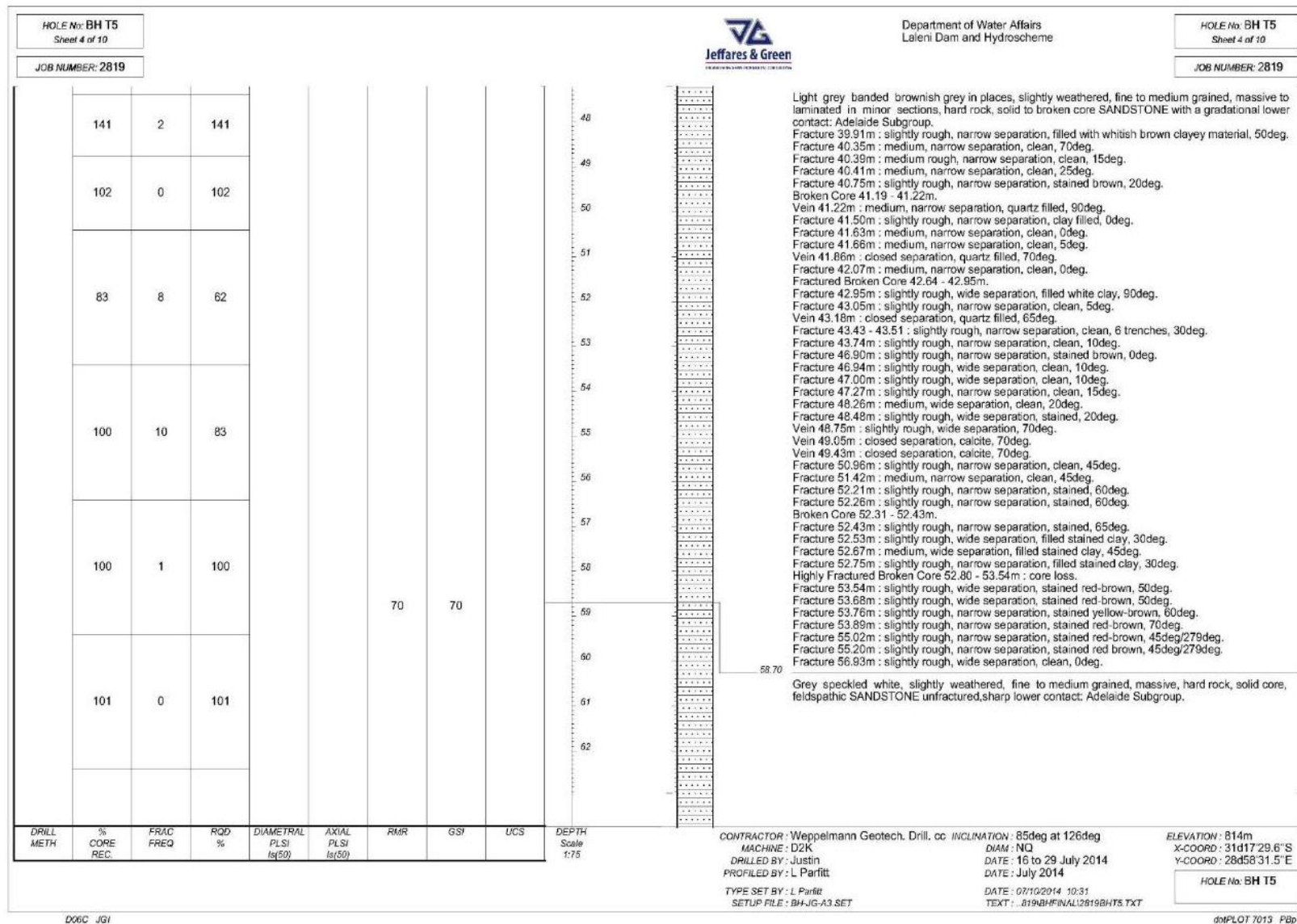


Fig D-9.4: Borehole Tunnel 5 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

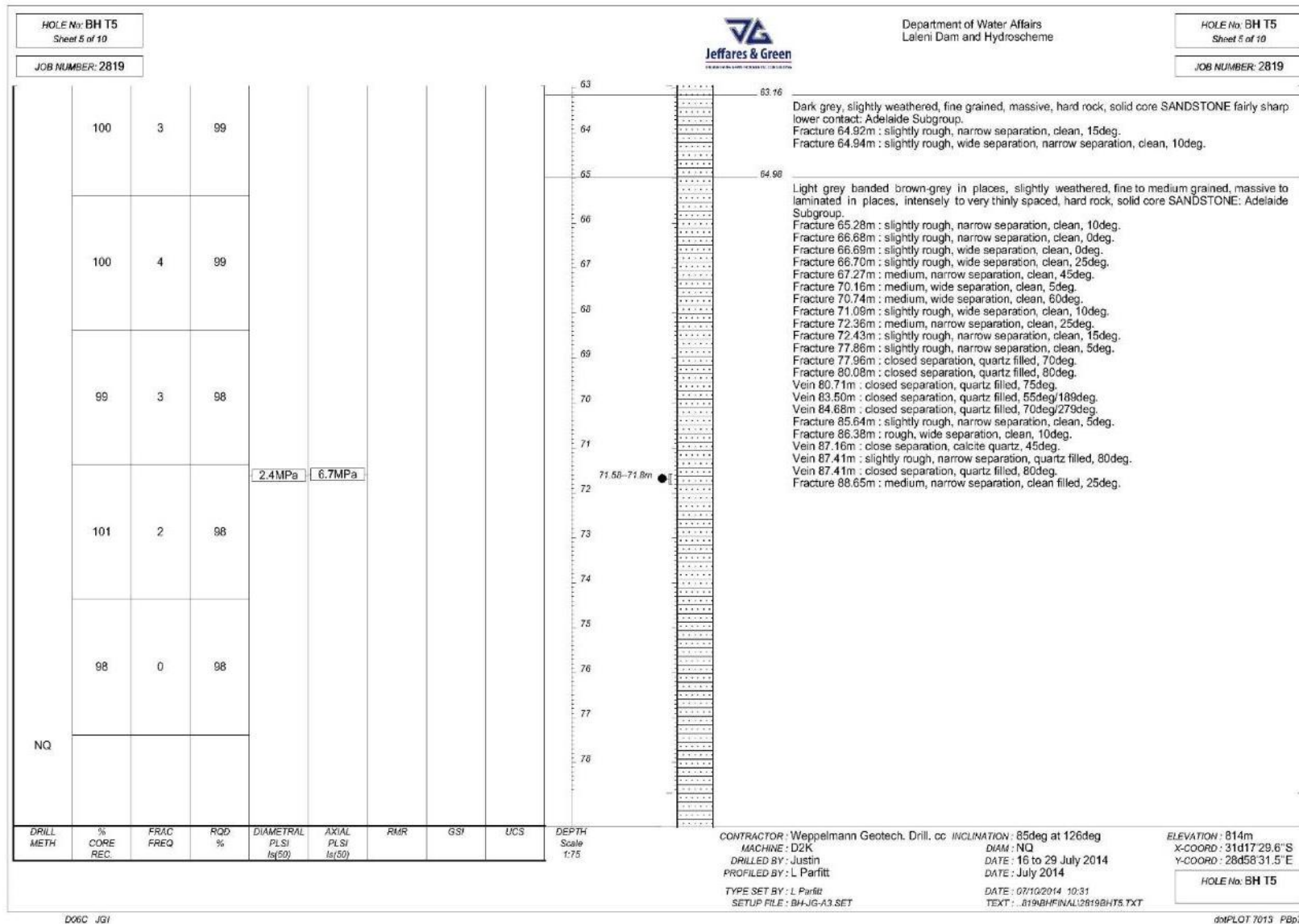


Fig D-9.5: Borehole Tunnel 5 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

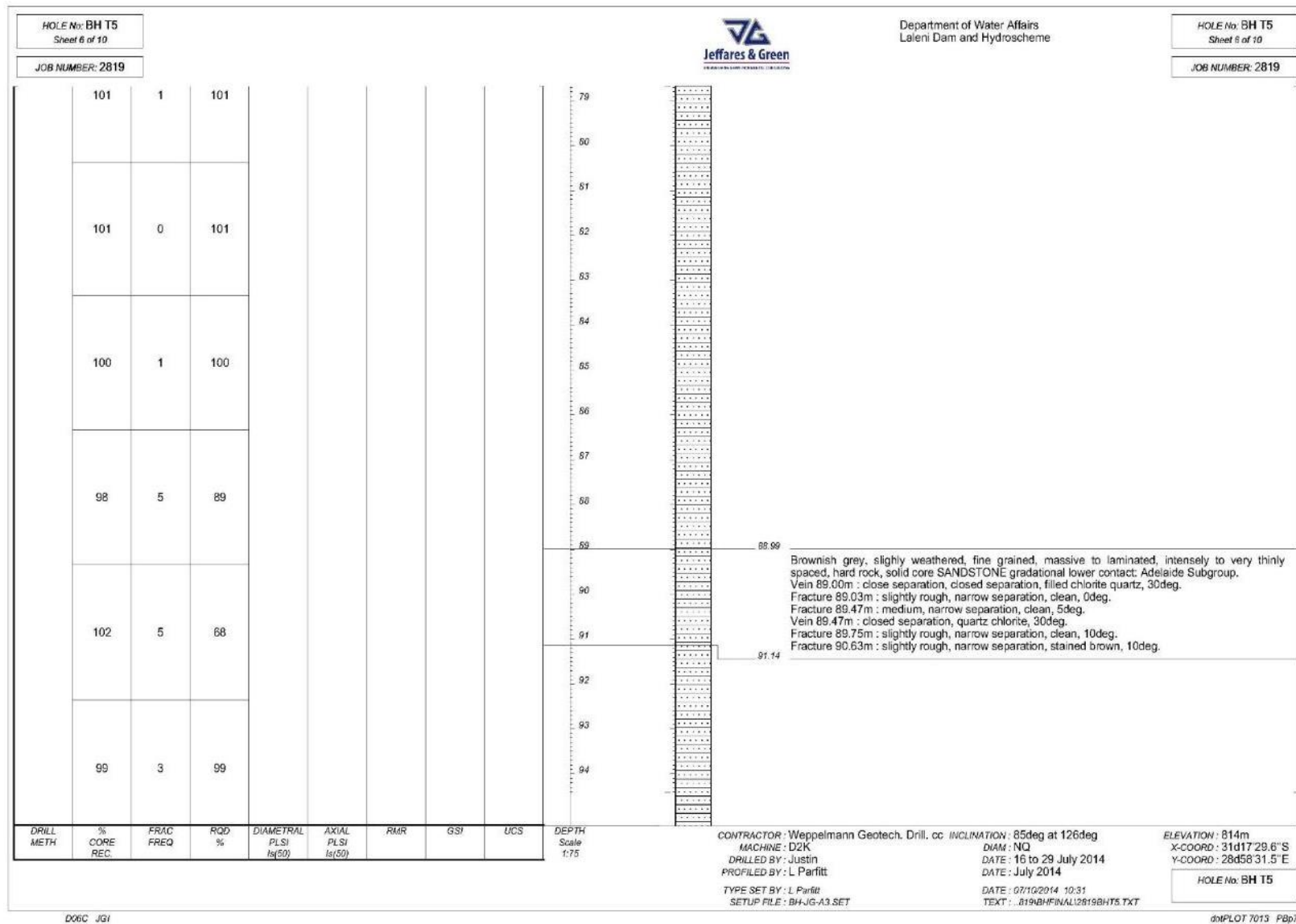


Fig D-9.6: Borehole Tunnel 5 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

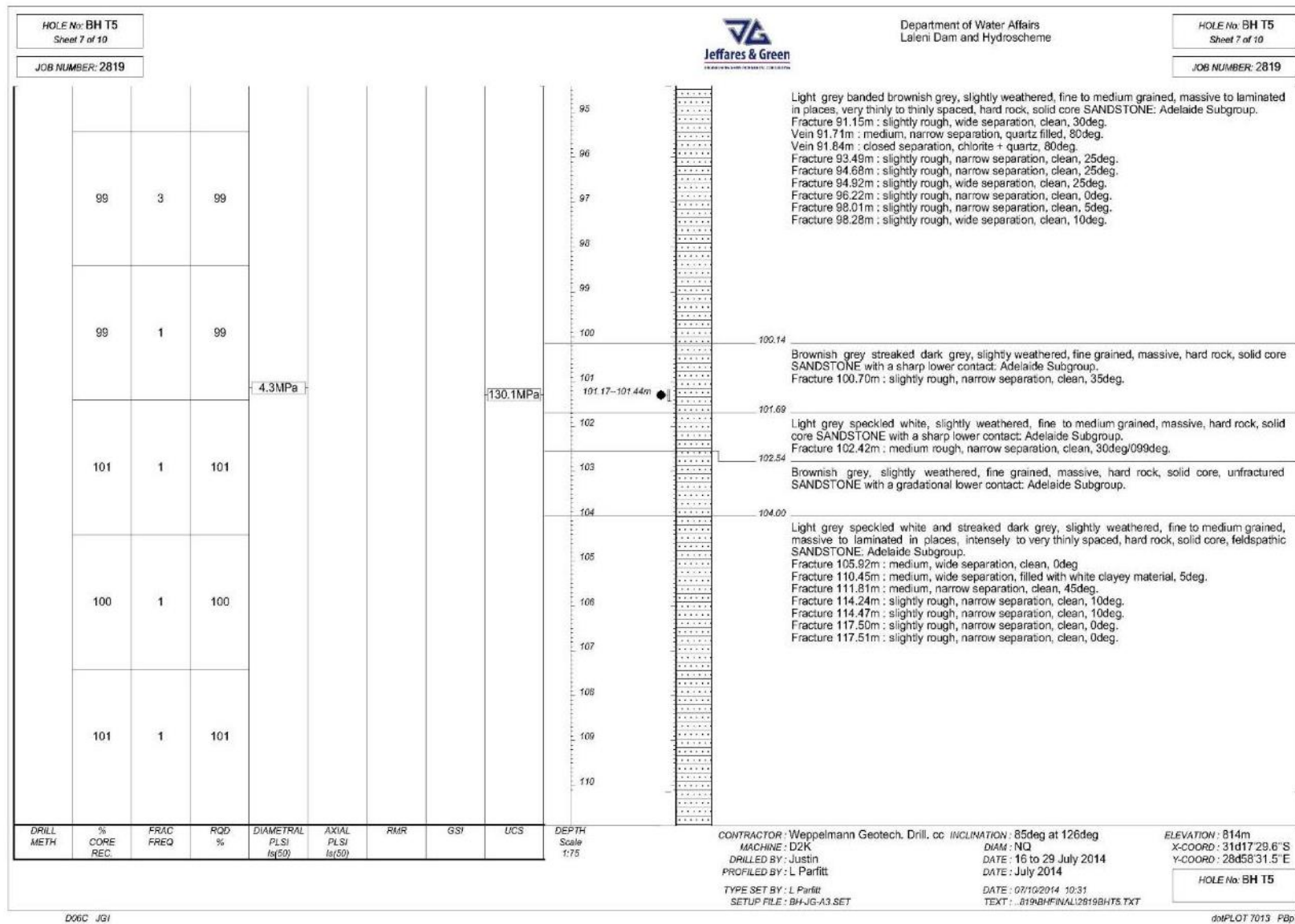


Fig D-9.7: Borehole Tunnel 5 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

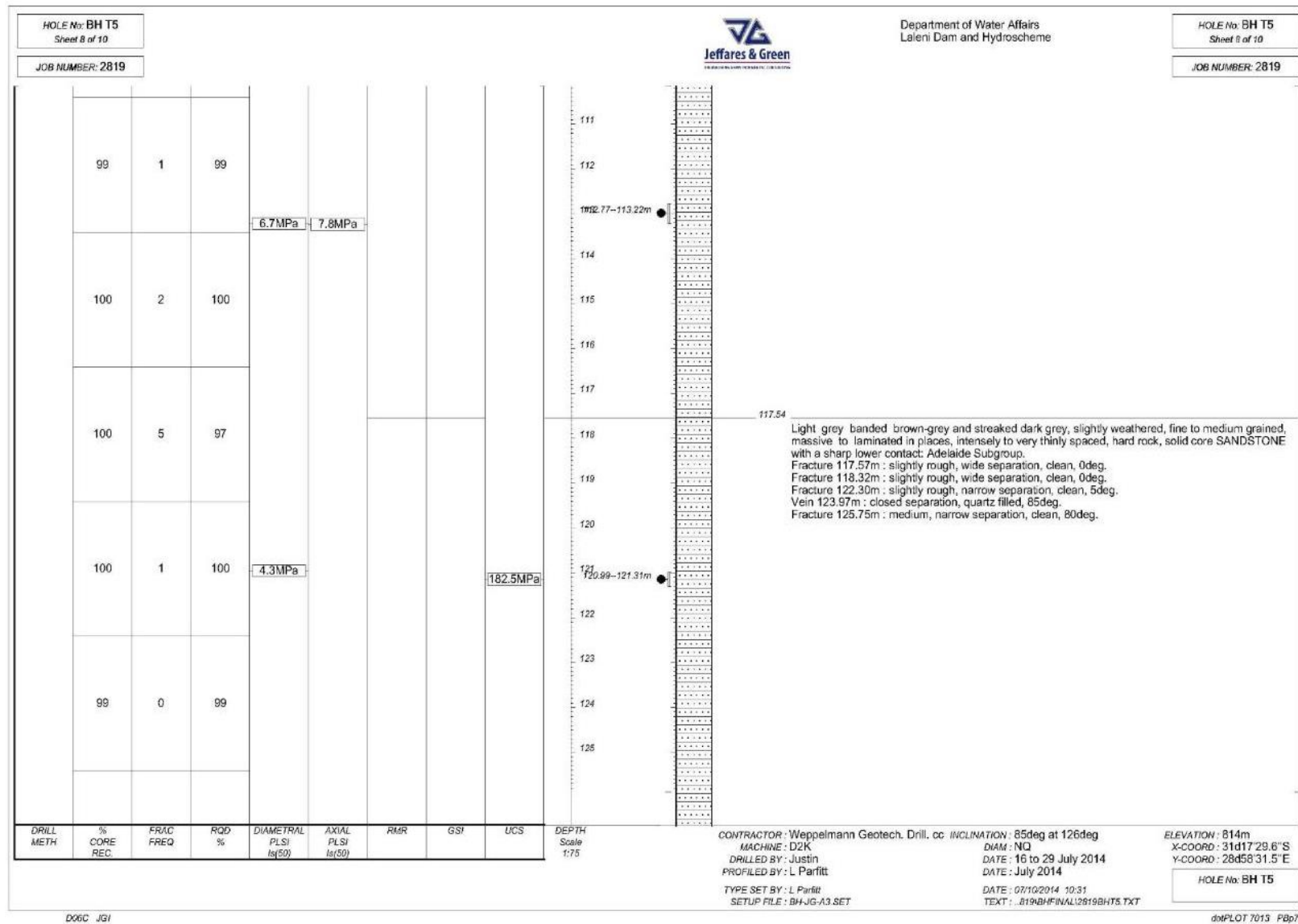


Fig D-9.8: Borehole Tunnel 5 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

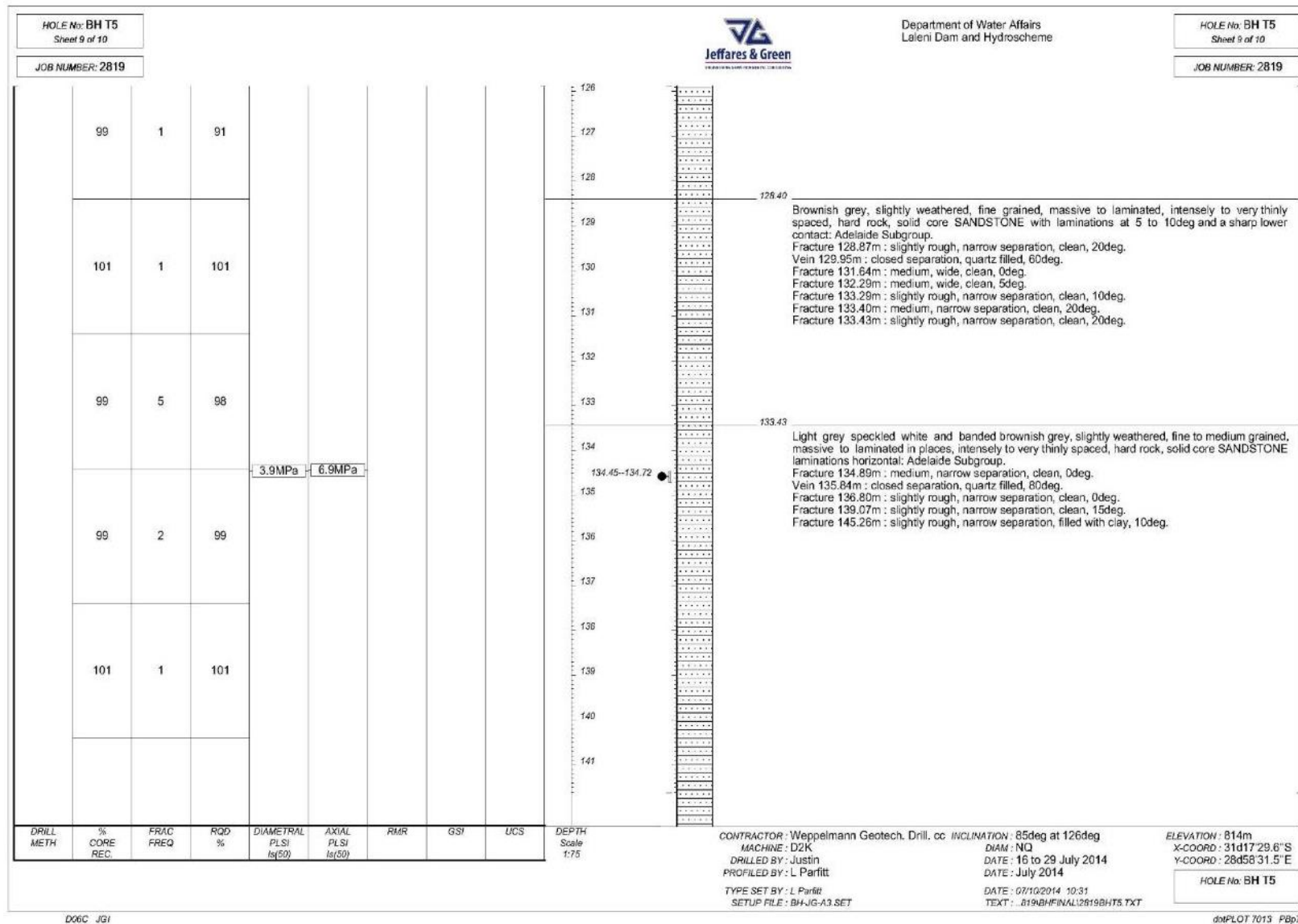


Fig D-9.9: Borehole Tunnel 5 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

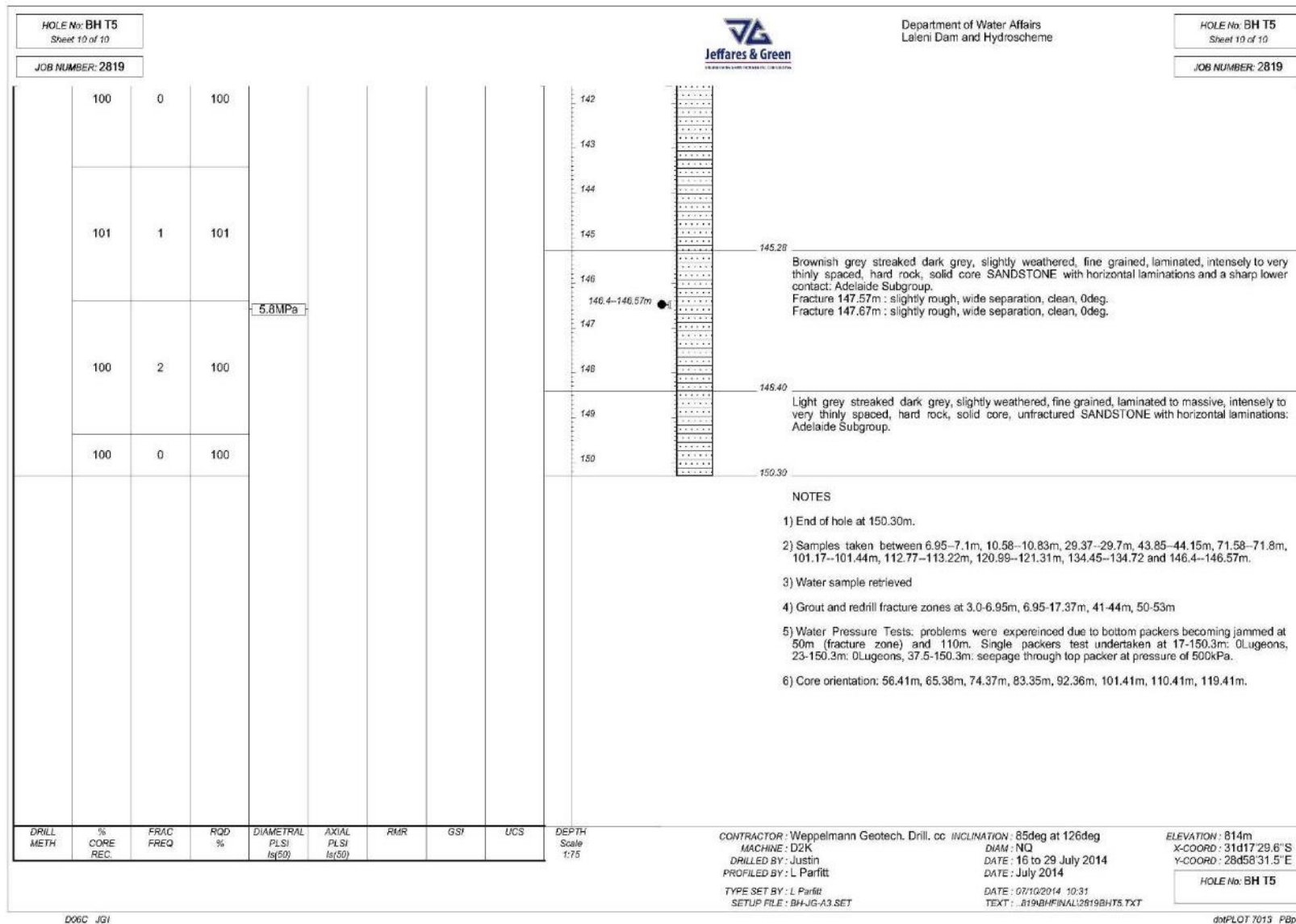


Fig D-9.10: Borehole Tunnel 5 – Log

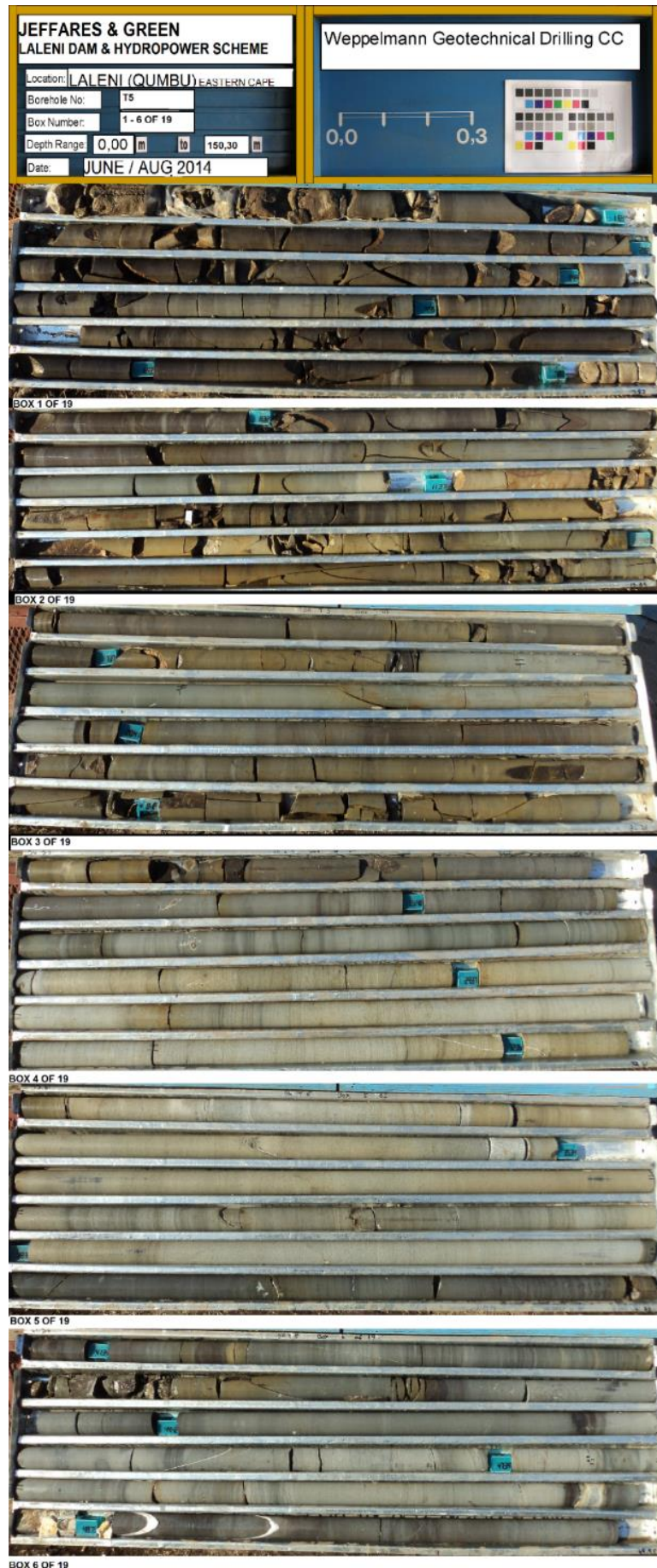


Fig D-11.1: Borehole Tunnel 5 – Box 1 to 6

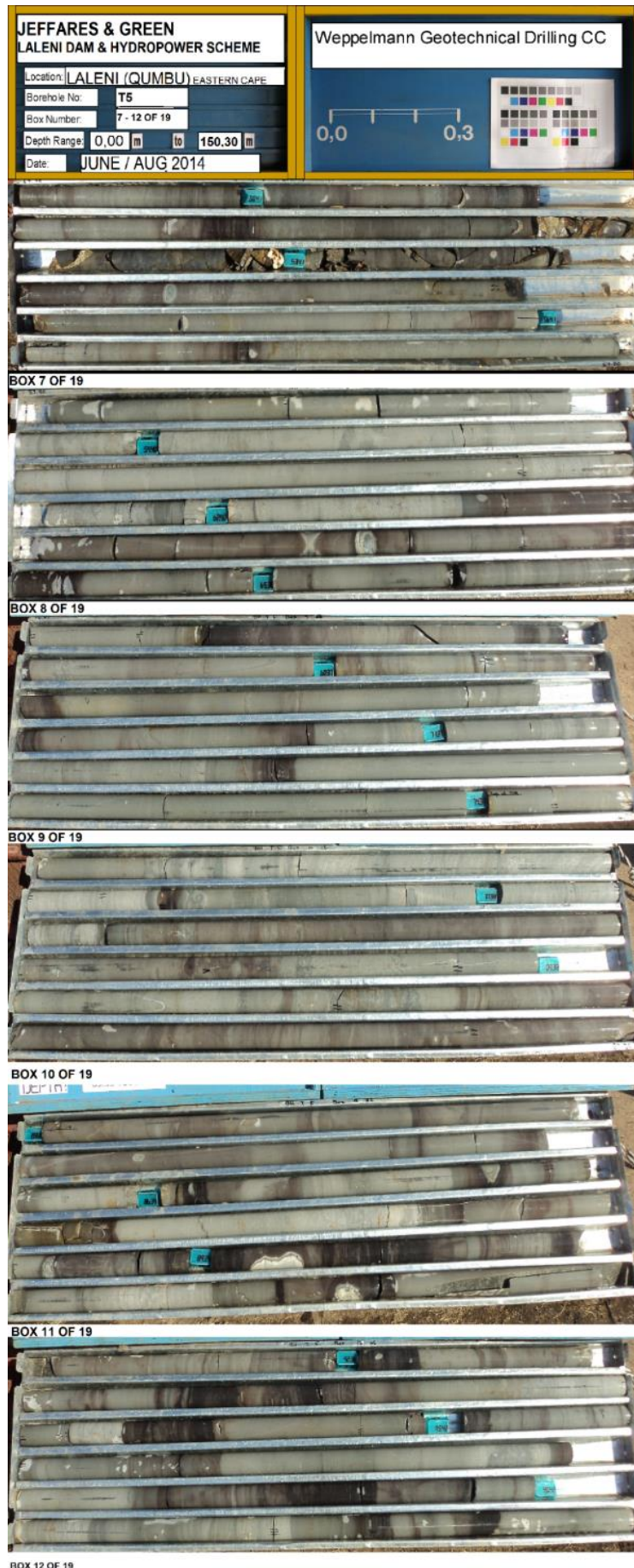


Fig D-11.2: Borehole Tunnel 5 – Box 7 to 12

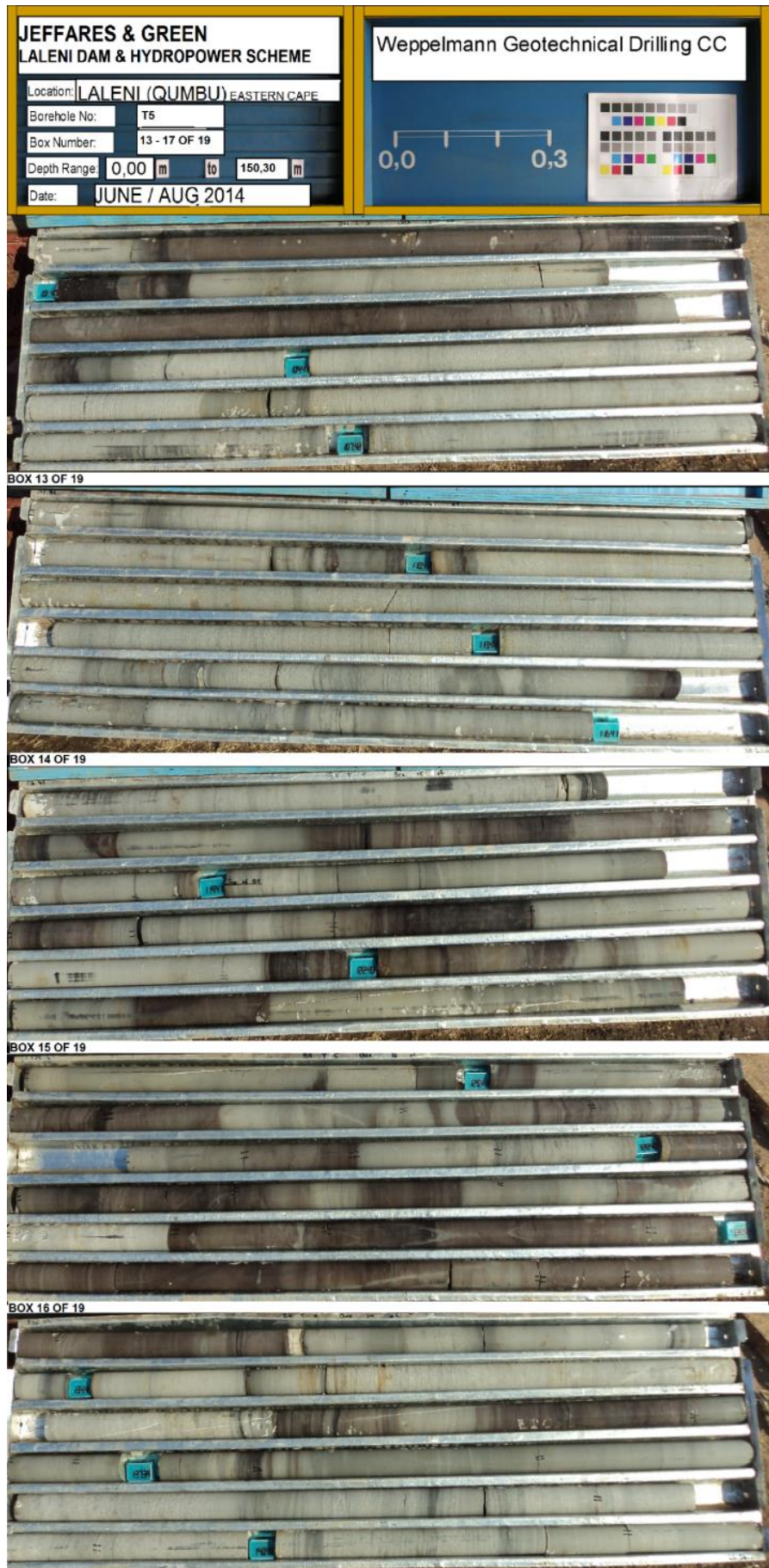


Fig D-11.3: Borehole Tunnel 5 – Box 13 to 17

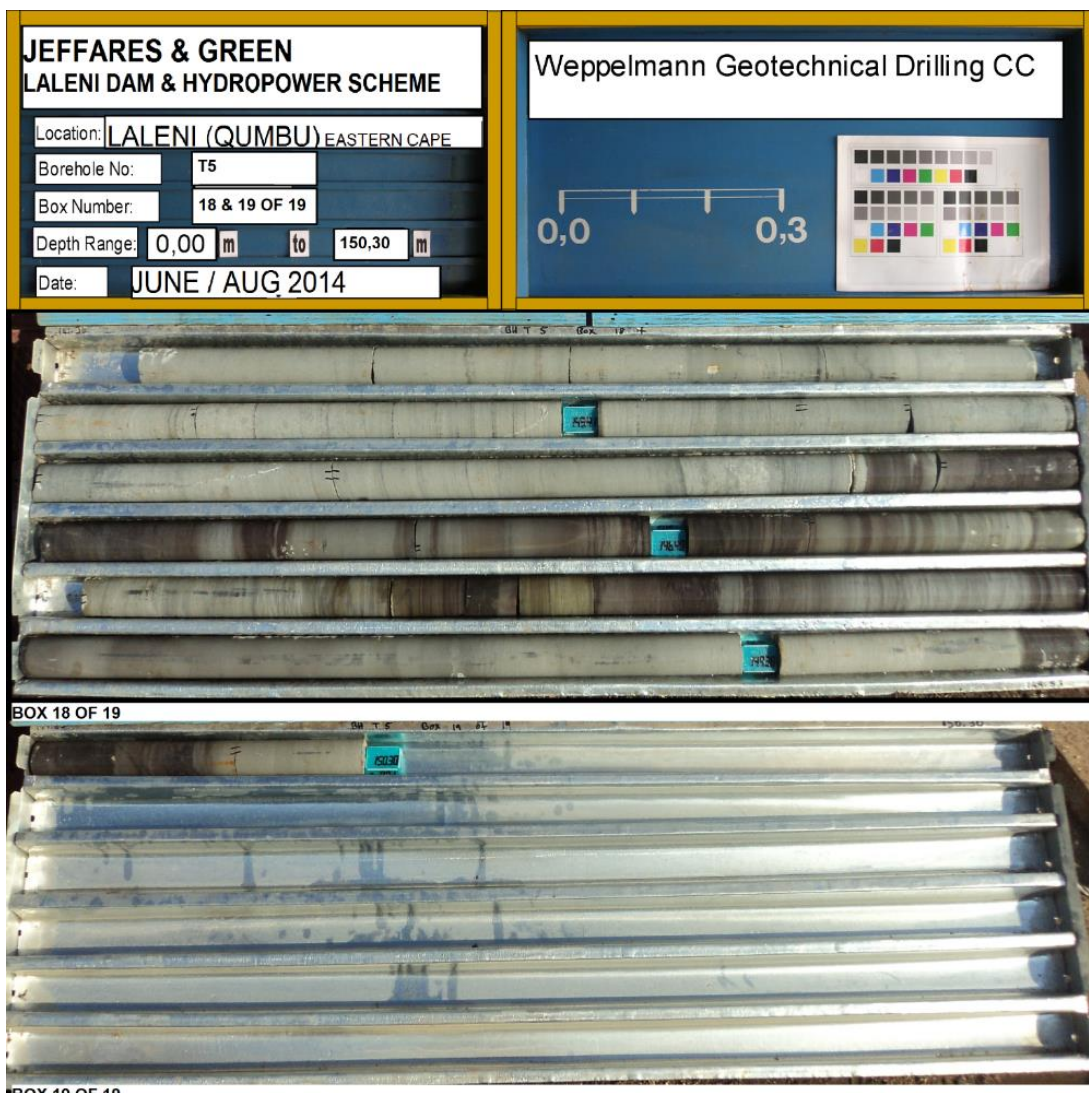


Fig D-11.4: Borehole Tunnel 5 – Box 18 to 19

Guage Pressure (Bars) A	Pressure due to Guage Height (Bars) B	Pressure due to water height above water table (Bars) C	Test Pressure (Bars) A+B+C	Duration (Seconds)	Injected Volume (l)	Top Level	Bottom Level	Length of hole	time in minutes	litres / metre /minute	Lugeon Units
WPT 17 – 150.3m											
1.55	0	0	1.55	600	103.9	17	150.3	133.3	10	0.0779	1
2.7	0	0	2.7	600	136.3	17	150.3	133.3	10	0.1023	0
3.85	0	0	3.85	600	142.4	17	150.3	133.3	10	0.1068	0
2.7	0	0	2.7	600	122.2	17	150.3	133.3	10	0.0917	0
1.33	0	0	1.33	600	110.4	17	150.3	133.3	10	0.0828	1
WPT 23 – 150.3m											
2.1	0	0	2.1	600	123.8	23	150.3	127.3	10	0.0973	0
3.65	0	0	3.65	600	181.6	23	150.3	127.3	10	0.1427	0
4	0	0	4	300	196.1	23	150.3	127.3	10	0.154	0
3.65	0	0	3.65	600	168	23	150.3	127.3	10	0.132	0
2.1	0	0	2.1	600	116.5	23	150.3	127.3	10	0.0915	0
WPT 37.5 – 150.3m											
3.4	0	0	3.4	600	141.6	37.5	150.3	112.8	10	0.1255	0
5	0	0	5	600	194	37.5	150.3	112.8	10	0.172	0
5	0	0	5	300	110	37.5	150.3	112.8	10	0.0975	0
	0	0	0			37.5	150.3	112.8	10	0	0
3.4	0	0	3.4	600	150.9	37.5	150.3	112.8	10	0.1338	0
Mzimvubu	Input Data			Depth from top of embankment to water table (m)					0		
Borehole No	T5				Height of guage above top of embankment (m)					0	

Table D-1: Water Pressure Tests – Borehole T5

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

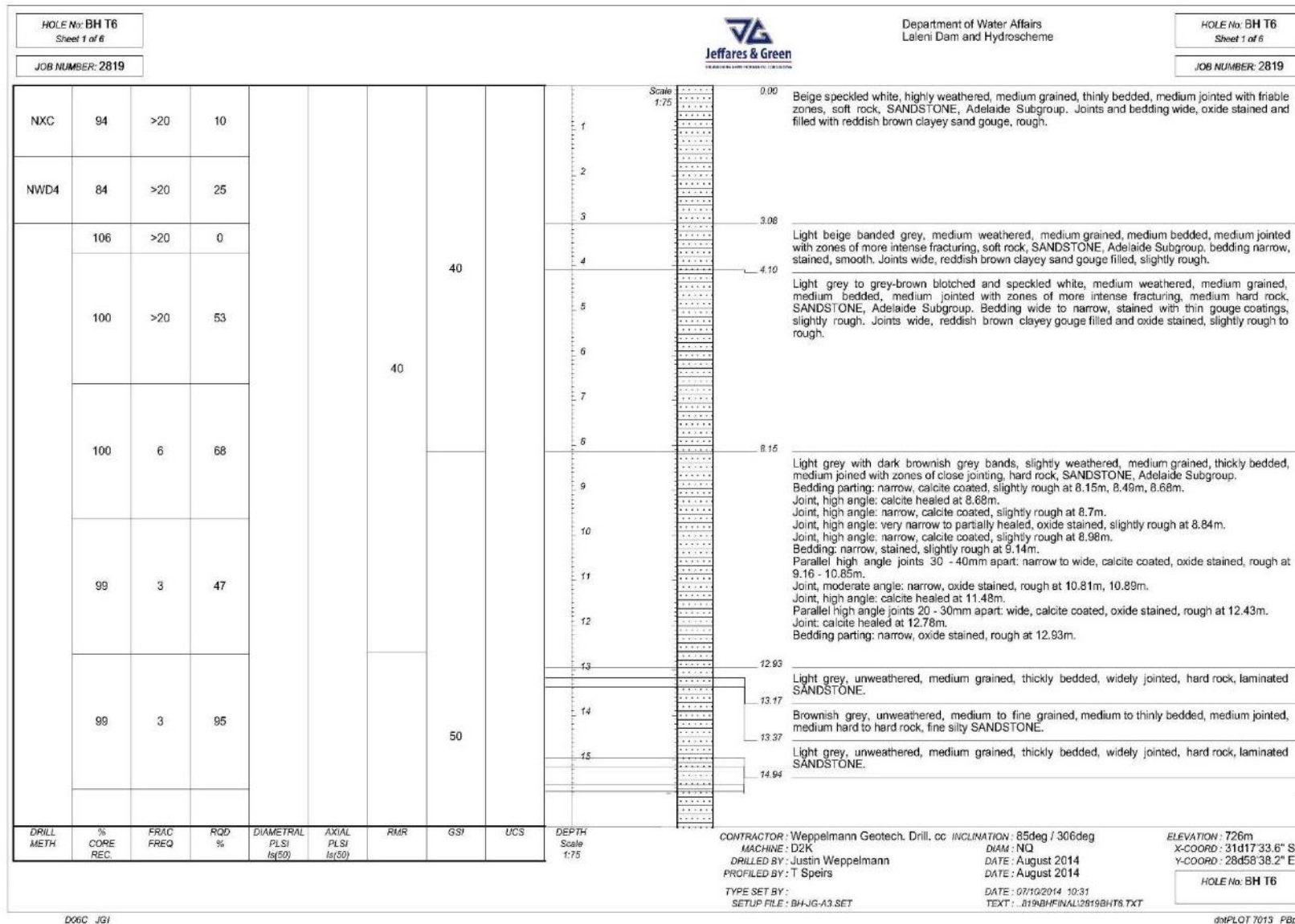


Fig D-12.1: Borehole Tunnel 6 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

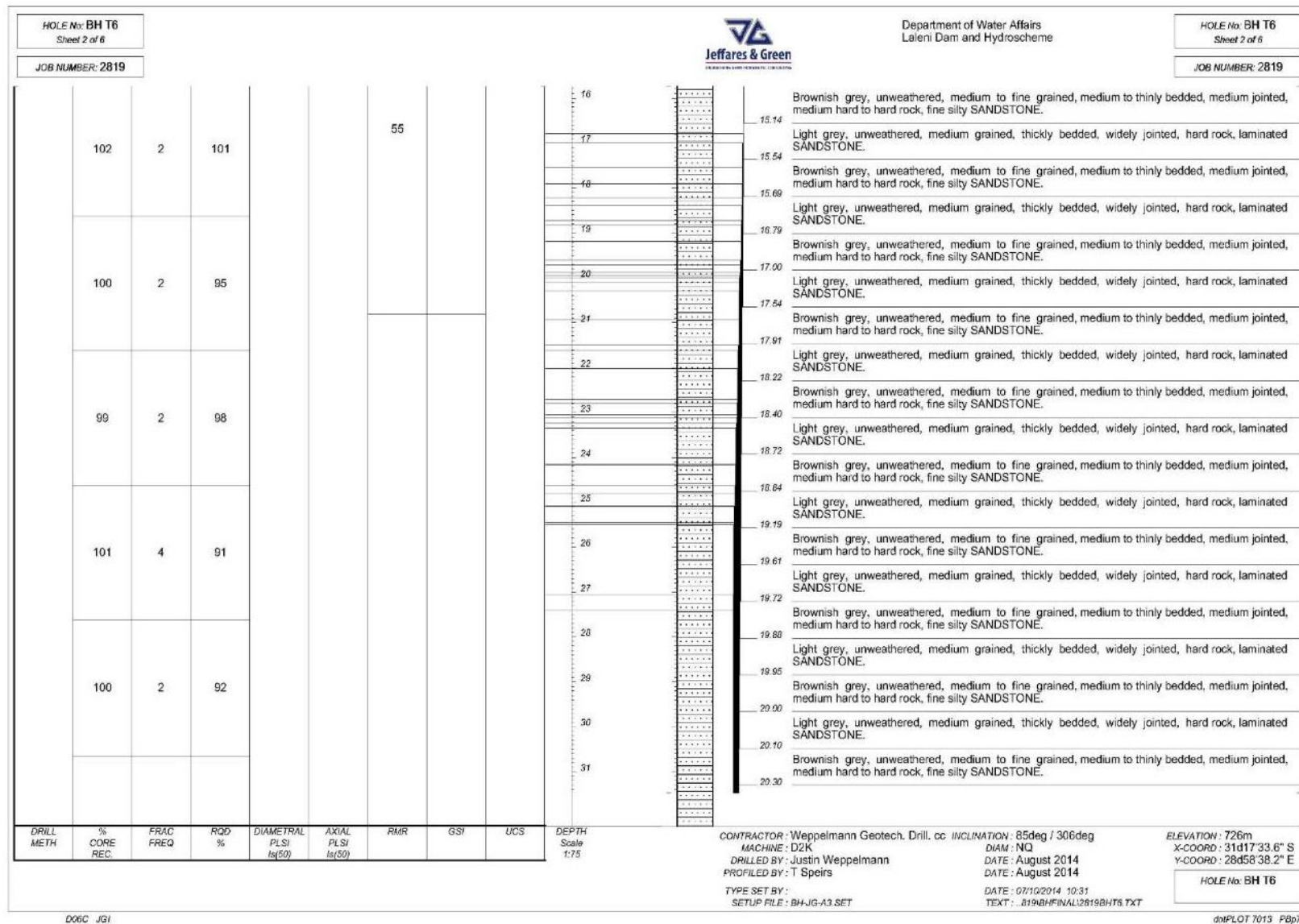


Fig D-12.2: Borehole Tunnel 6 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

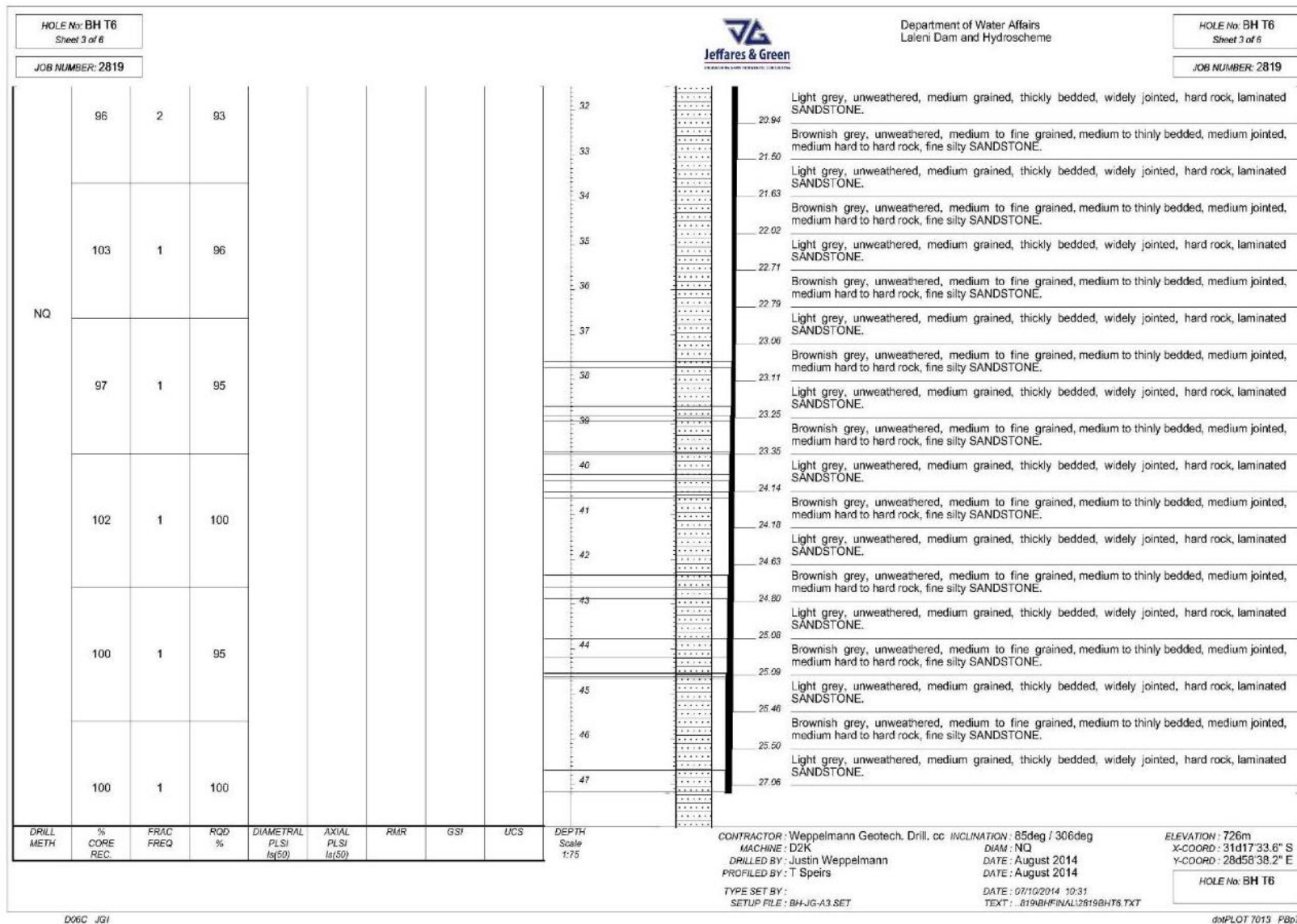


Fig D-12.4: Borehole Tunnel 6 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

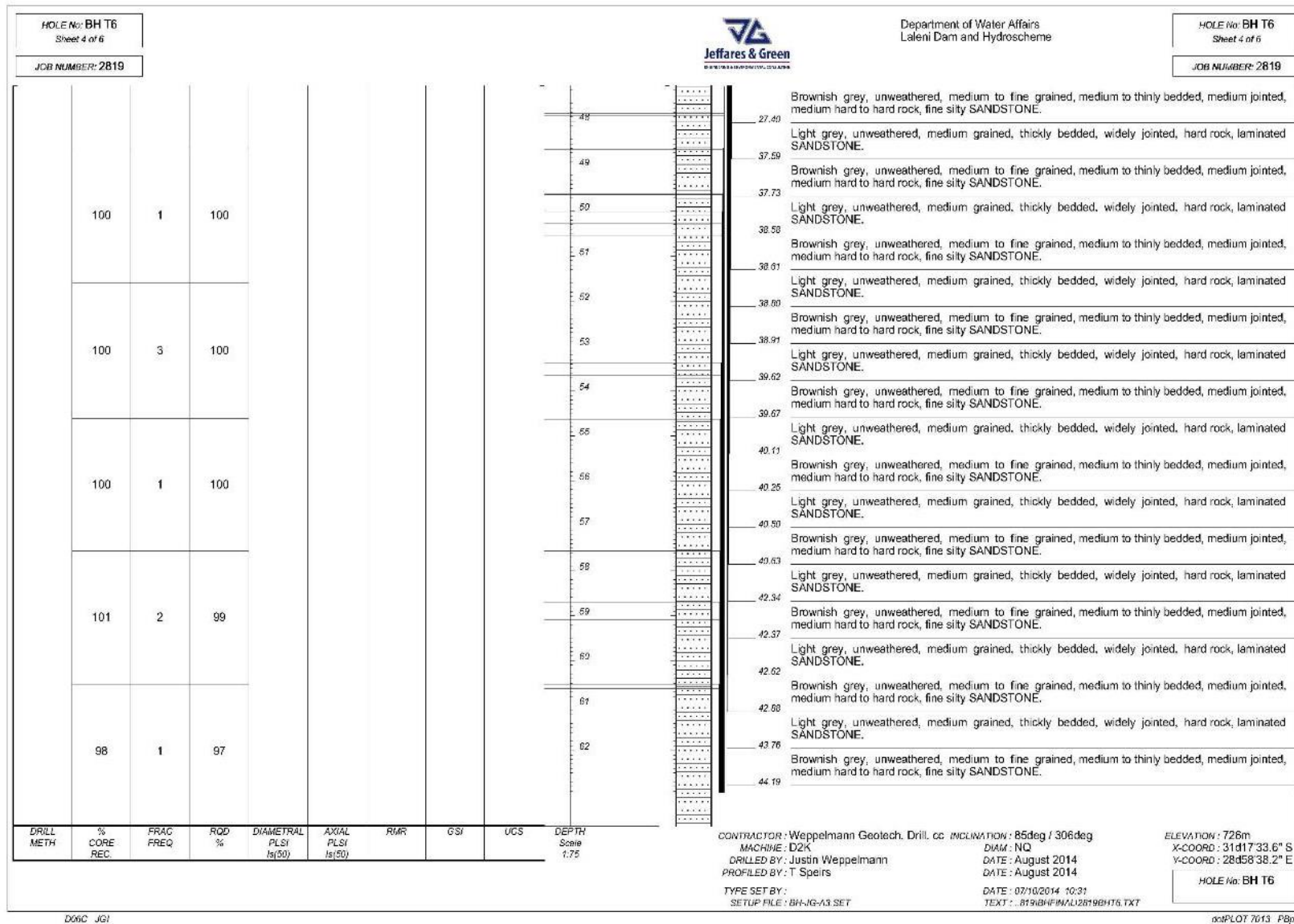


Fig D-12.4: Borehole Tunnel 6 – Log

FEASIBILITY STUDY FOR THE MZIMVUBU WATER PROJECT
 GEOTECHNICAL INVESTIGATIONS: LALINI DAM AND HYDROPOWER SCHEME: APPENDICES

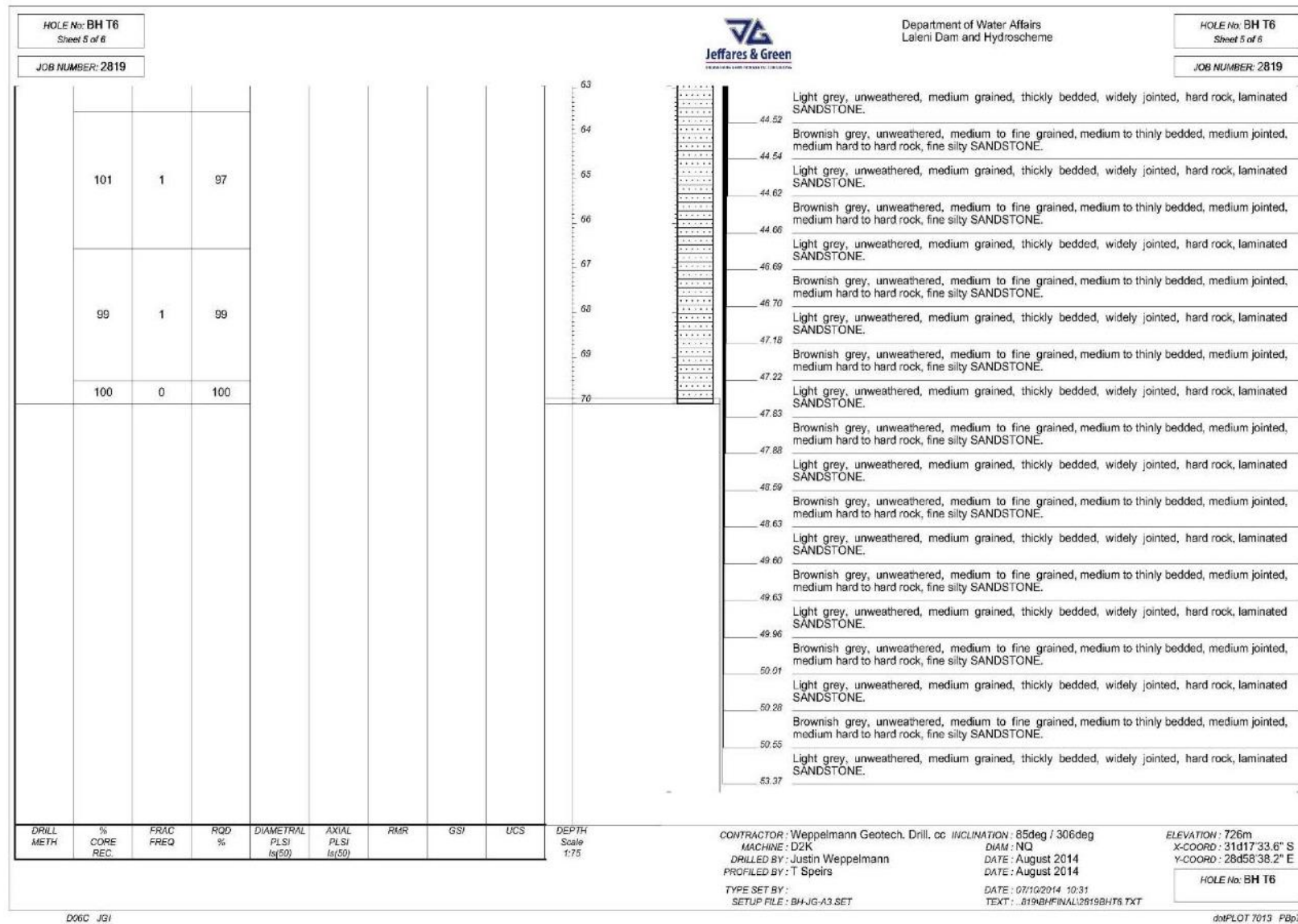


Fig D-12.5: Borehole Tunnel 6 – Log

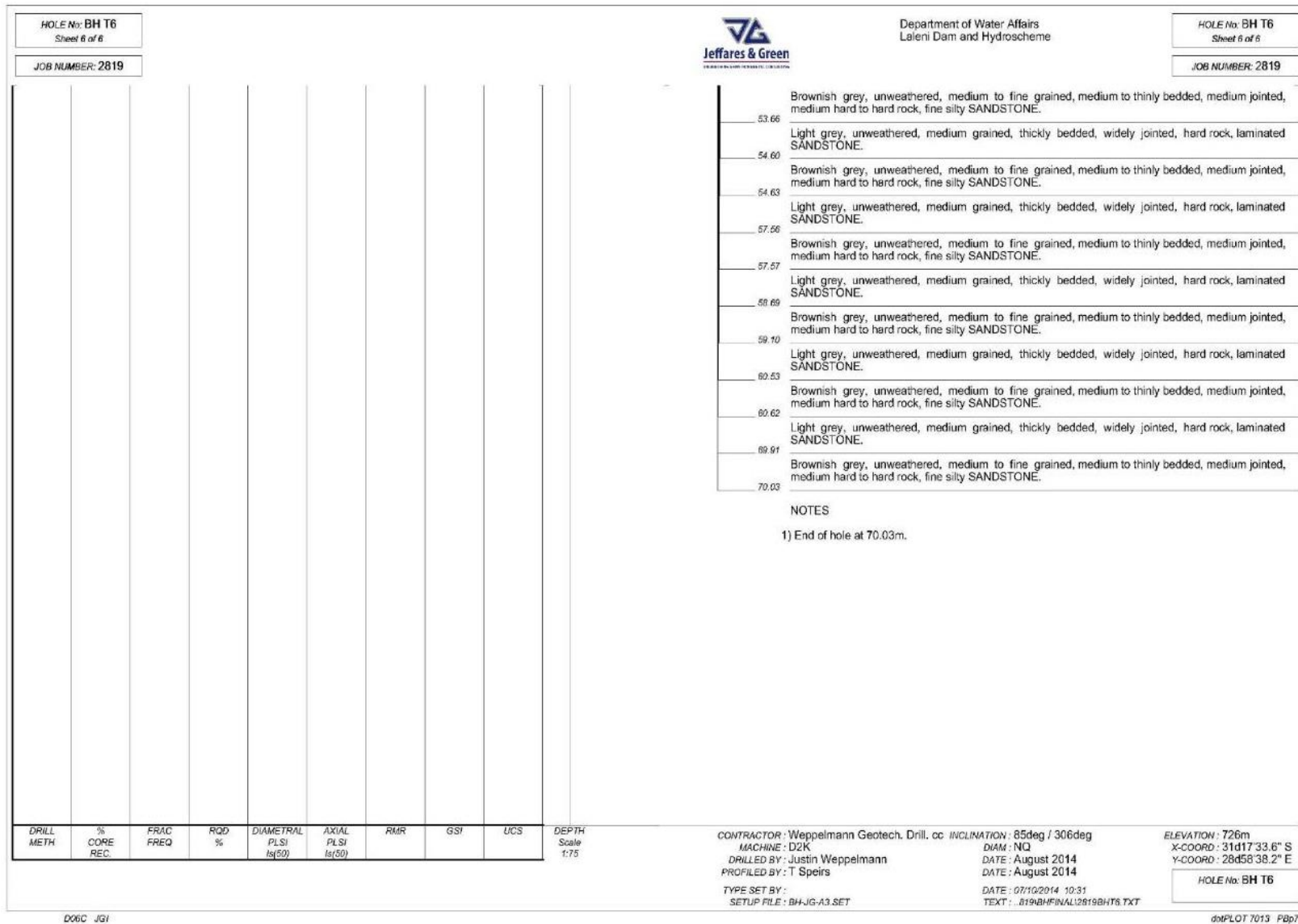


Fig D-12.6: Borehole Tunnel 6 – Log

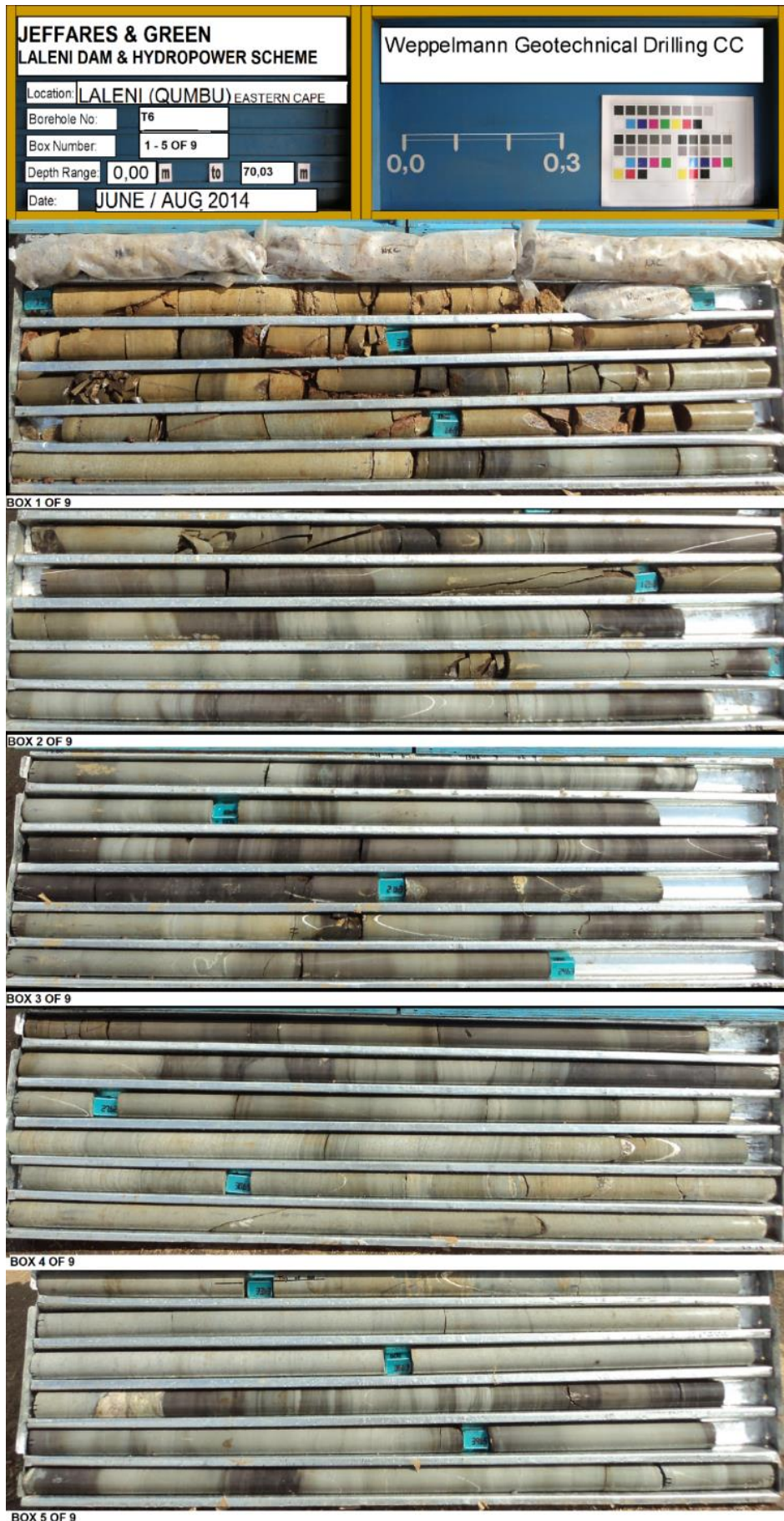


Fig D-13.1: Borehole Tunnel 6 – Box 1 to 5

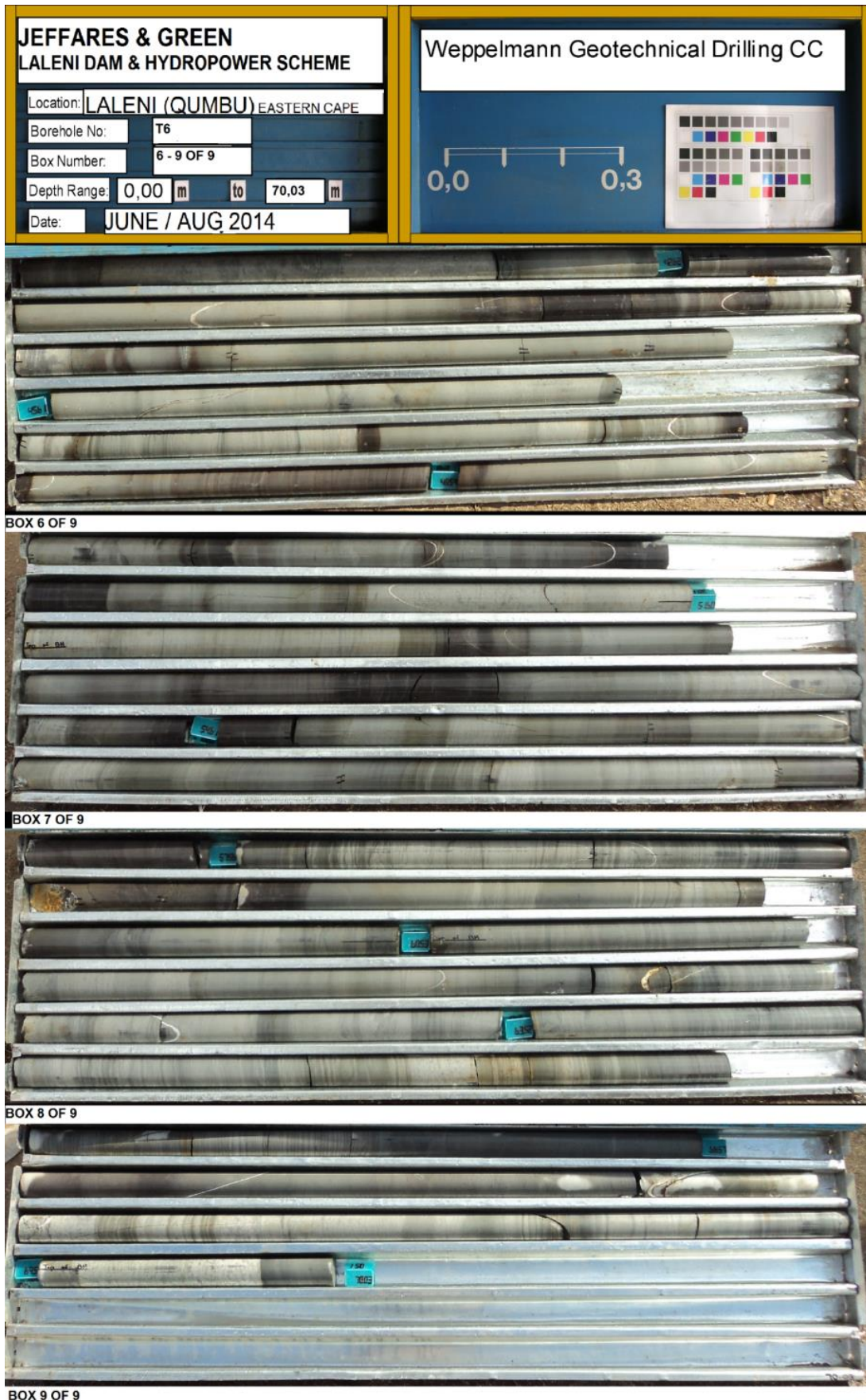


Fig D-13.2: Borehole Tunnel 6 – Box 6 to 9

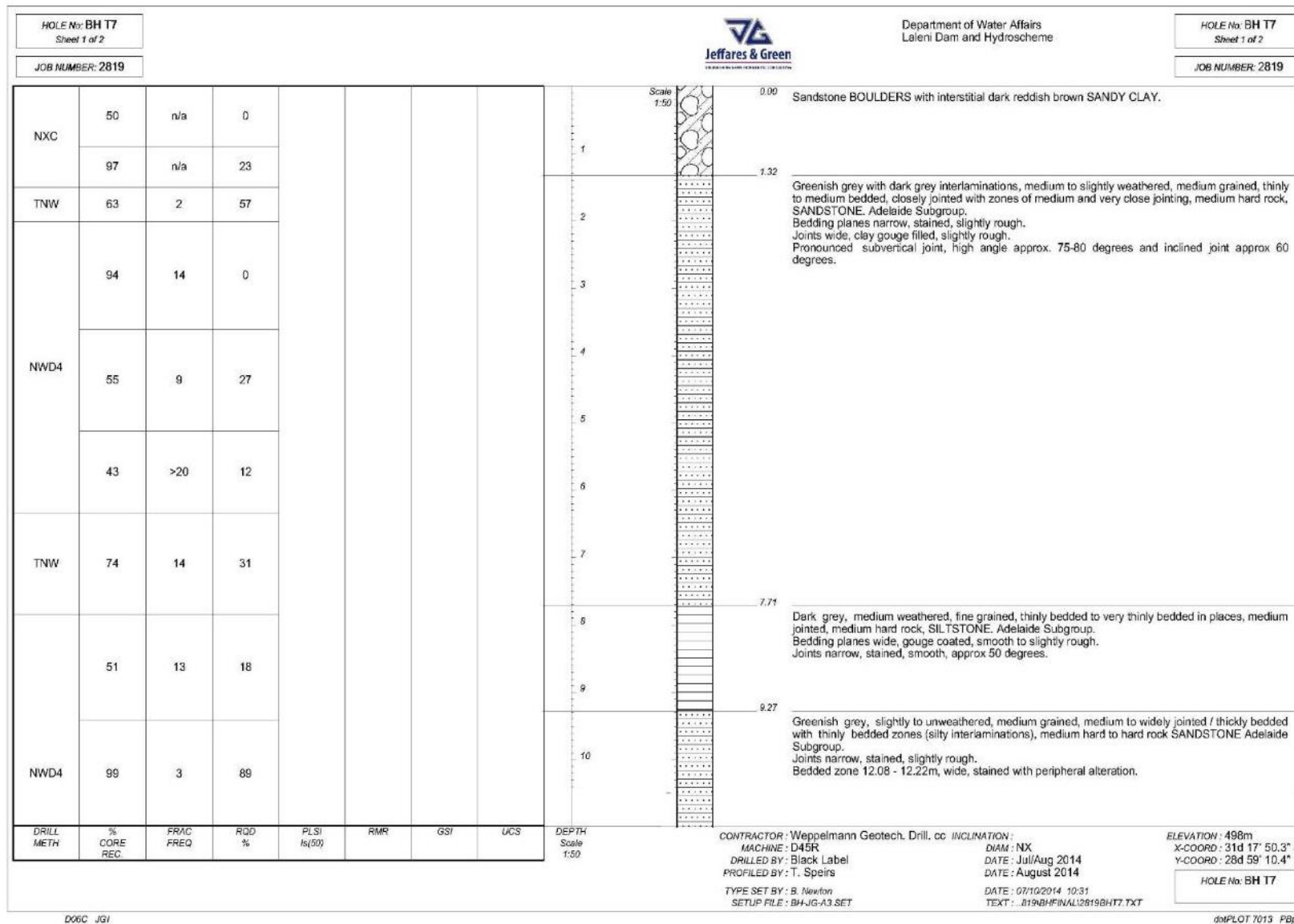


Fig D-14.1: Borehole Tunnel 7 – Log

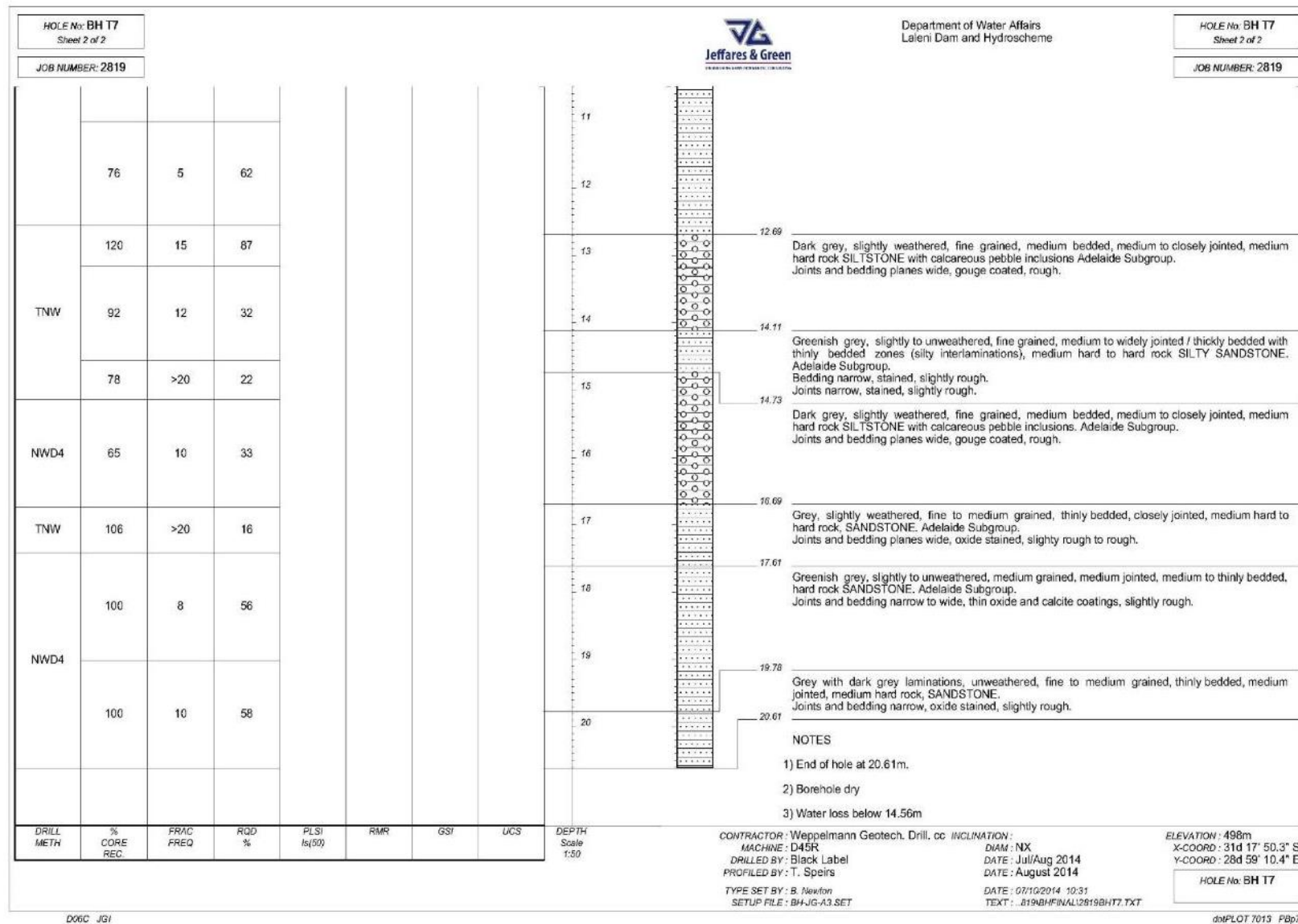


Fig D-14.2: Borehole Tunnel 7 – Log

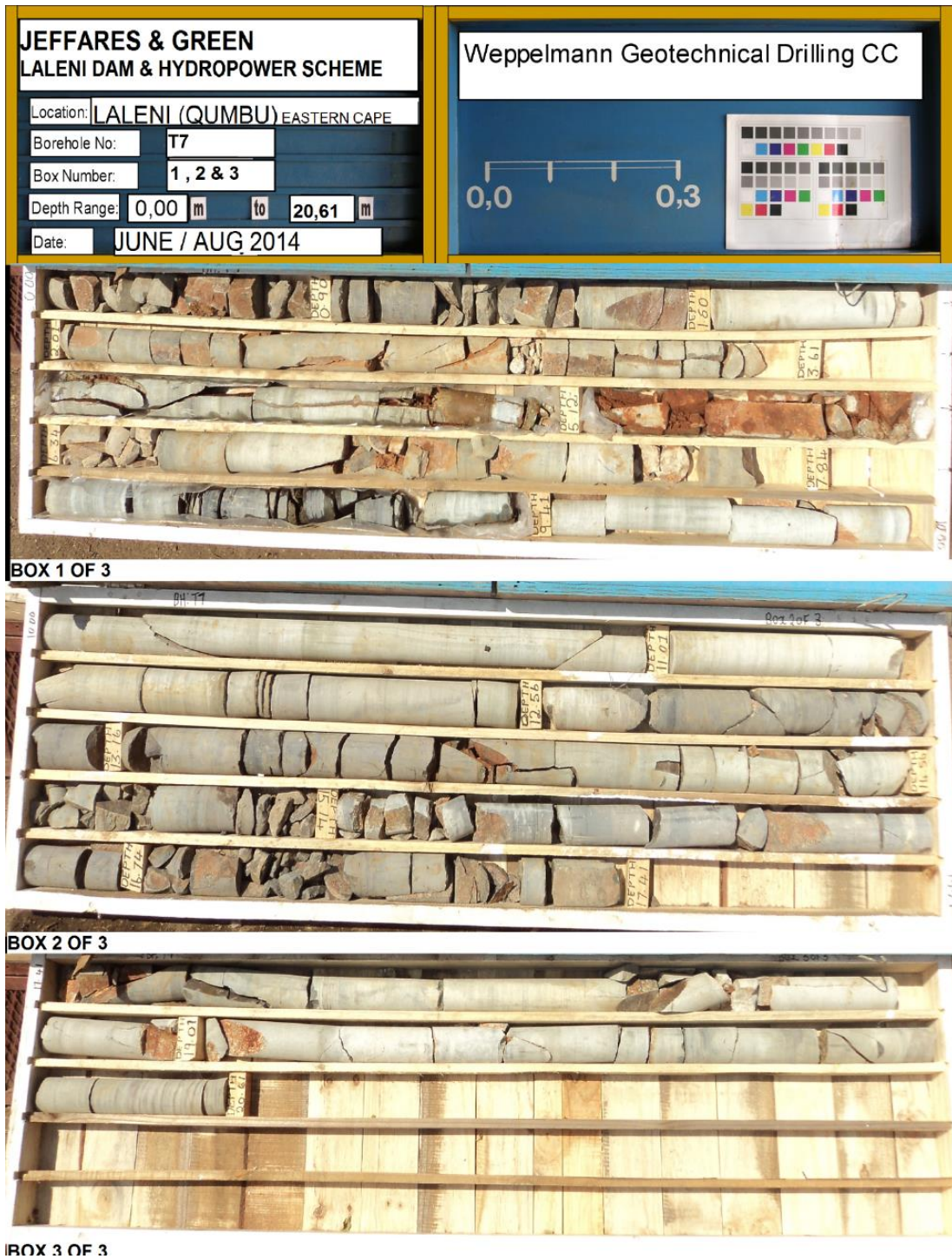


Fig D-14: Borehole Tunnel 6 – Box 1 to 3

APPENDIX E

LABORATORY TEST RESULTS

E1:

UNCONFINED COMPRESSIVE STRENGTH TEST RESULTS



CONTEST

Concrete Technology Services

P O Box 1675, Hillcrest, 3650, South Africa. Tel (031) 700 9394 (031) 700 9342
E-mail : ukhonkolo@contest.co.za

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Ref: CJ 14/08/1005 27 August, 2014
Client: Jaffares & Green
Project: 2819: Laleni Dam & Tunnel
Order: Not Given

LABORATORY REPORT TESTING OF ROCK CORES

1. CLIENT

1.1 Jaffares & Green (Pty) Ltd, PO Box 794, HILTON,
Pietermaritzburg, 3201.

2. BRIEF FROM CLIENT

2.1 Contest was requested to determine the compressive strength of eleven rock cores received.

3. SAMPLES

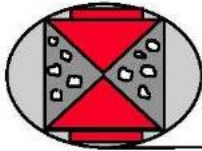
3.1 Eleven rock cores, referenced Q2, 5.89 - 6.09 Dolorite, Q3, 4.49-4.63 Dolorite, D2, 4.51-4.76 Dolorite, D3, 3.80-3.98 Sandstone, D4, 6.67-6.80 Dolorite, T2 50,77-50.93 Sandstone, T2, 50,32 Dolorite, T2, 74,05 Dolorite, T4, 96, 13 Sandstone, 74,97,10 Dolorite, T4, 107.09 Dolrite by the client and 1005/1, 1005/2, 1005/3, 1005/4, 1005/5, 1005/6, 1005/7, 1005/8, 1005/9, 1005/10 and 1005/11 by Contest, were received on 12.08.2014.

4. TESTING

4.1 Eleven cores were tested on 18.08.2014 in accordance with SANS 5865:2006.

5. INFORMATION SUPPLIED BY THE CLIENT

4.1 Site : Laleni Dam and Tunnel
4.2 Location : Not Given
4.3 Drilling contractor : Not given



CONTEST

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Ref: CJ 14/08/1005

27 August, 2014

Client: Jaffares & Green

Project: 2819: Laleni Dam & Tunnel

Order: Not Given

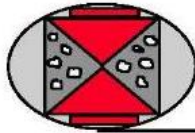
6. CORE PREPARATION

- 6.1 Testing of cores were carried out by our laboratories in Westmead
- 5.2 The cores were measured as received; any significant details recorded and then marked up for trimming.
- 5.3 The cores were photographed in surface dry and wet states.
- 5.4 After trimming to length the cores were weighed in air and water, in order to determine the density.

7. RESULTS

- 7.1 See appended Annexure A and photographs.

RJL RAW B Tech (Civil Eng)



CONTEST
 Concrete Technology Services

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ANNEXURE A						
Client	Jeffares & Green					
Project	2819: Laleni Dam & Tunnel					
Job Number	CJ 14/08/1005					
Date Cast	Not Given					
Date Tested	18.08.2014					
Age, Day	Not Given					
Date Received	12.08.2014					
Direction of Dilling	Not Given					
Report date	27.08.2014					
Client Reference						
Contest Reference	Q2 5.89-6.09	Q3 4.49-4.63	D2 4.51-4.76	D3 3.80-3.98	D4 6.67-6.80	T2 50.77-50.93
	1005/1	1005/2	1005/3	1005/4	1005/5	1005/6
DIMENSIONS						
Max length (mm)	204	140	275	195	128	175
Min length (mm)	201	139	250	180	127	160
Diameter (mm)	51.28	51.20	60.08	51.06	60.11	47.06
Trim Length (mm)	51.25	51.20	60.07	51.08	60.09	47.05
Capped length (mm)	51.25	51.20	60.07	51.08	60.09	47.05
Trim length/diameter	1.00	1.00	1.00	1.00	1.00	1.00
Cap length/diameter	1.00	1.00	1.00	1.00	1.00	0.00
REINFORCEMENT						
	Steel 1	Steel 1	Steel 1	Steel 1	Steel 1	Steel 1
Dist. from end (mm)	0.0	0.0	0.0	0.0	0.0	0.0
Diameter (mm)	0.0	0.0	0.0	0.0	0.0	0.0
Mass (g)	0.0	0.0	0.0	0.0	0.0	0.0
	Steel 2	Steel 2	Steel 2	Steel 2	Steel 2	Steel 2
Dist. from end (mm)	0.0	0.0	0.0	0.0	0.0	0.0
Diameter (mm)	0.0	0.0	0.0	0.0	0.0	0.0
Mass (g)	0.0	0.0	0.0	0.0	0.0	0.0
DENSITY						
Mass in air (g)	322.50	3227.00	456.90	270.30	450.50	219.50
Density (kg/m ³)	2986	1071	2655	2531	2638	2680
Air Voids %	0.00	0.00	0.00	0.00	0.00	0.00
CORRECTIONS						
Length	1.00	1.00	1.00	1.00	1.00	1.00
Steel reinforcement	1.00	1.00	1.00	1.00	1.00	1.00
Air voids	1.00	1.00	1.00	1.00	1.00	1.00
LOADING						
Load at failure (kN)	390.4	351.3	544.3	227.1	411.9	228.2
Failure mode	normal	normal	normal	normal	normal	normal
STRENGTH (MPa)						
Uncorrected Strength	189.0	170.6	192.0	110.9	145.2	131.2
Length corrected	189.0	170.6	192.0	110.9	145.2	131.2
Length/Steel corrected	189.0	170.6	192.0	110.9	145.2	131.2
Length/Voids/Steel corrected	189.0	170.6	192.0	110.9	145.2	131.2

SABS 0100-2, Clause 14.4.2, states that: "At least THREE representative cores shall be taken from each member or predetermined volume of concrete in locations that are considered potentially deficient."
 If the core strengths given in this report are to be used for acceptance in accordance with SABS 0100-2 (or any other specification with similar requirements to those of SABS 0100-2) then the requirements must be complied with otherwise, acceptance based on core strengths using the results given in this report, cannot be made.

This annexure must be read in conjunction with the complete report as indicated by the page number.

RJL RAW B Tech (Civil Eng)

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E - 5

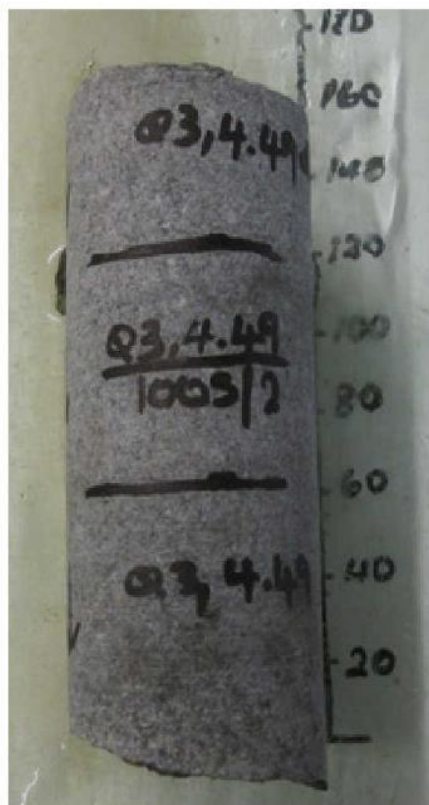
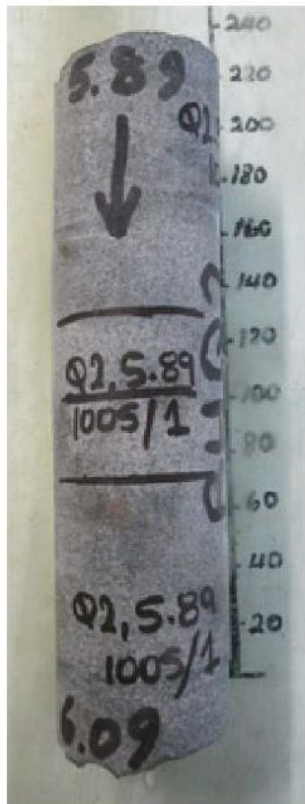


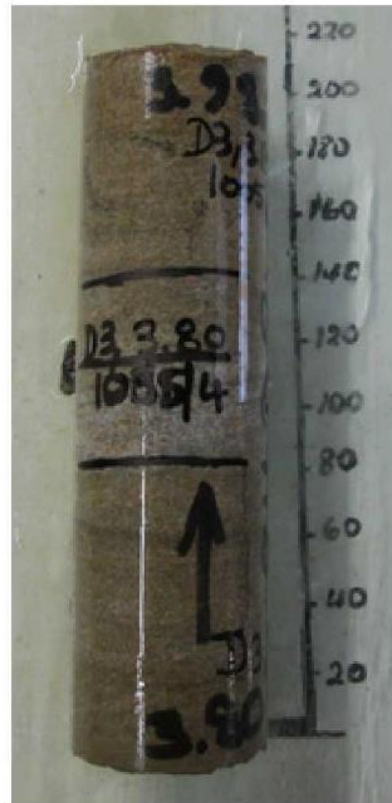
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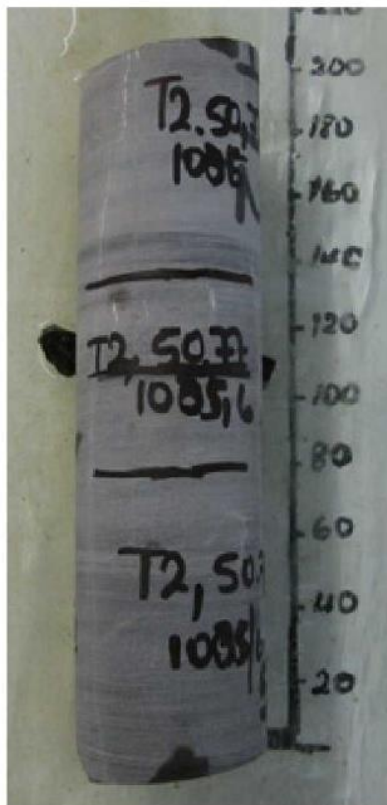
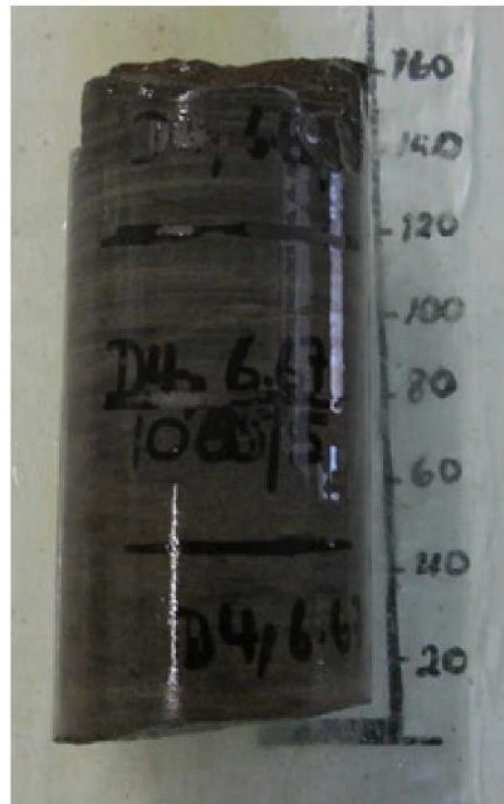
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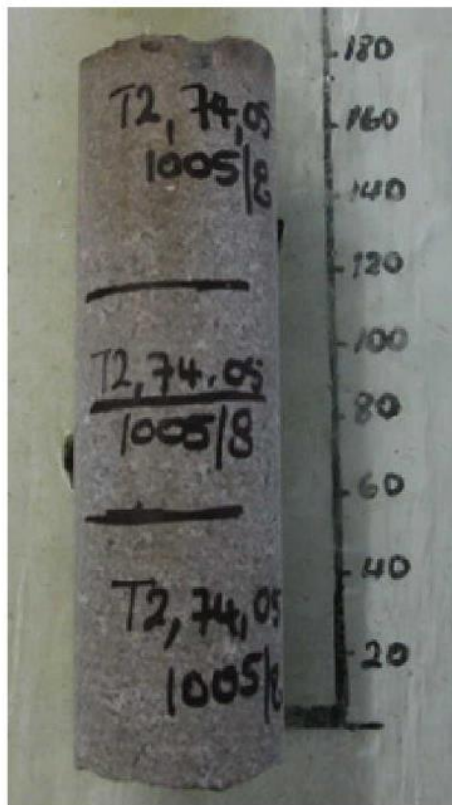
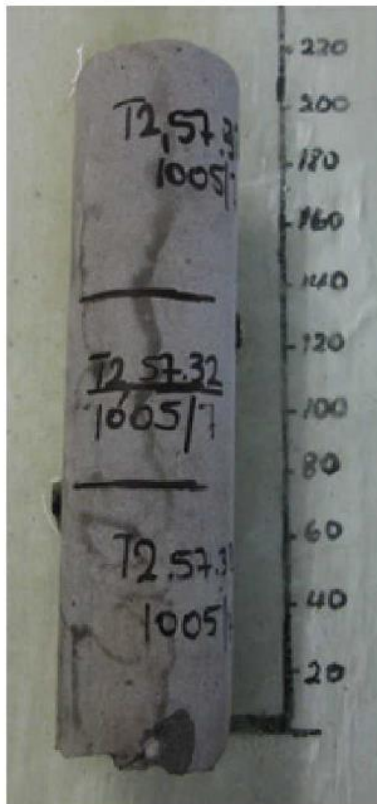
ANNEXURE A					
Client	Jeffares & Green				
Project	Laleni Dam & Tunnel				
Job Number	CJ 14/08/1005				
Date Cast	Not Given				
Date Tested	18.08.2014				
Age, Day	Not Given				
Date Received	12.08.2014				
Direction of Drilling	Not Given				
Report date	27.08.2014				
Client Reference	T2, 50.32	T2, 74.05	T4 96.13	74, 97, 10	T4, 107, 09
Contest Reference	1005/7	1005/8	1005/9	1005/10	1005/11
DIMENSIONS					
Max length (mm)	190	165	140	133	205
Min length (mm)	189	163	139	132	204
Diameter (mm)	47.05	47.10	47.04	47.31	47.08
Trim Length (mm)	47.05	47.06	47.04	47.04	47.07
Capped length (mm)	47.05	47.06	47.04	47.04	47.07
Trim length/diameter	1.00	1.00	1.00	1.01	1.00
Cap length/diameter	1.00	1.00	1.00	1.01	1.00
REINFORCEMENT					
	Steel 1	Steel 1	Steel 1	Steel 1	Steel 1
Dist. from end (mm)	0.0	0.0	0.0	0.0	0.0
Diameter (mm)	0.0	0.0	0.0	0.0	0.0
Mass (g)	0.0	0.0	0.0	0.0	0.0
	Steel 2	Steel 2	Steel 2	Steel 2	Steel 2
Dist. from end (mm)	0.0	0.0	0.0	0.0	0.0
Diameter (mm)	0.0	0.0	0.0	0.0	0.0
Mass (g)	0.0	0.0	0.0	0.0	0.0
DENSITY					
Mass in air (g)	241.50	240.30	218.20	238.10	237.90
Density (kg/m ³)	2931	2923	2704	2921	2933
Air Voids %	0.00	0.00	0.00	0.00	0.00
CORRECTIONS					
Length	1.00	1.00	1.00	1.00	1.00
Steel reinforcement	1.00	1.00	1.00	1.00	1.00
Air voids	1.00	1.00	1.00	1.00	1.00
LOADING					
Load at failure (kN)	533.1	254.7	184.6	202.4	156.8
Failure mode	normal	normal	normal	normal	normal
STRENGTH (MPa)					
Uncorrected Strength	306.6	146.2	106.2	115.1	90.1
Length corrected	306.6	146.2	106.2	115.1	90.1
Length/Steel corrected	306.6	146.2	106.2	115.1	90.1
Length/Voids/Steel corrected	306.6	146.2	106.2	115.1	90.1

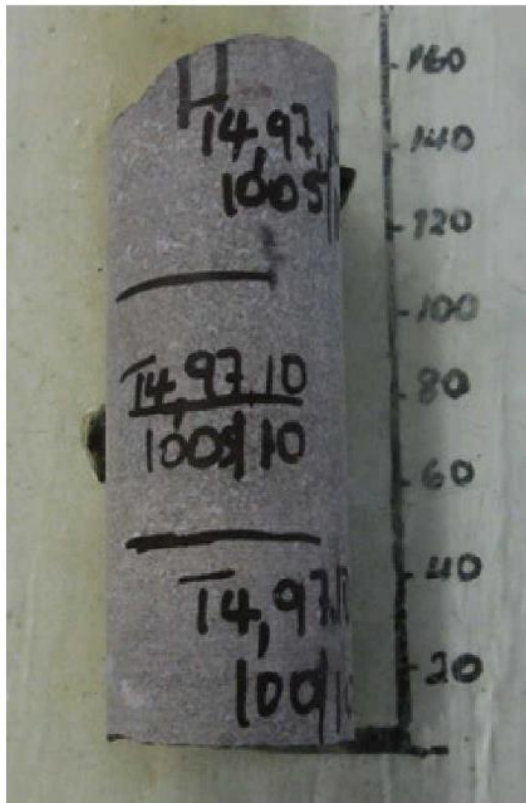
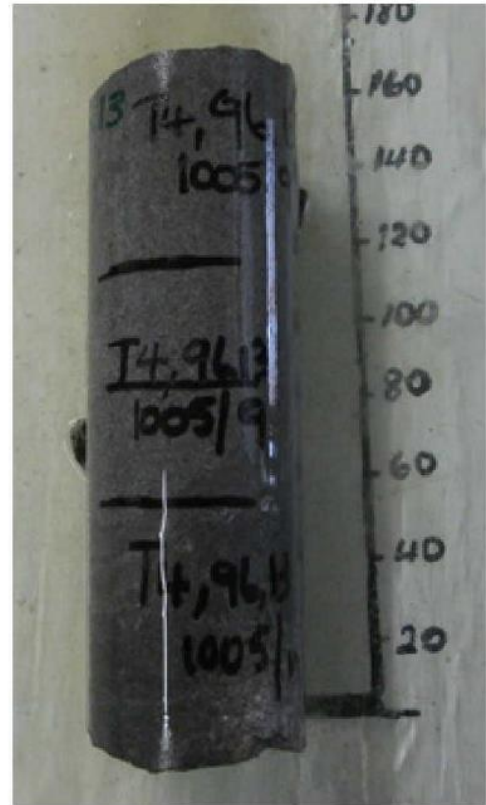
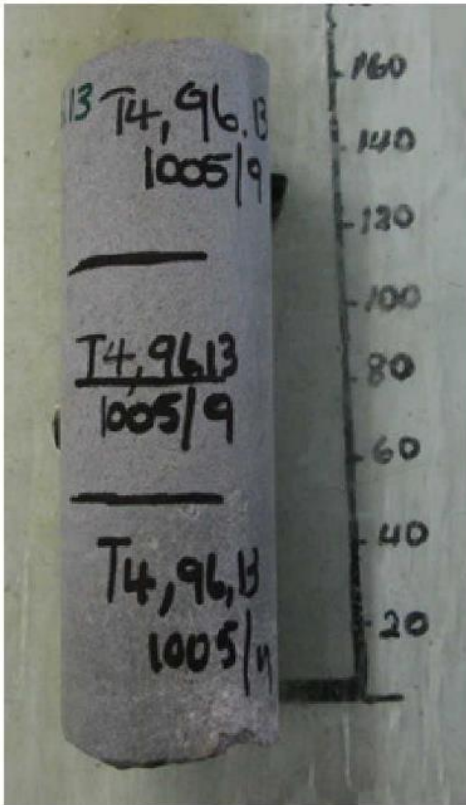
SABS 0100-2, Clause 14.4.2, states that: "At least THREE representative cores shall be taken from each member or predetermined volume of concrete in locations that are considered potentially deficient."
 If the core strengths given in this report are to be used for acceptance in accordance with SABS 0100-2 (or any other specification with similar requirements to those of SABS 0100-2) then the requirements must be complied with otherwise, acceptance based on core strengths using the results given in this report, cannot be made.

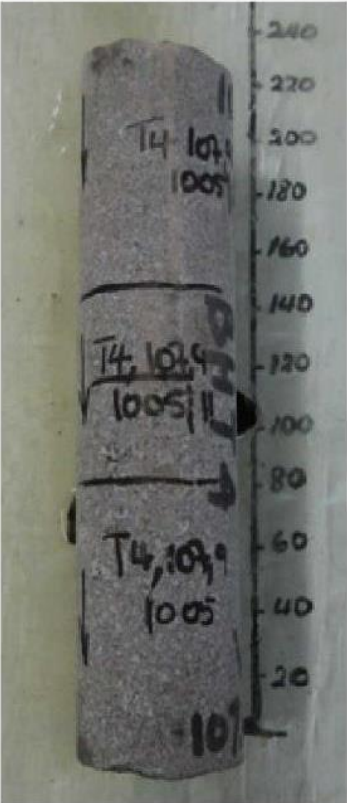














CONTEST

Concrete Technology Services

P O Box 1675, Hillcrest, 3650, South Africa. Tel (031) 700 9394 (031) 700 9342
E-mail : contest@contest.co.za www.contest.co.za

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Ref: CJ 14/081018

27 August 2014

Client: Jeffares and Green

Project: Not Given

Order No: Not Given

LABORATORY REPORT TESTING OF ROCK CORES

1. CLIENT

1.1 Jeffares & Green (Pty) Ltd, PO Box 794, HILTON,
Pietermaritzburg, 3201

2. BRIEF FROM CLIENT

2.1 Contest was requested to determine the compressive strength of three rock cores on 15.08.14. These were tested in accordance with SANS 5865:2006 for concrete.

3. SAMPLES

3.1 Three cores referenced, BHT5 29.37m-29.70m, BHT5 101.17m-101.41 and BHT5 120.99-121.31m by the client and 1018/1, 1018/2, 1018/3 and 1018/4 by Contest were received on 15.08.2014.

4. SAMPLE INFORMATION SUPPLIED BY THE CLIENT

4.1 Site : Not Given
4.2 Location : Not Given

5. TESTING

5.1 Three cores were tested on 23.08.2014 in accordance with SANS 5865:2006.

6. CORE PREPARATION

6.1 Testing of cores were carried out by our laboratories in Westmead

Adam Investments cc. Reg. No 1988/019362/23 t/a CONTEST Concrete Technology Services
Managing Member: R.J.L Raw B Tech (Civil Eng)
Members: MT Clark, JS Dunnnett, MC Mzobe, VA Horton
Consultant: A J M Horton Pr Tech (Eng), Dip ACT, HND (Chem), HNC (Civ. Eng), FICT, MSA Corr I

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Testing, Training and Consulting in Concrete

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Ref: CJ 14/081018 27 August 2014
Client: Jeffares and Green
Project: Not Given
Order No: Not Given

- 6.2 The cores were measured as received; any significant details recorded and then marked up for trimming.
- 6.3 The cores were photographed in surface dry and wet states.
- 6.4 After trimming to length the cores were weighed in air and water, in order to determine the density.
- 6.5 Finally the cores were capped using sulphur mortar and soaked for 48 hours in water prior to testing for compressive strength.

7. RESULTS

- 7.1 See appended Annexure A and photographs.

8. INTERPRETATION OF CORE COMPRESSIVE STRENGTH

- 8.1 If the core strengths given in this report are to be used for acceptance in accordance with SABS 0100-2 (or any other specification with similar requirements to those of SABS 0100-2) then the requirements must be complied with otherwise, acceptance based on core strengths using the results given in this report, cannot be made.
- 8.2 Results given in the attached core report have been adjusted to allow for the presence of transverse reinforcing bars, for length/diameter ratios that fall outside the prescribed ratio and for excess voids, when necessary. Adjustments have been made in accordance with the guidelines given in SANS 5865:2006.
- 8.3 The following extract from SABS 0100-2: Edition 2.2:2000 should be used when assessing the acceptability of concrete based on the compressive strength of cores. The extract is reproduced with the kind permission of the SABS.

14.4.2 CORE TESTS

At least **three** representative cores shall be taken from each member or predetermined volume of concrete in locations that are considered potentially deficient.

14.4.3 ACCEPTANCE OF CONCRETE ON THE BASIS OF CORE STRENGTHS

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The results relate only to the items tested

Ref: CJ 14/081018 27 August 2014
Client: Jeffares and Green
Project: Not Given
Order No: Not Given

14.4.3.1 If the average core strength is at least 80% of the specified strength (see 14.3.3), and if no single core strength is less than 70% of the specified strength, the concrete shall be accepted.

14.4.3.2 If the concrete in a certain area fails to comply with 14.4.3.1 because a single core result falls below 70% of the specified strength, a further set of three cores may be taken from the same area, to determine the extent of deficient concrete. If the new set of three cores complies with the requirements of 14.4.3.1, the area represented by this second set of cores shall be considered acceptable. If the new set of cores fails to comply with the requirements of 14.4.3.1, 14.4.3.3 applies.

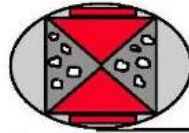
14.4.3.3 If the concrete does not meet the acceptance criteria of 14.4.3.1 or 14.4.3.2, the following should be considered in relation to the deficient part of the structure:

- a) strength requirements for the member(s);
- b) performance of a full scale load test as in clause 15
- c) strengthening of the deficient part of the structure; and
- d) removal and replacement of the deficient part of the structure

8.4 The acceptance criteria given above are based on internationally accepted norms and make allowance for the fact that concrete is specified on the compressive strength of cubes. Cubes give an indication of the potential strength of concrete as mixed, made and cured under standard conditions. The strength of a core is subject to the practical differences associated with the achievement of the cube strength under site conditions and hence the lower strength limits allowed in paragraph 14.4.3 of SABS 0100-2: Edition 2.2:2000.



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B Tech (Civil Eng)



CONTEST

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ANNEXURE A			
Client	Jeffares & Green		
Project	Not Given		
Job Number	CJ 14/081018		
Date Cast	Not Given		
Date Tested	23.08.2014		
Age, Day	Not Given		
Date Received	15.08.2014		
Direction of Drilling	Not Given		
Report date	27.08.2014		
Client Reference			
Contest Reference	BHT5 29.37m-29.70m	BHT5 101.17m-101.41m	BHT5 120.99m-121.31m
	1018/1	1018/2	1018/3
DIMENSIONS			
Max length (mm)	340	250	330
Min length (mm)	336	245	328
Diameter (mm)	47.26	47.19	47.13
Trim Length (mm)	47.25	47.19	47.12
Capped length (mm)	47.25	47.19	47.12
Trim length/diameter	1.00	1.00	1.00
Cap length/diameter	1.00	1.00	1.00
REINFORCEMENT			
	Steel 1	Steel 1	Steel 1
Dist. from end (mm)	0.0	0.0	0.0
Diameter (mm)	0.0	0.0	0.0
Mass (g)	0.0	0.0	0.0
	Steel 2	Steel 2	Steel 2
Dist. from end (mm)	0.0	0.0	0.0
Diameter (mm)	0.0	0.0	0.0
Mass (g)	0.0	0.0	0.0
DENSITY			
Mass in air (g)	208	225	217
Density (kg/m ³)	2541	2745	2648
Air Voids %	0.00	0.00	0.00
CORRECTIONS			
Length	1.00	1.00	1.00
Steel reinforcement	1.00	1.00	1.00
Air voids	1.00	1.00	1.00
LOADING			
Load at failure (kN)	213.6	227.5	318.4
Failure mode	normal	normal	normal
STRENGTH (MPa)			
Uncorrected Strength	121.7	130.1	182.5
Length corrected	121.7	130.1	182.5
Length/Steel corrected	121.7	130.1	182.5
Length/Voids/Steel corrected	121.7	130.1	182.5
<p>SABS 0100-2, Clause 14.4.2, states that: "At least THREE representative cores shall be taken from each member or predetermined volume of concrete in locations that are considered potentially deficient."</p> <p>If the core strengths given in this report are to be used for acceptance in accordance with SABS 0100-2 (or any other specification with similar requirements to those of SABS 0100-2) then the requirements must be complied with otherwise, acceptance based on core strengths using the results given in this report, cannot be made.</p>			

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E2:

**PETROGRAPHIC AND X-RAY
DIFFRACTION ANALYSES**



UNIVERSITEIT VAN PRETORIA
UNIVERSITY OF PRETORIA
YUNIBESITHI YA PRETORIA
Faculty of Natural and Agricultural Sciences

**Faculty of Natural &
Agricultural Sciences
XRD & XRF Facility
Department of Geology
Pretoria 0002, South Africa**

Direct Telephone: (012) 420-2722
Direct Telefax: (012) 362 5219
E-Mail: wiebke.grote@up.ac.za
<http://www.up.ac.za/academic/science>

CLIENT: Jeffares & Green (Pty) Ltd

DATE: 15 September 2014

SAMPLES: 8 Samples

YOUR REFERENCE: 2819: LALENI DAM & TUNNEL

ANALYSIS: XRD and petrographic investigation of 8 core samples

TEST CONDUCTED

8 x Thin sections, 8 x XRD

After crushing and sampling with a riffle splitter, the representative samples were milled in a tungsten carbide vessel and prepared for XRD analysis using a back loading preparation method for qualitative & quantitative XRD.

The samples were analyzed using a PANalytical X'Pert Pro powder diffractometer with X'Celerator detector and variable divergence- and receiving slits with Fe filtered Co-K α radiation. The phases were identified using X'Pert Highscore plus software.

One thin section of each sample was prepared and investigated under the microscope.

MACROSCOPIC DESCRIPTION:

Q2 6.24-6.36; T2 57.5-57.58; T4 97.23-97.31 and T4 115.97-116.06. Massive and fine grained medium grey intrusive igneous rock with a distinct dolerite/basalt texture.

D3 3.61-3.75; T2 50.63-50.77; T4 96.04-96.13 and T5 112.77-113.22. Dense, finely laminated, and very fine grained, light grey meta-sedimentary rocks (weakly metamorphosed siltstone).

MICROSCOPIC DESCRIPTION:

Thin Section Description:

Sample name: Q2 6.24-6.36

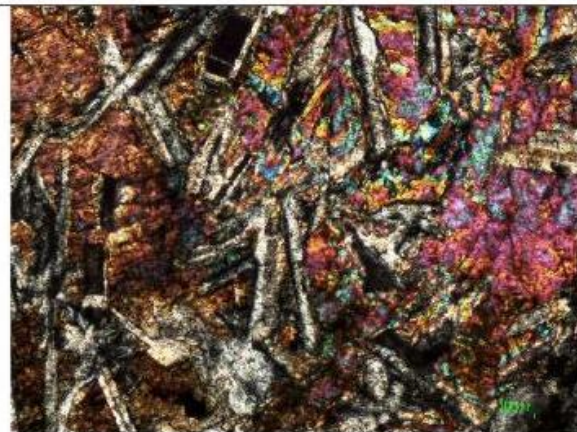
Constituents:

Fine grained euhedral lath-shaped plagioclase crystals (~56%) intergrown by clinopyroxene/diopside (~31%). Lesser quartz (~5%), muscovite (~6%) and minor hornblende (~2%) are present. Traces of opaque minerals might be present.

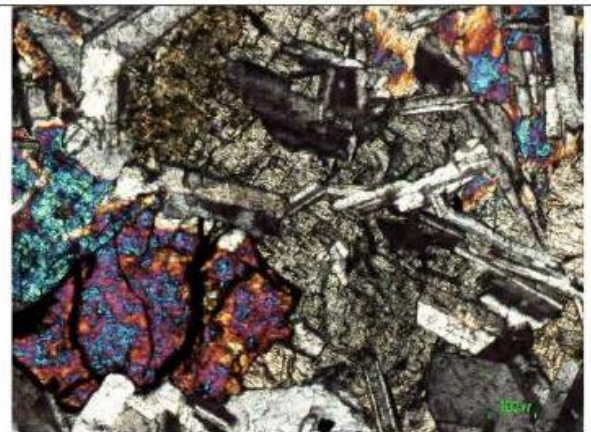
Description:

The section is dominated by generally lath-shaped fine grained feldspar (plagioclase) showing albite twinning, and subhedral to anhedral clinopyroxene (diopside). The laths of plagioclase are randomly orientated and the clinopyroxenes (augite) occupy a more interstitial position to the plagioclase. The degree of alteration is relatively low. Hornblende and mica (muscovite) might be alteration products.

Grain size data: 100 - 800 micron



Photomicrograph Q2 6.24-6.36: Cross polarized light. 10x Magnification.



Photomicrograph Q2 6.24-6.36: Cross polarized light. 10x Magnification.

Sample name: D3 3.61-3.75

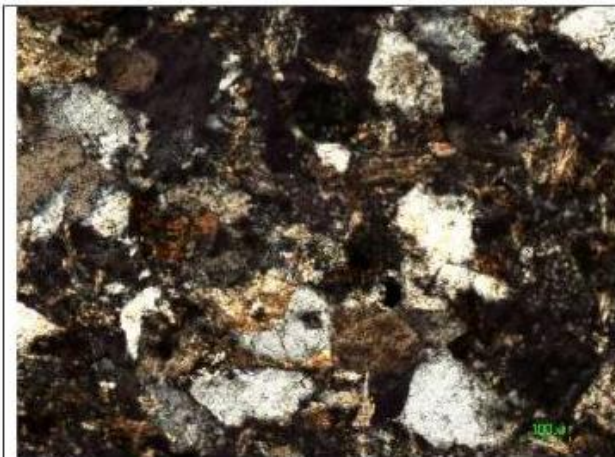
Constituents:

Fine grained quartz (~48%), plagioclase (~22%), microcline (~8%), muscovite (~7%), chlorite (~8%) and laumontite (~7%).

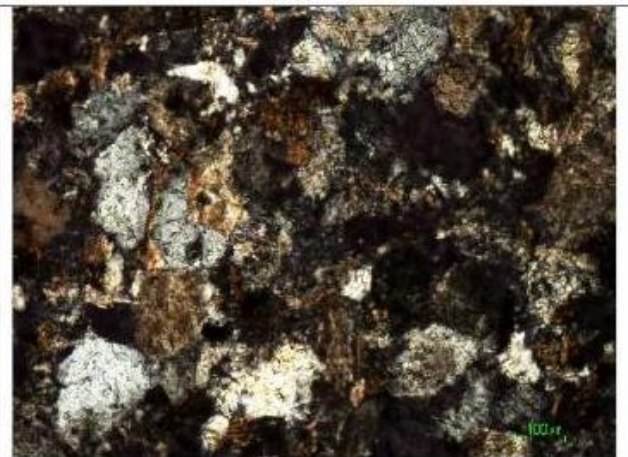
Description:

The section is dominated by angular to subrounded quartz, plagioclase and microcline embedded in a fine groundmass. The presence of the zeolite laumontite seems to be diagenetic (low grade metamorphism).

Grain Size: 100 – 500 micron



Photomicrograph D3 3.61-3.75: Cross polarized light. 10x Magnification.



Photomicrograph D3 3.61-3.75: Cross polarized light. 10x Magnification.

Sample name: T2 50.63-50.77

Constituents:

Extremely fine grained quartz (~42%), plagioclase (~19%), muscovite (~17%), microcline (~9%) chlorite (~9%) and laumontite (~4%).

Description:

The sample is extremely fine grained. Optical study is hampered by the fine grain and is difficult to distinguish between the various minerals.

Grain Size: <100 micron



Photomicrograph T2 50.63-50.77: Cross polarized light. 10x Magnification.



Photomicrograph T2 50.63-50.77: Cross polarized light. 10x Magnification.

Sample name: T2 57.5-57.58

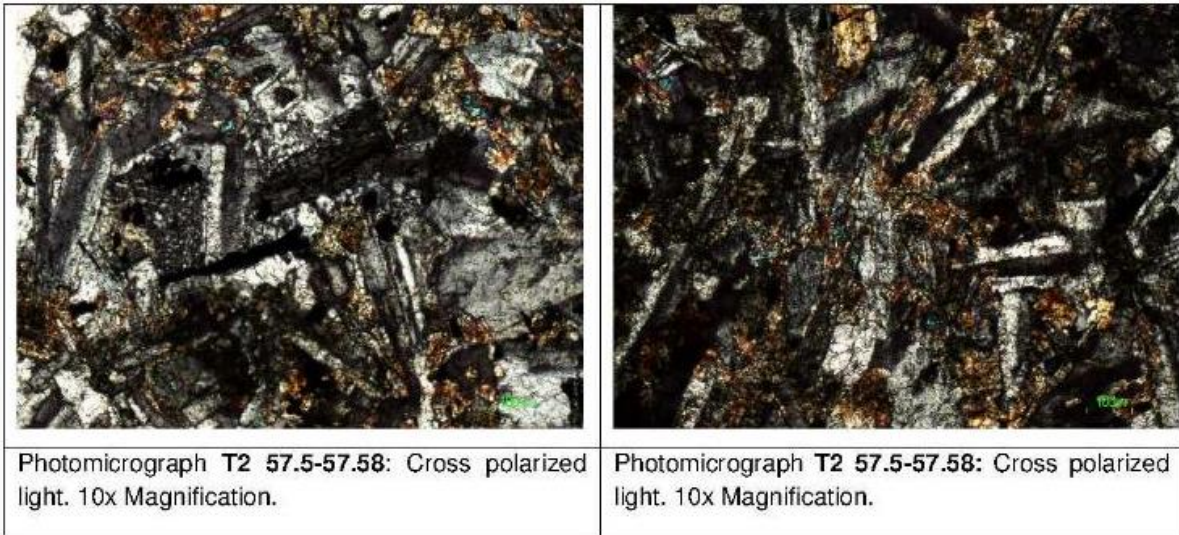
Constituents:

Fine grained euhedral lath-shaped plagioclase crystals (~49%) intergrown by clinopyroxene/diopside (~18%) and enstatite/orthopyroxene (~13%). Lesser quartz (~7%), orthoclase (~5%), hornblende (~5%) and minor muscovite/mica (~3%) are present. Traces of opaque minerals might be present.

Description:

The section is dominated by generally lath-shaped fine grained feldspar (plagioclase) showing albite twinning, and subhedral to anhedral clinopyroxene (diopside). The laths of plagioclase are randomly orientated and the diopside occupies a more interstitial position to the plagioclase. The amphibole/hornblende and mica might be alteration products.

Grain size data: 100 - 1000 micron



Sample name: T4 96.04-96.13

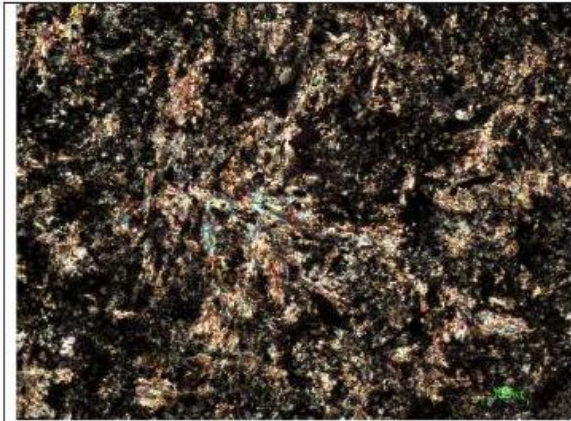
Constituents:

Very fine grained quartz (~36%), plagioclase (~19%), muscovite (~19%), microcline (~9%), chlorite (~11%) and laumontite (~5.5%)

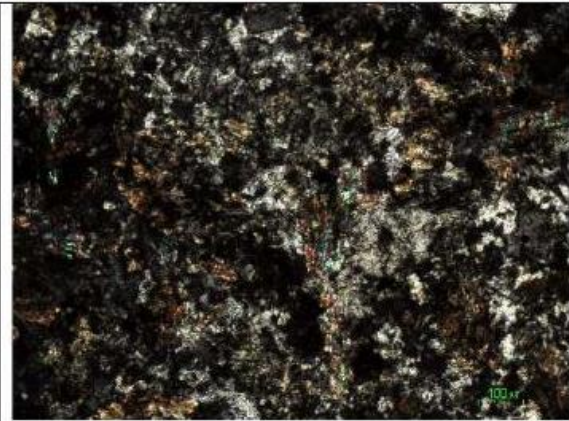
Description:

The sample is very fine grained. Optical study is hampered by the fine grain and is difficult to distinguish between the various minerals.

Grain size data: <200 micron



Photomicrograph T4 96.04-96.13: Cross polarized light. 10x Magnification.



Photomicrograph T4 96.04-96.13: Cross polarized light. 10x Magnification.

Sample name: T4 97.23 - 97.31

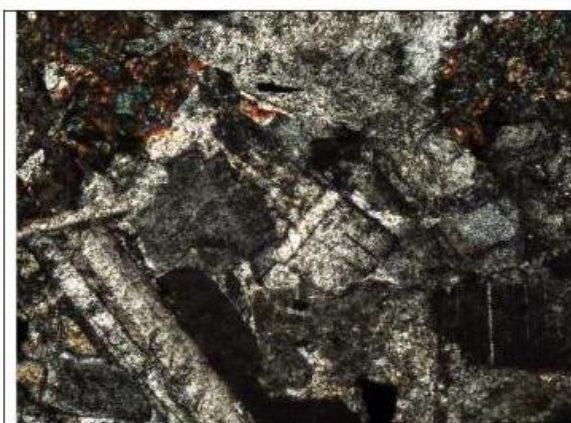
Constituents:

Fine grained euhedral lath-shaped plagioclase crystals (~33%) intergrown by clinopyroxene/diopside (~31%). Lesser orthoclase (~12%), prehnite (~13%) quartz (~6%), actinolite/amphibole (~4%) and minor epidote (~1%) are present. Traces of opaque minerals might be present.

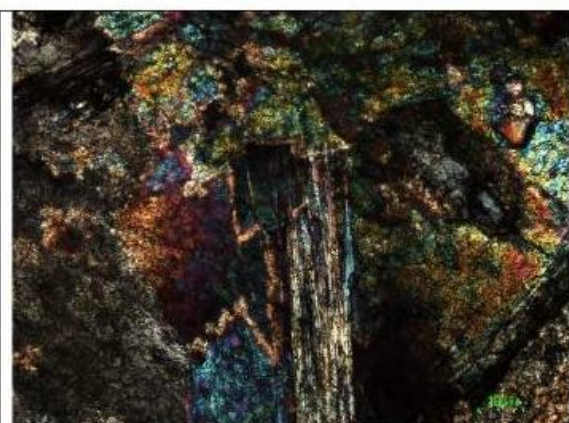
Description:

The section is dominated by generally lath-shaped fine grained feldspar (plagioclase) showing albite twinning, and subhedral to anhedral clinopyroxene (diopside). The laths of plagioclase are randomly orientated and the clinopyroxenes (diopside) occupy a more interstitial position to the plagioclase.

Grain size data: 100 - 1000 micron



Photomicrograph T4 97.23-9.31: Cross polarized light. 10x Magnification.



Photomicrograph T4 97.23-9.31: Cross polarized light. 10x Magnification.

Sample name: T4 115.97-116.06

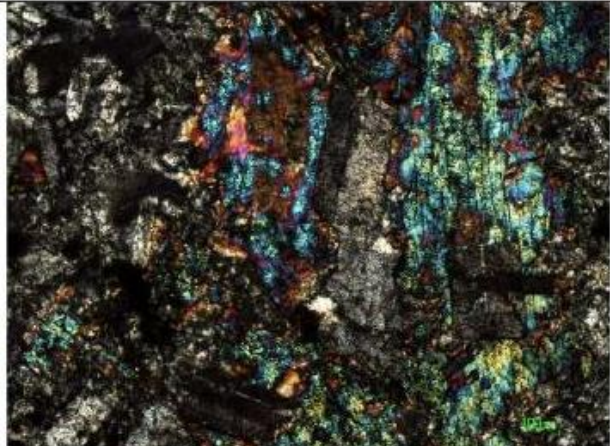
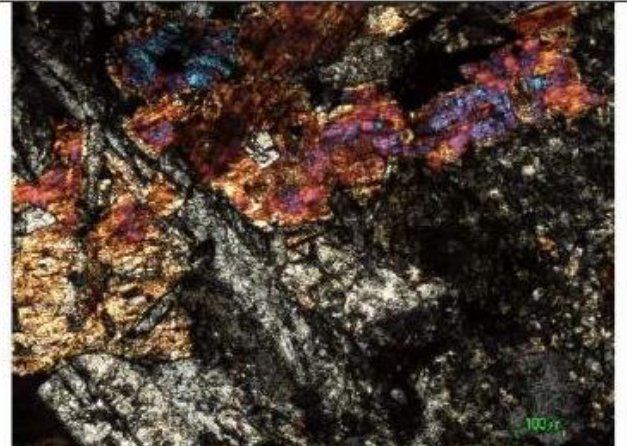
Constituents:

Fine grained euhedral lath-shaped plagioclase crystals (~51%) intergrown by clinopyroxene/diopside (~25%) and microcline (~9%). Lesser quartz (~4%), muscovite (~5%), hornblende (~4%) and about 2% opaque minerals (ilmenite) is present.

Description:

The section is dominated by generally lath-shaped medium to fine grained feldspar (plagioclase) showing albite twinning, and subhedral to anhedral clinopyroxene (augite). The laths of plagioclase are randomly orientated and the diopside occupy a more interstitial position to the plagioclase. The hornblende and muscovite (mica) might be alteration products. The degree of alteration is quite low.

Grain size data: 100 - 1000 micron

	
<p>Photomicrograph T4 115.97-116.06: Cross polarized light. 10x Magnification.</p>	<p>Photomicrograph T4 115.97-116.06: Cross polarized light. 10x Magnification.</p>

Sample name: T5 112.77-113.22

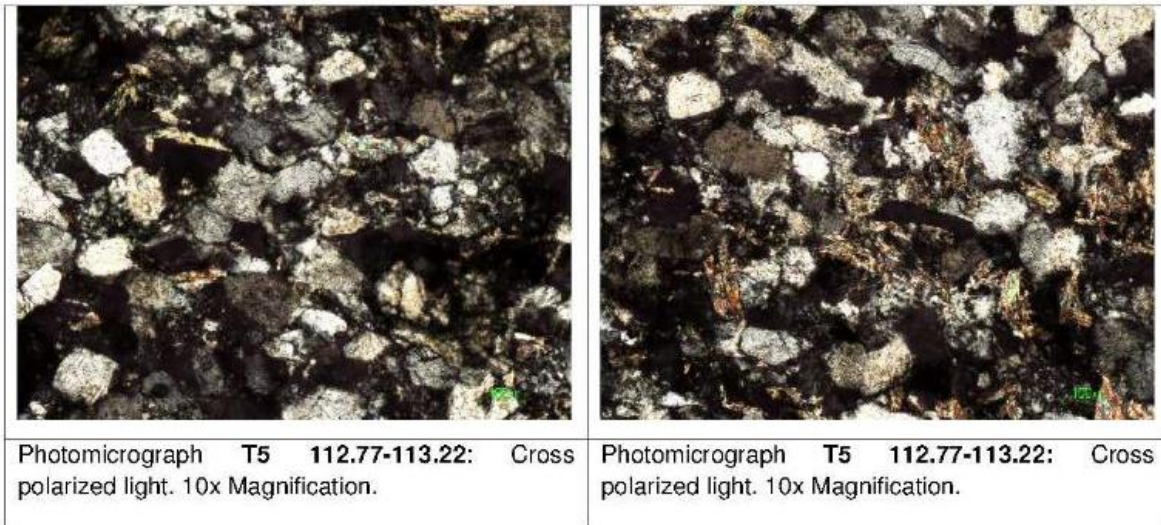
Constituents:

Fine grained plagioclase crystals (~40%), quartz (~38%), microcline (~9%), muscovite (~7%) and lesser chlorite (~6%).

Description:

The section is dominated by angular to subrounded quartz, plagioclase and microcline embedded in a fine groundmass. No laumontite was detected in this sample.

Grain size data: 100 – 300 micron



The overall bulk composition of the sample estimated by **quantitative XRD** analysis using the Rietveld method is as follows (3 σ error = standard deviation):

D3_3.61-3.75			Q2_6.24-6.36			T2_50.63-50.77		
	weight%	3 σ error		weight%	3 σ error		weight%	3 σ error
Chlorite	8.44	1.11	Diopside	31.42	0.95	Chlorite	9.4	0.87
Laumontite	6.64	1.35	Hornblende	1.84	0.9	Laumontite	3.67	1.14
Microcline	7.98	1.11	Muscovite	5.63	0.72	Microcline	9.1	0.99
Muscovite	6.92	0.81	Plagioclase	56.31	1.11	Muscovite	17.35	0.87
Plagioclase	21.66	1.14	Quartz	4.8	0.33	Plagioclase	18.95	1.21
Quartz	48.36	1.44				Quartz	41.54	1.06
T2_57.5-57.58			T4_96.04-96.13			T4_97.23-97.31		
	weight%	3 σ error		weight%	3 σ error		weight%	3 σ error
Diopside	18.22	0.9	Chlorite	11.3	1.02	Actinolite	4.25	0.54
Enstatite	12.86	0.96	Laumontite	5.49	1.04	Diopside	30.81	0.93
Hornblende	5.35	0.6	Microcline	8.92	0.99	Epidote	1.03	0.54
Muscovite	2.72	0.57	Muscovite	19.45	0.9	Orthoclase	11.9	0.9
Orthoclase	4.82	0.75	Plagioclase	19.16	1.2	Plagioclase	32.92	1.08
Plagioclase	49.03	1.02	Quartz	35.68	1.08	Prehnite	12.64	0.81
Quartz	7	0.36				Quartz	6.45	0.39
T4_115.97-116.06			T5_112.77-113.22					
	weight%	3 σ error		weight%	3 σ error			

Diopside	25.07	1.29	Chlorite	5.59	0.72			
Hornblende	3.92	1.05	Laumontite	0	0			
Ilmenite	1.99	0.9	Microcline	8.98	0.99			
Microcline	8.94	0.87	Muscovite	6.95	0.72			
Muscovite	4.63	0.66	Plagioclase	40.47	1.17			
Plagioclase	51.11	1.62	Quartz	38	0.96			
Quartz	4.34	0.39						

Ideal Mineral Composition:

Actinolite	$\text{Ca}_2(\text{Mg,Fe})_5\text{Si}_8\text{O}_{22}(\text{OH})$
Chlorite	$(\text{Mg,Fe})_5\text{Al}(\text{AlSi}_3\text{O}_{10})(\text{OH})_8$
Diopside	$\text{CaMgSi}_2\text{O}_6$
Enstatite	MgSiO_3
Epidote	$\text{Al}_2\text{Ca}_2\text{MnSi}_3\text{O}_{12}(\text{OH})$
Hornblende	$\text{Ca}_2(\text{Fe,Mg})_4\text{Al}(\text{Si}_7\text{Al})\text{O}_{22}(\text{OH,F})_2$
Ilmenite	FeTiO_3
Laumontite	$\text{CaAl}_2\text{Si}_4\text{O}_{12}(\text{H}_2\text{O})_2$
Microcline	KAlSi_3O_8
Muscovite	$\text{KA}_2((\text{OH})_2\text{AlSi}_3\text{O}_{10})$
Orthoclase	KAlSi_3O_8
Plagioclase	$(\text{Na,Ca})(\text{Si,Al})_4\text{O}_8$
Prehnite	$\text{Ca}_2\text{Al}(\text{AlSi}_3\text{O}_{10})(\text{OH})_2$
Quartz	SiO_2

EXECUTIVE SUMMARY:

- Samples Q2 6.24-6.36; T2 57.5-57.58; T4 97.23-97.31 and T4 115.97-116.06 are dolerite or basalts. No smectite (swelling clays) was detected. The prehnite in sample T4 97.23-97.31 might be secondary or a hydrothermal mineral.
- The dolerite/basalt samples appear holocrystalline, relatively fresh and fine grained with an ophitic texture and these samples seem to be fairly similar.
- Samples D3 3.61-3.75; T2 50.63-50.77; T4 96.04-96.13 and T5 112.77-113.22, vary between very fine grained to extremely fine grained. These seem to be meta-sedimentary rocks, which have undergone low grade metamorphism.

If you have any questions, kindly contact the laboratory.

Analyst:



Wiebke Grote

E3:

SAND TEST RESULTS



CONTEST

Concrete Technology Services

P O Box 1675, Hillcrest, 3650, South Africa. Tel (031) 700 9394 (031) 700 9342
E-mail : contest@contest.co.za Web Page: www.contest.co.za

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Ref: CJ14/08/1004 18 September 2014
Client: Jeffares & Green
Subject: Aggregate Testing Report
Project: Tsitsa River

Testing Report

CLIENT:

Jeffares & Green (Pty) Ltd, P O Box 794, HILTON, 3245

SYNOPSIS:

Contest carried out aggregate testing on five aggregate samples received as per the client's request.

BRIEF FROM CLIENT:

Contest was requested to carry out the following test and report the results;

- Chloride Content
- Grading
- Organic Impurities
- pH
- Soluble deleterious Impurities
- Soluble Sulphates

SAMPLES:

The following sample was received on 12.08.2014.

- Sand (S1 – Tsitsa River) (our lab ref1004/1)
- Sand (S2 – Tsitsa River) (our lab ref 1004/2)
- Sand (S3 – Tsitsa River) (our lab ref 1004/3)
- Sand (S4 – Tsitsa River) (our lab ref 1004/4)
- Sand (combined S1 – S4) (our lab ref 1004/5)

TESTS:

Test	Test Method
Chloride Content	SM SABS 830:1994
Grading	SM SABS 829:1994

Adam Investments cc. Reg. No 1988/019362/23 t/a CONTEST Concrete Technology Services
Managing Member: R.J.L Raw B Tech (Civil Eng)
Members: MT Clark, JS Dunnett, MC Mzobe
Consultant: A J M Horton Pr Tech (Eng), Dip ACT, HND (Chem), HNC (Civ. Eng), FICT, MSA Corr I

Testing, Training and Consulting in Concrete

Page 1 of 6

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Ref: CJ14/08/1004 18 September 2014
 Client: Jeffares & Green
 Subject: Aggregate Testing Report
 Project: Tsitsa River

Organic Impurities	SM SABS 832:1976
pH	TMH 1 Method A 20
Soluble Deleterious Impurities	SABS SM 834:1996
Soluble Sulphates	Gravimetric

RESULTS:

See attached Appendix A for results

COMMENT:

Chloride Content:

The test results of the sample tested meets the requirements of SANS 1083:2006 which are given below;

Chloride Content, expressed as Cl ⁻ , mass percentage, max.	Type of Concrete;	SABS (mass %, as Cl ⁻) of sand	BS (mass %, as Cl ⁻) of combined aggregate:
	Concrete for pre-stressing:	0.01	0.01
	Concrete made with supersulphated and sulphated resisting cement	-	0.03
	Normal reinforced concrete:	0.03	0.05
	Non-reinforced concrete:	0.08	-

The SABS limits are based on the mass of sand used, the BS limit on the total aggregate. The aggregate tested met the SABS requirements and would meet the BS limits for most concrete mixes.

Organic Impurities:

The colour of the test solution made with the aggregate sample (1004/5) was similar in colour than that of the colour depth comparator. The sample is therefore unlikely to have a sufficient quantity of visible organic impurities to negatively affect concrete setting times or compressive strength.

pH:

Typical values for aggregates are between 6.5 and 10. The aggregate tested met these requirements.

Soluble Deleterious Impurities:

The test method used was SABS 834:1994 except that instead of testing cubes at 7 days only, additional cubes were made for 28d testing in case any results at 7d proved to be suspect. Compressive strength of cubes at 24 hours and 3 days was carried out to determine any early age effect on the concrete strength.

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The results relate only to the items tested

Ref: CJ14/08/1004
 Client: Jeffares & Green
 Subject: Aggregate Testing Report
 Project: Tsitsa River

18 September 2014

SABS 1083 requires that the strength developed at 7 days by the cubes made with the unwashed sand must be at least 85% of the cubes made with the washed aggregate.

The test sample met this requirement, giving a result of 97.5 %.

Soluble Sulphates:

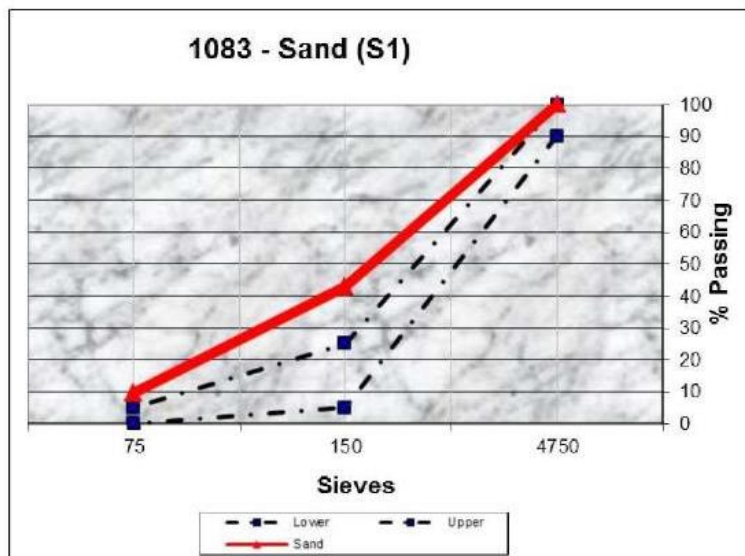
The commentary on SABS 1083:1994 states that it is difficult to define a limit to the level of sulphate or sulphide that can be regarded as tolerable in aggregate.

SABS 0100-2:1990 limits the total water-soluble sulphate content of concrete to 4% of the mass of cement.

The test results given above for the sample tested would be within this limit for most concrete mixes.

Gradings:

The sand did not meet the grading requirements for SANS 1083:2006.



Sieve Size (mm)	Cumulative % passing	
	Limits	Sample
6.7		100
4.750	90 - 100	100
2.360		100
1.180		100
0.600		100
0.300		99
0.150	5 - 25	43
0.075	0 - 5*	9.5

* 0 - 10 crushed rock

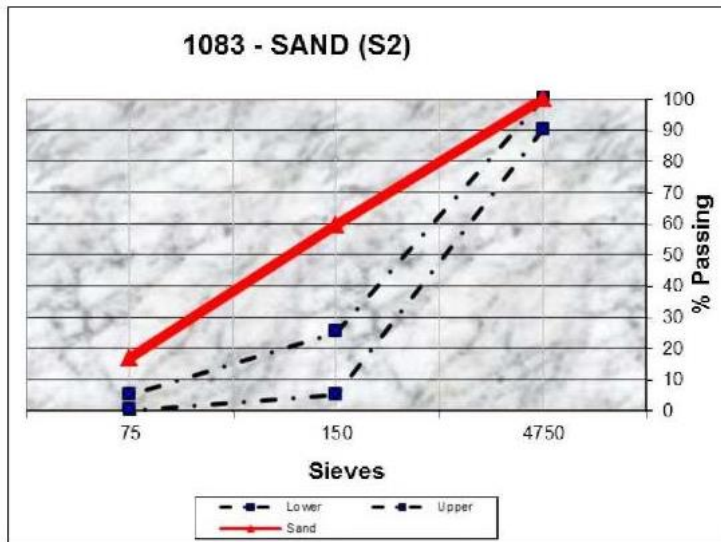
RD	0.00
LBD	0
CBD	0
FM	0.58
VOIDS	#DIV/0!

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Ref: CJ14/08/1004
 Client: Jeffares & Green
 Subject: Aggregate Testing Report
 Project: Tsitsa River

18 September 2014

The sand did not meet the grading requirements for SANS 1083:2006.

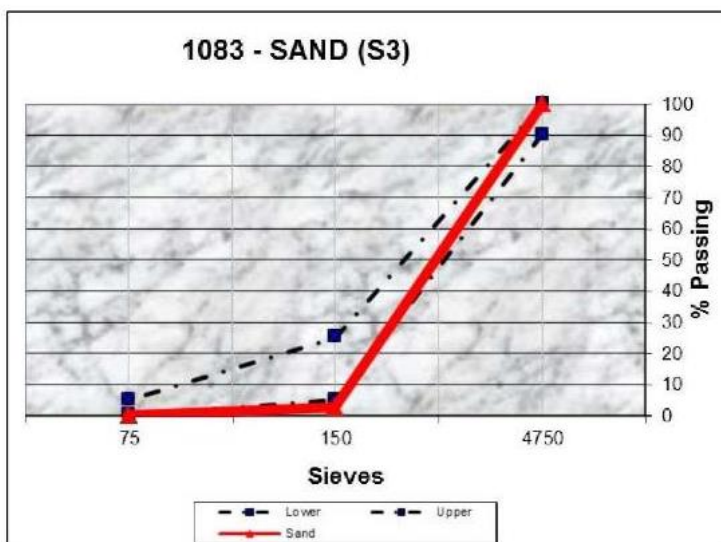


Sieve Size (mm)	Cumulative % passing	
	Limits	Sample
6.7		100
4.750	90 - 100	100
2.360		100
1.180		100
0.600		100
0.300		99
0.150	5 - 25	60
0.075	0 - 5*	16.9

* 0 - 10 crushed rock

RD	0.00
LBD	0
CBD	0
FM	0.42
VOIDS	#DIV/0!

The sand met the grading requirements for SANS 1083:2006, except for the 0.150mm sieve size.



Sieve Size (mm)	Cumulative % passing	
	Limits	Sample
6.7		100
4.750	90 - 100	100
2.360		99
1.180		97
0.600		93
0.300		69
0.150	5 - 25	3
0.075	0 - 5*	0.2

* 0 - 10 crushed rock

RD	0.00
LBD	0
CBD	0
FM	1.39
VOIDS	#DIV/0!

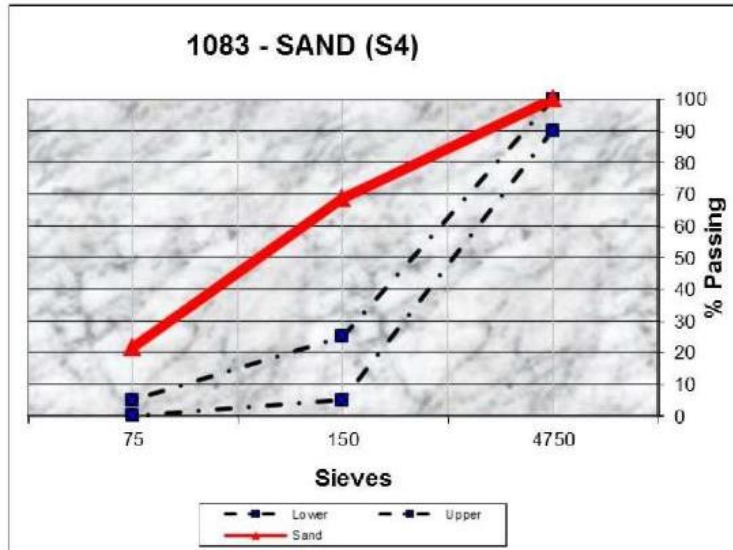
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The results relate only to the items tested

Ref: CJ14/08/1004
 Client: Jeffares & Green
 Subject: Aggregate Testing Report
 Project: Tsitsa River

18 September 2014

The sand did not meet the grading requirements for SANS 1083:2006.

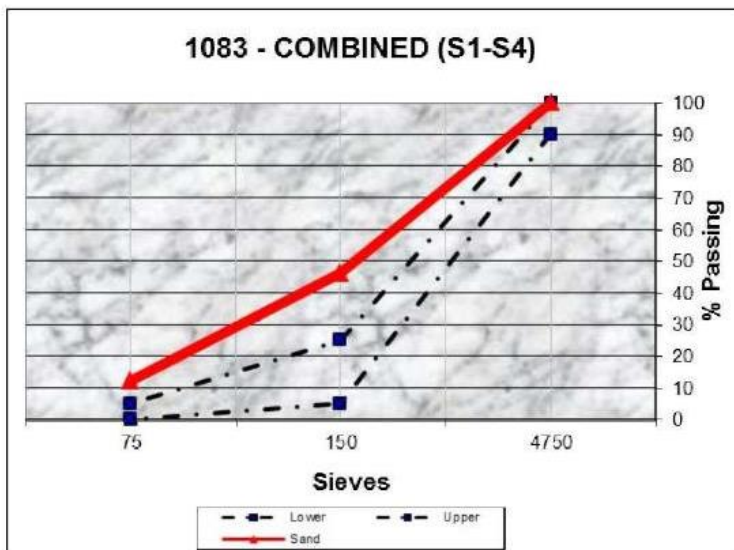


Sieve Size (mm)	Cumulative % passing	
	Limits	Sample
6.7		100
4.750	90 - 100	100
2.360		100
1.180		100
0.600		100
0.300		100
0.150	5 - 25	68
0.075	0 - 5*	21.6

* 0 - 10 crushed rock

RD	0.00
LBD	0
CBD	0
FM	0.32
VOIDS	#DIV/0!

The sand did not meet the grading requirements for SANS 1083:2006.



Sieve Size (mm)	Cumulative % passing	
	Limits	Sample
6.7		100
4.750	90 - 100	100
2.360		100
1.180		99
0.600		99
0.300		93
0.150	5 - 25	46
0.075	0 - 5*	12.2

* 0 - 10 crushed rock

RD	0.00
LBD	0
CBD	0
FM	0.63
VOIDS	#DIV/0!

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Ref: CJ14/08/1004

18 September 2014

Client: Jeffares & Green

Subject: Aggregate Testing Report

Project: Tsitsa River



R J L Raw
B Tech (Civil Eng)

CONTEST CONCRETE TECHNOLOGY SERVICES



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 Concrete Technology Services

APPENDIX A

Client: Jeffares & Green	Sample number						
	1004/1	1004/2	1004/3	1004/4	1004/5		
Job no. CJ14/08/1004							
	S1	S2	S3	S4	COMBINED (S1 S4)		
Tests	Aggregates						
Chloride content (%)	-	-	-	-	0.01		
Grading	See report	See report	See report	See report	See report		
Organic impurities	-	-	-	-	similar to reference		
pH	-	-	-	-	7.86		
Soluble deleterious impurities	-	-	-	-	see below		
Soluble sulphates (mg/l)	-	-	-	-	<10		

Soluble deleterious impurities

Sample	Age	Unwashed	Washed	Unwashed as a % of washed
Combined Sand	24 hr	1.5	1.5	100
	3d	4.4	5.0	88
	7d	7.8	8.0	97.5
	28d			

R J L RAW
 B Tech (Civil Eng)

E4:

INDICATOR AND COMPACTION TESTING – CORE AND SHELL MATERIALS

36 Fourth Street, Booyens Reserve, Johannesburg 2091
 PO Box 82223, Southdale 2135
 Tel: +27 (0)11 835 3117 • Fax: +27 (0)11 835 2503
 E-mail: jhb@civilab.co.za • Website: www.civilab.co.za



T0062

Civilab

Civil Engineering Testing Laboratories

Client : JEFFARES & GREEN CONSULTING ENGINEERS
Address : P O BOX 1109
 : SUNNINGHILL
 : 2157

Client Reference :
Order No. : Jaco Oliver

Attention :
Facsimile : 011 807 1607
E-mail : chettyn@jgi.co.za

Date Received : 12/08/2014
Date Tested : 12/08/2014 - current
Date Reported : 22/09/2014

Project : Mzimvubu Water Project
Project No. : 2014-B-1687

Page : 1 of 14

Herewith please find the test report(s) pertaining to the above project. All tests were conducted in accordance with prescribed test method(s). Information herein consists of the following:

Test(s) conducted / Item(s) measured	Qty.	Test Method(s)	Authorized By	Page(s)
Atterberg Limits < 0.425mm	22.000	TMH1 A2, A3, A4	J Marques	2-12
Sieve Analysis 0.075mm (Mass Grading)	22.000	TMH1 A1	J Marques	2-12
Hydrometer Analysis	22.000	ASTM D422	J Marques	2-12
MDD & OMC	2.000	TMH1 A7	J Marques	13-14

Any test results contained in this report and marked with * in the table above are "not SANAS accredited" and are not included in the schedule of accreditation for this laboratory.

Any information contained in this test report pertain only to the areas and/or samples tested. Documents may only be reproduced or published in their full context.

While every care is taken to ensure that all tests are carried out in accordance with recognised standards, neither Civilab (Proprietary) Limited nor its employees shall be liable in any way whatsoever for any error made in the execution or reporting of tests or any erroneous conclusions drawn therefrom or for any consequences thereof.

All interpretations, Interpolations, Opinions and/or Classifications contained in this report falls outside our scope of accreditation.

The following parameters, where applicable, were excluded from the classification procedure: Chemical modifications, Additional fines, Fractured Faces, Soluble Salts, pH, Conductivity, Coarse Sand Ratio, Durability (COLTO: G4-G9).

The following parameters, where applicable, were assumed: Rock types were assumed to be of an Arenaceous nature with Siliceous cementing material.

Unless otherwise requested or stated, all samples will be discarded after a period of 3 months.

Deviations in Test Methods:

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 E-mail: jhb@civilab.co.za • Website: www.civilab.co.za



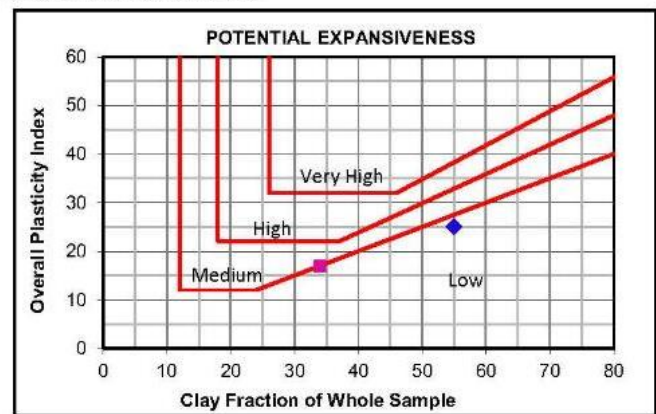
T0062

Civil Engineering Testing Laboratories

Client	: JEFFARES & GREEN CONSULTING ENGINEERS	Date Received:	12/08/2014
Project	: Mzimvubu Water Project	Date Reported:	22/09/2014
Project No	: 2014-B-1687	Page No.:	2 of 14

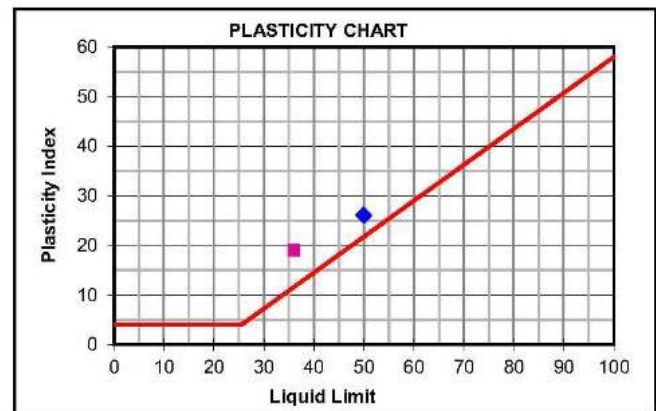
FOUNDATION INDICATOR

Laboratory Number	1	2
Field Number	CTP 1	CTP 2
Client Reference		
Depth (m)	1.1-2.4	1.0-1.9
Position		
Coordinates	X	
	Y	
Description		
Additional Information		
Calcrete / Crushed Stabilizing Agent		



Moisture Content & Relative Density-TMH1 Metod A12T

Moisture Content (%)		
Relative Density (S.G.)		



Sieve Analysis (Wet Preparation) - TMH1 Method A1(a)

Percentage Passing	75.0 mm	100	100
	63.0 mm	100	100
	53.0 mm	100	100
	37.5 mm	100	100
	26.5 mm	100	100
	19.0 mm	100	100
	13.2 mm	100	100
	4.75 mm	98	99
	2.00 mm	97	95
	0.425 mm	96	92
	0.075 mm	81	74
Grading Modulus		0.26	0.39

Atterberg Limits - TMH1 Method A2, A3 & A4

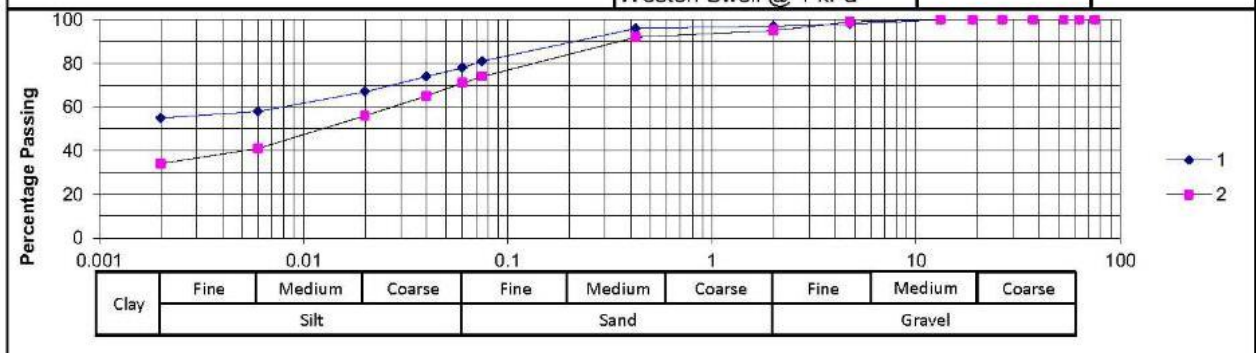
Laboratory Number	1	2
Liquid Limit (%)	50	36
Plasticity Index (%)	26	19
Linear Shrinkage (%)	12.0	9.5
Overall PI (%)	25	17

Hydrometer Analysis - ASTM Method D422

Percentage Passing	0.060 mm	78	71
	0.040 mm	74	65
	0.020 mm	67	56
	0.006 mm	58	41
	0.002 mm	55	34
Gravel (%)		3	5
Sand (%)		19	24
Silt (%)		23	37
Clay (%)		55	34

Classifications

HRB	A-7-6(20)	A-6(12)
Unified	CH	CL
Weston Swell @ 1 kPa		



36 Fourth Street, Booyens Reserve, Johannesburg 2091
 PO Box 82223, Southdale 2135
 Tel: +27 (0)11 835 3117•Fax: +27 (0)11 835 2503
 E-mail: jhb@civilab.co.za•Website: www.civilab.co.za

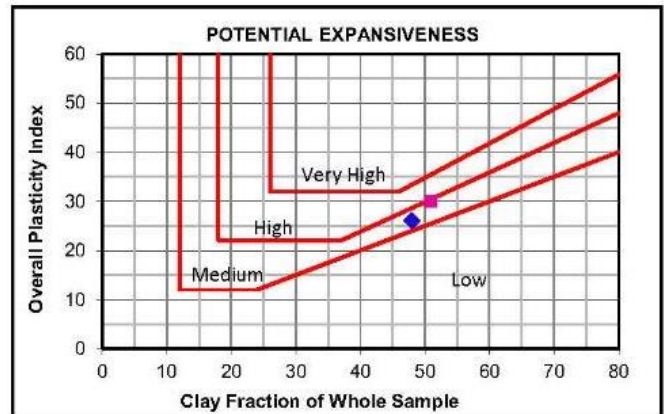


Civil Engineering Testing Laboratories

Client :	JEFFARES & GREEN CONSULTING ENGINEERS	Date Received:	12/08/2014
Project :	Mzimvubu Water Project	Date Reported:	22/09/2014
Project No :	2014-B-1687	Page No. :	3 of 14

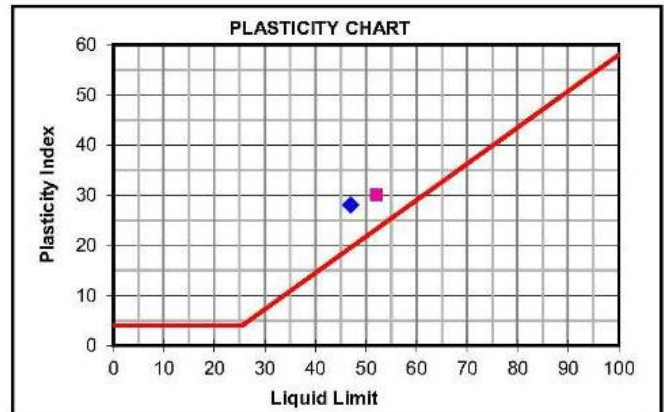
FOUNDATION INDICATOR

Laboratory Number	3	4
Field Number	CTP 2	CTP 3
Client Reference		
Depth (m)	1.9-2.8	0.8-2.5
Position		
Coordinates	X	
	Y	
Description		
Additional Information		
Calcrete / Crushed Stabilizing Agent		



Moisture Content & Relative Density-TMH1 Method A12T

Moisture Content (%)		
Relative Density (S.G.)		



Sieve Analysis (Wet Preparation) - TMH1 Method A1(a)

Sieve Size (mm)	Sample 3 (%)	Sample 4 (%)
75.0	100	100
63.0	100	100
53.0	100	100
37.5	100	100
26.5	100	100
19.0	100	100
13.2	100	100
4.75	99	100
2.00	97	100
0.425	94	100
0.075	75	90
Grading Modulus	0.34	0.1

Laboratory Number	3	4
-------------------	---	---

Atterberg Limits - TMH1 Method A2, A3 & A4

Liquid Limit (%)	47	52
Plasticity Index (%)	28	30
Linear Shrinkage (%)	12.5	12.5
Overall PI (%)	26	30

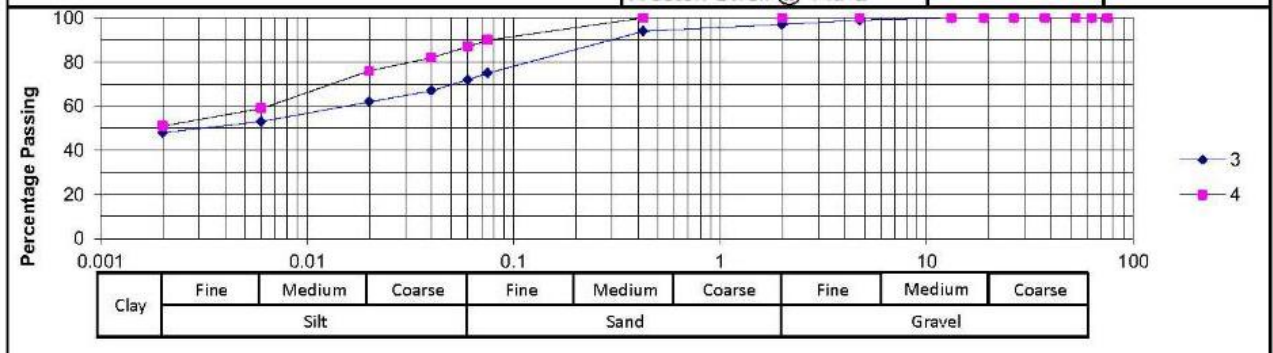
Hydrometer Analysis - ASTM Method D422

Sieve Size (mm)	Sample 3 (%)	Sample 4 (%)
0.060	72	87
0.040	67	82
0.020	62	76
0.006	53	59
0.002	48	51

Gravel (%)	3	
Sand (%)	25	13
Silt (%)	24	36
Clay (%)	48	51

Classifications

HRB	A-7-6(20)	A-7-6(20)
Unified	CL	CH
Weston Swell @ 1 kPa		



36 Fourth Street, Booyens Reserve, Johannesburg 2091
 PO Box 82223, Southdale 2135
 Tel: +27 (0)11 835 3117 • Fax: +27 (0)11 835 2503
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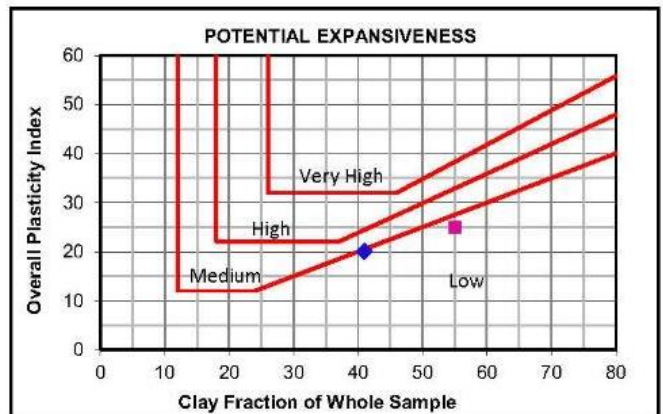


Civil Engineering Testing Laboratories

Client	: JEFFARES & GREEN CONSULTING ENGINEERS	Date Received:	12/08/2014
Project	: Mzimvubu Water Project	Date Reported:	22/09/2014
Project No	: 2014-B-1687	Page No.:	4 of 14

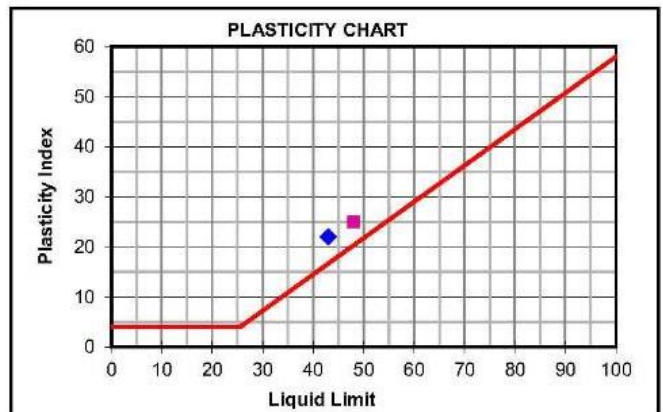
FOUNDATION INDICATOR

Laboratory Number	5	6
Field Number	CTP 4	CTP 4
Client Reference		
Depth (m)	0.4-1.0	1.0-1.8
Position		
Coordinates	X	
	Y	
Description		
Additional Information		
Calcrete / Crushed Stabilizing Agent		



Moisture Content & Relative Density-TMH1 Metod A12T

Moisture Content (%)		
Relative Density (S.G.)		



Sieve Analysis (Wet Preparation) - TMH1 Method A1(a)

Percentage Passing	75.0 mm	100	100
	63.0 mm	100	100
	53.0 mm	100	100
	37.5 mm	100	100
	26.5 mm	100	100
	19.0 mm	100	100
	13.2 mm	100	100
	4.75 mm	99	100
	2.00 mm	94	100
	0.425 mm	91	99
	0.075 mm	76	84
Grading Modulus		0.39	0.17

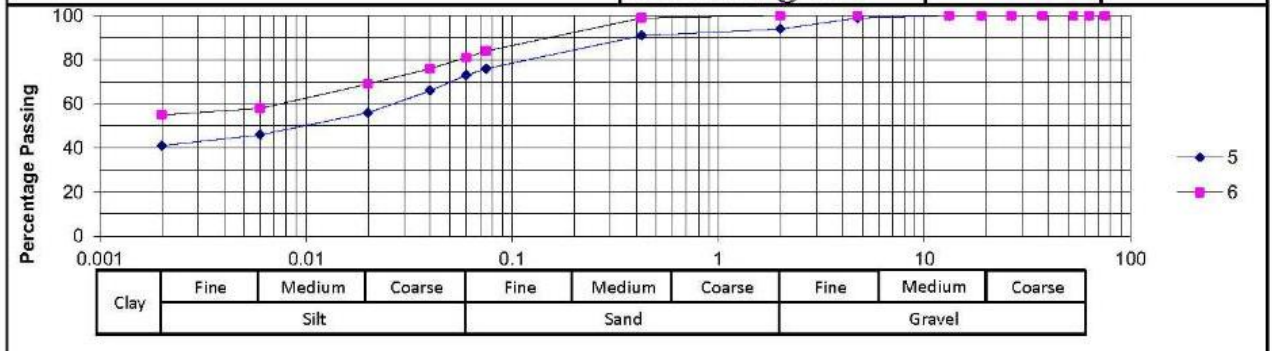
Hydrometer Analysis - ASTM Method D422

Percentage Passing	0.060 mm	73	81
	0.040 mm	66	76
	0.020 mm	56	69
	0.006 mm	46	58
	0.002 mm	41	55
Gravel	%	6	
Sand	%	21	19
Silt	%	32	26
Clay	%	41	55

Laboratory Number	5	6	
Atterberg Limits - TMH1 Method A2, A3 & A4			
Liquid Limit	%	43	48
Plasticity Index	%	22	25
Linear Shrinkage	%	9.5	10.5
Overall PI	%	20	25

Classifications

HRB	A-7-6(16)	A-7-6(20)
Unified	CL	CL
Weston Swell @ 1 kPa		



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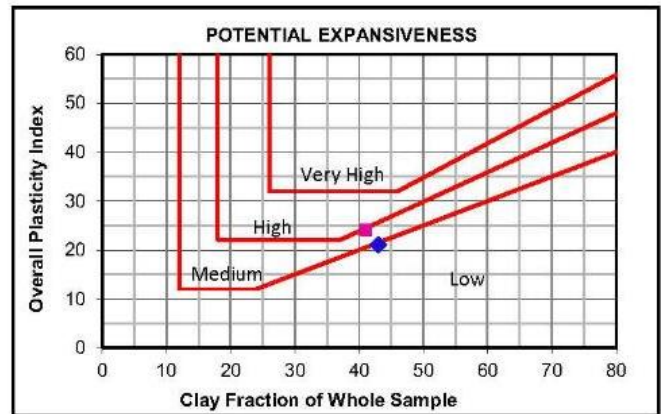


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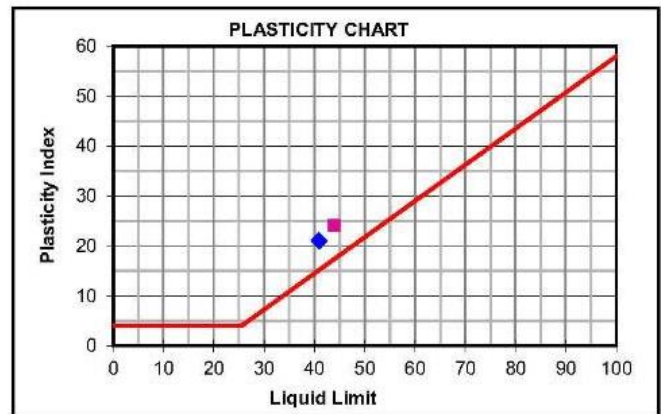
FOUNDATION INDICATOR

Laboratory Number	7	8
Field Number	CTP 5	CTP 6
Client Reference		
Depth (m)	1.1-2.5	1.0-2.7
Position		
Coordinates	X	
	Y	
Description		
Additional Information		
Calcrete / Crushed Stabilizing Agent		



Moisture Content & Relative Density-TMH1 Metod A12T

Moisture Content (%)		
Relative Density (S.G.)		



Sieve Analysis (Wet Preparation) - TMH1 Method A1(a)

Percentage Passing	75.0 mm	100	100
	63.0 mm	100	100
	53.0 mm	100	100
	37.5 mm	100	100
	26.5 mm	100	100
	19.0 mm	100	100
	13.2 mm	100	100
	4.75 mm	100	100
	2.00 mm	100	100
	0.425 mm	100	100
	0.075 mm	88	83
Grading Modulus		0.12	0.17

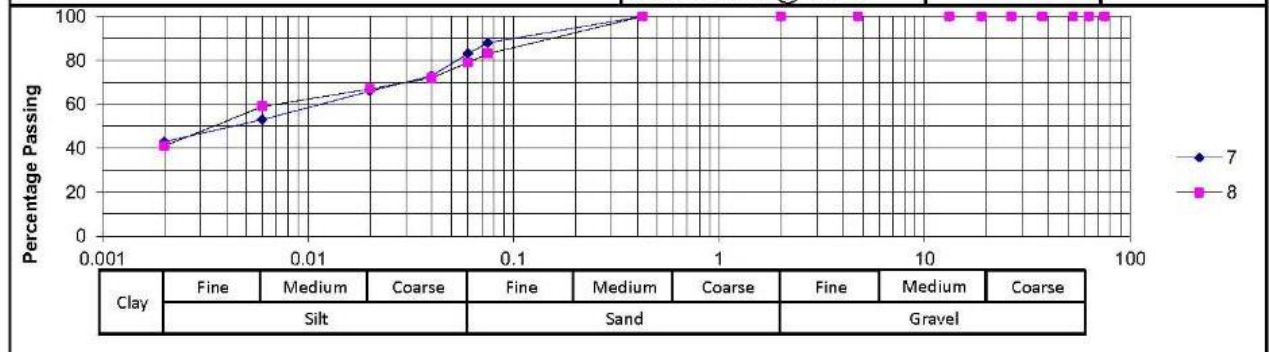
Laboratory Number	7	8
Atterberg Limits - TMH1 Method A2, A3 & A4		
Liquid Limit %	41	44
Plasticity Index %	21	24
Linear Shrinkage %	8.0	11.5
Overall PI %	21	24

Hydrometer Analysis - ASTM Method D422

Percentage Passing	0.060 mm	83	79
	0.040 mm	73	72
	0.020 mm	66	67
	0.006 mm	53	59
	0.002 mm	43	41
Gravel %			
Sand %		17	21
Silt %		40	38
Clay %		43	41

Classifications

HRB	A-7-6(19)	A-7-6(20)
Unified	CL	CL
Weston Swell @ 1 kPa		



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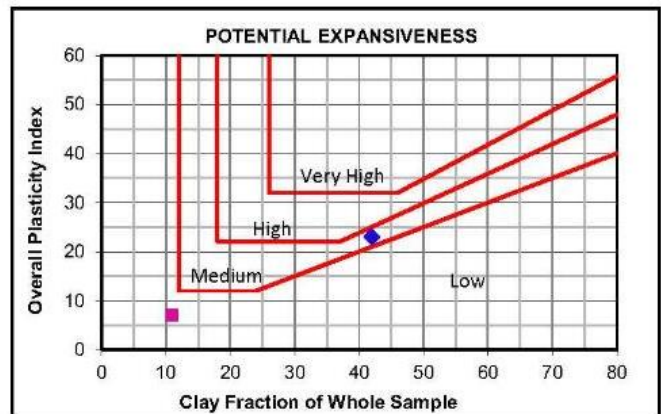


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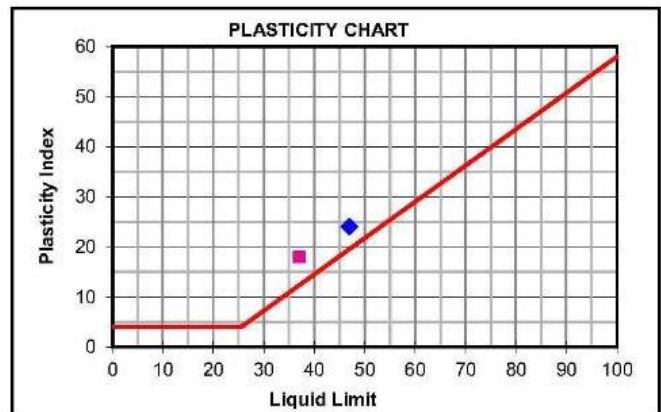
FOUNDATION INDICATOR

Laboratory Number	9	10
Field Number	CTP 7	FTP 1
Client Reference		
Depth (m)	0.7-2.5	0.2-0.7
Position		
Coordinates	X	
	Y	
Description		
Additional Information		
Calcrete / Crushed Stabilizing Agent		



Moisture Content & Relative Density - TMH1 Method A12T

Moisture Content (%)		
Relative Density (S.G.)		



Sieve Analysis (Wet Preparation) - TMH1 Method A1(a)

Percentage Passing	75.0 mm	100	100
	63.0 mm	100	100
	53.0 mm	100	100
	37.5 mm	100	92
	26.5 mm	100	81
	19.0 mm	100	69
	13.2 mm	100	66
	4.75 mm	99	54
	2.00 mm	97	45
	0.425 mm	94	38
	0.075 mm	81	30
Grading Modulus		0.28	1.87

Atterberg Limits - TMH1 Method A2, A3 & A4

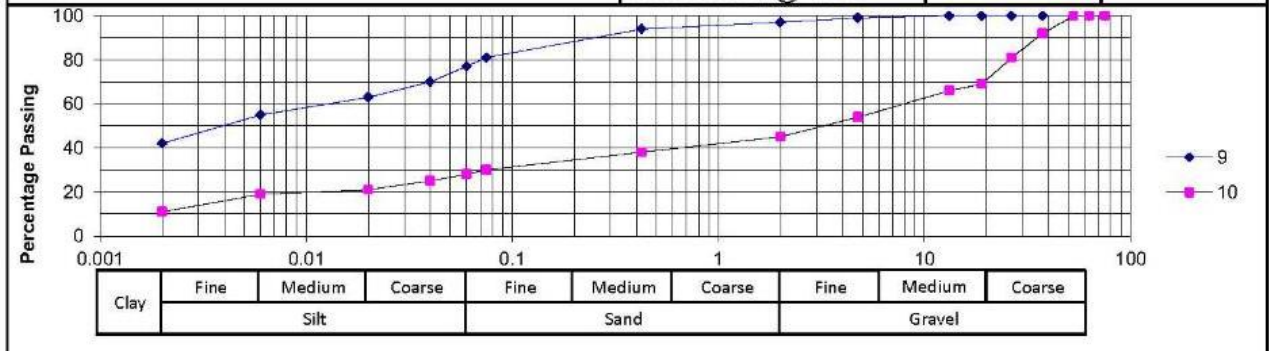
Laboratory Number	9	10
Liquid Limit (%)	47	37
Plasticity Index (%)	24	18
Linear Shrinkage (%)	10.5	9.5
Overall PI (%)	23	7

Hydrometer Analysis - ASTM Method D422

Percentage Passing	0.060 mm	77	28
	0.040 mm	70	25
	0.020 mm	63	21
	0.006 mm	55	19
	0.002 mm	42	11
Gravel (%)		3	55
Sand (%)		20	17
Silt (%)		35	17
Clay (%)		42	11

Classifications

HRB	A-7-6(20)	A-2-6(1)
Unified	CL	GC
Weston Swell @ 1 kPa		



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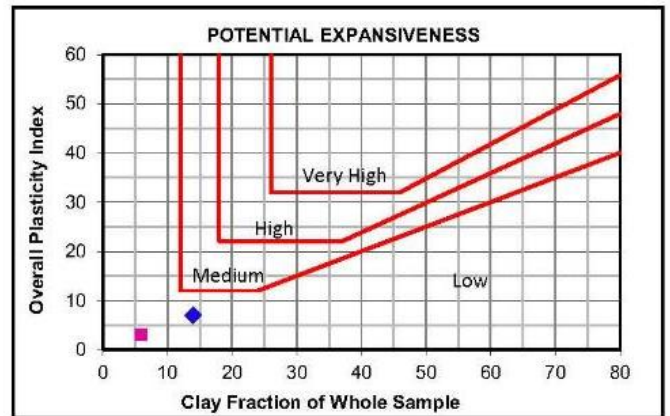


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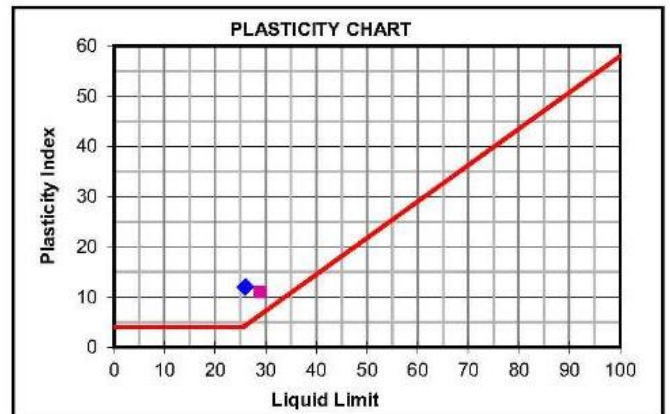
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FOUNDATION INDICATOR

Laboratory Number	11	12
Field Number	FTP 3	FTP 4
Client Reference		
Depth (m)	1.0-1.6	0.2-0.9
Position		
Coordinates	X	
	Y	
Description		
Additional Information		
Calcrete / Crushed Stabilizing Agent		



Moisture Content & Relative Density-TMH1 Method A12T		
Moisture Content (%)		
Relative Density (S.G.)		



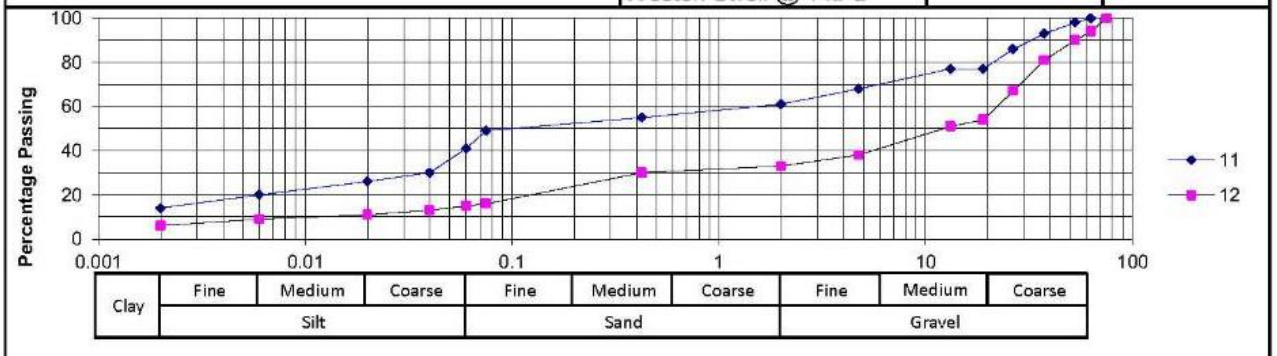
Sieve Analysis (Wet Preparation) - TMH1 Method A1(a)			
Percentage Passing	75.0 mm	100	100
	63.0 mm	100	94
	53.0 mm	98	90
	37.5 mm	93	81
	26.5 mm	86	67
	19.0 mm	77	54
	13.2 mm	77	51
	4.75 mm	68	38
	2.00 mm	61	33
	0.425 mm	55	30
0.075 mm	49	16	
Grading Modulus	1.35	2.21	

Laboratory Number	11	12
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Hydrometer Analysis - ASTM Method D422			
Percentage Passing	0.060 mm	41	15
	0.040 mm	30	13
	0.020 mm	26	11
	0.006 mm	20	9
	0.002 mm	14	6
Gravel	%	39	67
Sand	%	20	18
Silt	%	27	9
Clay	%	14	6

Atterberg Limits - TMH1 Method A2, A3 & A4			
Liquid Limit	%	26	29
Plasticity Index	%	12	11
Linear Shrinkage	%	6.0	6.0
Overall PI	%	7	3

Classifications		
HRB	A-6(3)	A-2-6(0)
Unified	GC	GC
Weston Swell @ 1 kPa		



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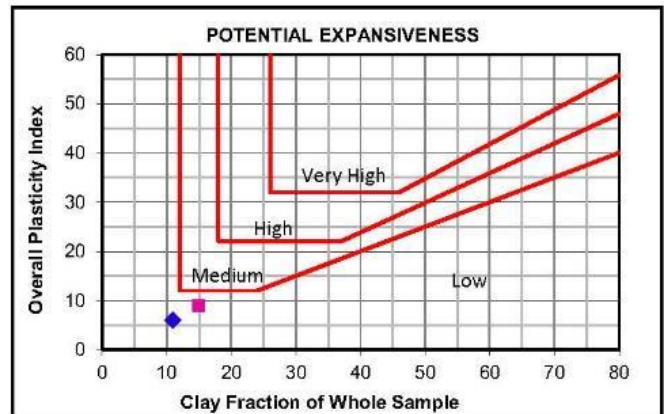


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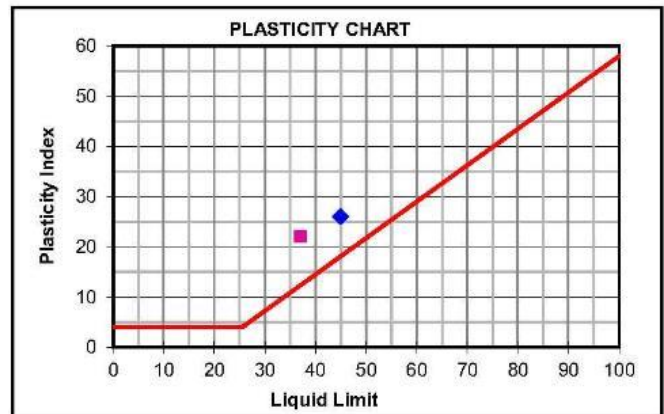
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FOUNDATION INDICATOR

Laboratory Number	13	14
Field Number	FTP 6	FTP 7
Client Reference		
Depth (m)	1.4-1.9	0.6-0.9
Position		
Coordinates	X	
	Y	
Description		
Additional Information		
Calcrete / Crushed		
Stabilizing Agent		



Moisture Content & Relative Density-TMH1 Metod A12T		
Moisture Content (%)		
Relative Density (S.G.)		

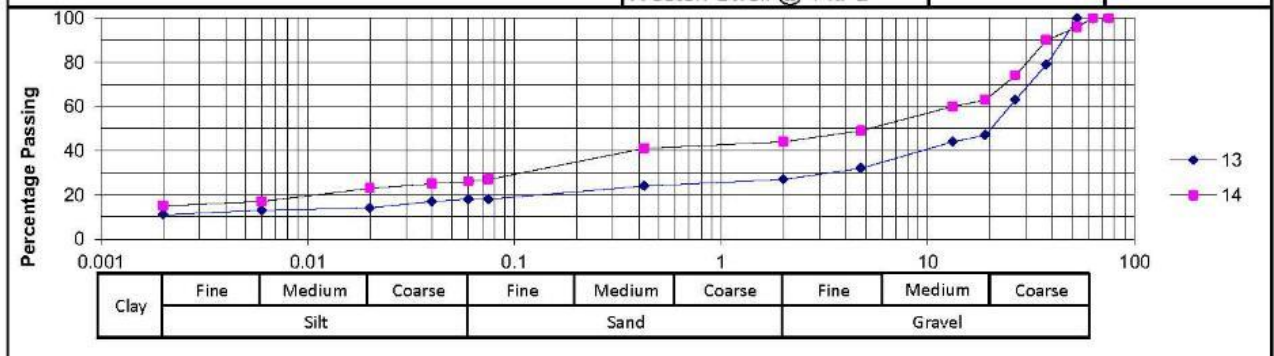


Sieve Analysis (Wet Preparation) - TMH1 Method A1(a)			
Percentage Passing	75.0 mm	100	100
	63.0 mm	100	100
	53.0 mm	100	96
	37.5 mm	79	90
	26.5 mm	63	74
	19.0 mm	47	63
	13.2 mm	44	60
	4.75 mm	32	49
	2.00 mm	27	44
	0.425 mm	24	41
0.075 mm	18	27	
Grading Modulus	2.31	1.88	

Laboratory Number	13	14
Atterberg Limits - TMH1 Method A2, A3 & A4		
Liquid Limit %	45	37
Plasticity Index %	26	22
Linear Shrinkage %	12.0	10.0
Overall PI %	6	9

Hydrometer Analysis - ASTM Method D422			
Percentage Passing	0.060 mm	18	26
	0.040 mm	17	25
	0.020 mm	14	23
	0.006 mm	13	17
	0.002 mm	11	15
Gravel %	73	56	
Sand %	9	18	
Silt %	7	11	
Clay %	11	15	

Classifications		
HRB	A-2-7(0)	A-2-6(1)
Unified	GC	GC
Weston Swell @ 1 kPa		



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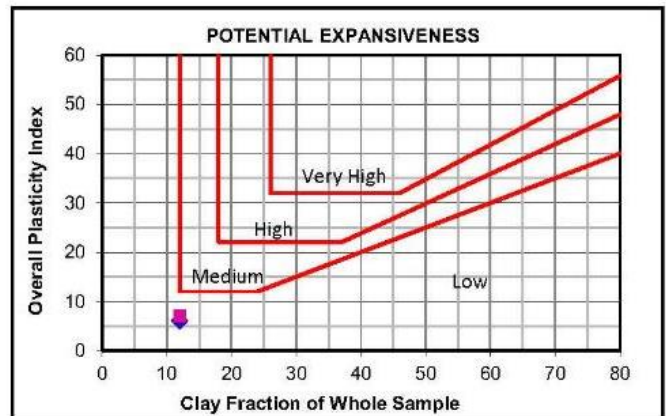


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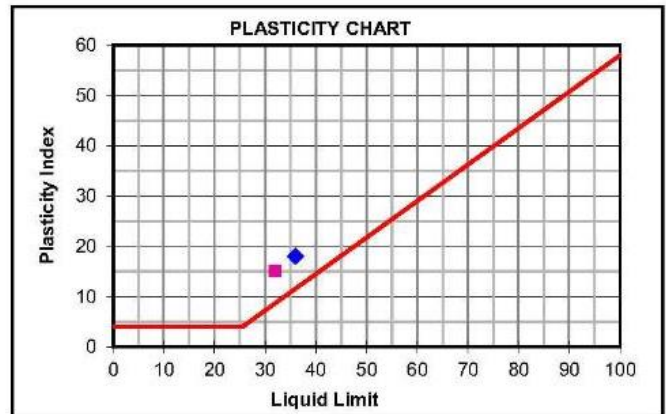
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FOUNDATION INDICATOR

Laboratory Number	15	16
Field Number	FTP 9	FTP 10
Client Reference		
Depth (m)	0.4-0.6	0.2-0.6
Position		
Coordinates	X Y	
Description		
Additional Information		
Calcrete / Crushed Stabilizing Agent		



Moisture Content & Relative Density-TMH1 Method A12T		
Moisture Content (%)		
Relative Density (S.G.)		

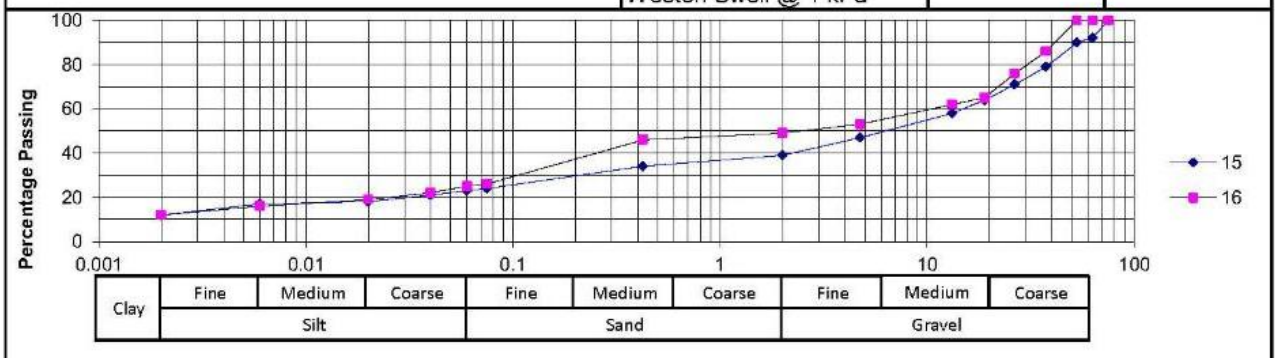


Sieve Analysis (Wet Preparation) - TMH1 Method A1(a)			
Percentage Passing			
	75.0 mm	100	100
	63.0 mm	92	100
	53.0 mm	90	100
	37.5 mm	79	86
	26.5 mm	71	76
	19.0 mm	64	65
	13.2 mm	58	62
	4.75 mm	47	53
	2.00 mm	39	49
	0.425 mm	34	46
	0.075 mm	24	26
Grading Modulus		2.03	1.79

Laboratory Number	15	16	
Atterberg Limits - TMH1 Method A2, A3 & A4			
Liquid Limit	%	36	32
Plasticity Index	%	18	15
Linear Shrinkage	%	9.5	7.5
Overall PI	%	6	7

Hydrometer Analysis - ASTM Method D422			
Percentage Passing			
	0.060 mm	23	25
	0.040 mm	21	22
	0.020 mm	18	19
	0.006 mm	17	16
	0.002 mm	12	12
Gravel	%	61	51
Sand	%	16	24
Silt	%	11	13
Clay	%	12	12

Classifications		
HRB	A-2-6(1)	A-2-6(1)
Unified	GC	GC
Weston Swell @ 1 kPa		



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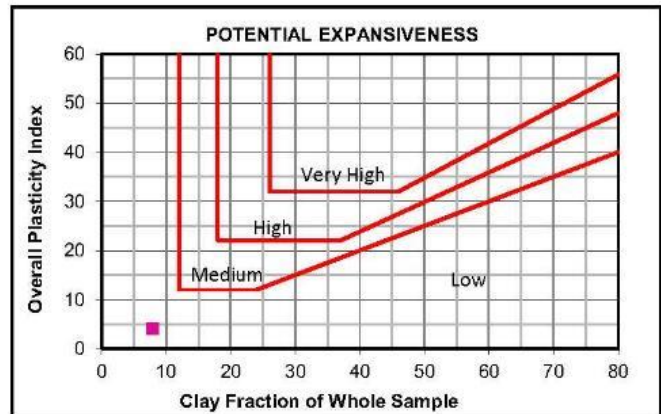


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Project :	Mzimvubu Water Project	Date Reported:	22/09/2014
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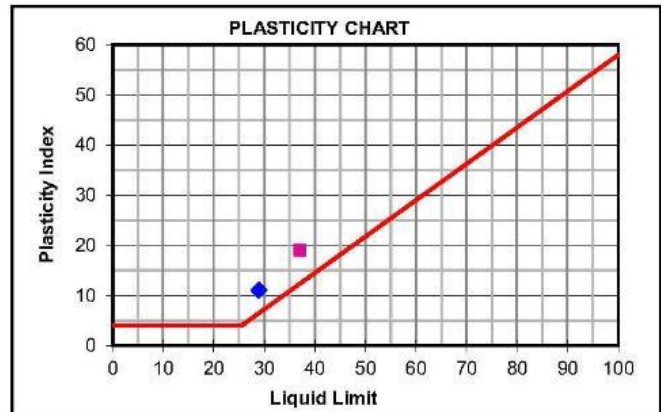
FOUNDATION INDICATOR

Laboratory Number	17	18
Field Number	FTP 11	FTP 12
Client Reference		
Depth (m)	0.2-0.7	0.3-0.6
Position		
Coordinates	X	
	Y	
Description		
Additional Information		
Calcrete / Crushed Stabilizing Agent		



Moisture Content & Relative Density-TMH1 Metod A12T

Moisture Content (%)	
Relative Density (S.G.)	



Sieve Analysis (Wet Preparation) - TMH1 Method A1(a)

Percentage Passing	75.0 mm	100
	63.0 mm	96
	53.0 mm	90
	37.5 mm	77
	26.5 mm	64
	19.0 mm	51
	13.2 mm	50
	4.75 mm	39
	2.00 mm	29
	0.425 mm	22
	0.075 mm	17
Grading Modulus		2.32

Laboratory Number	17	18
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Atterberg Limits - TMH1 Method A2, A3 & A4

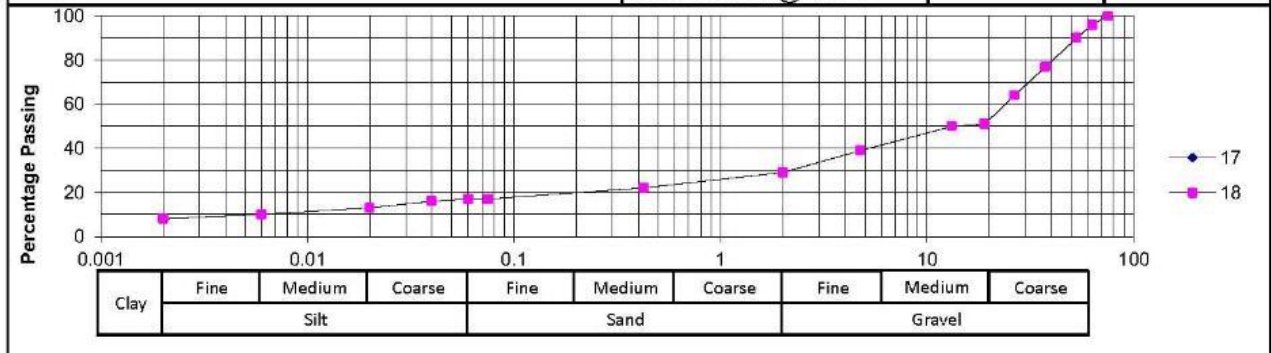
Liquid Limit	%	29	37
Plasticity Index	%	11	19
Linear Shrinkage	%	5.5	10.0
Overall PI	%	8	4

Hydrometer Analysis - ASTM Method D422

Percentage Passing	0.060 mm	17
	0.040 mm	16
	0.020 mm	13
	0.006 mm	10
	0.002 mm	8
Gravel	%	71
Sand	%	12
Silt	%	9
Clay	%	8

Classifications

HRB		A-2-6(0)
Unified		GC
Weston Swell @ 1 kPa		



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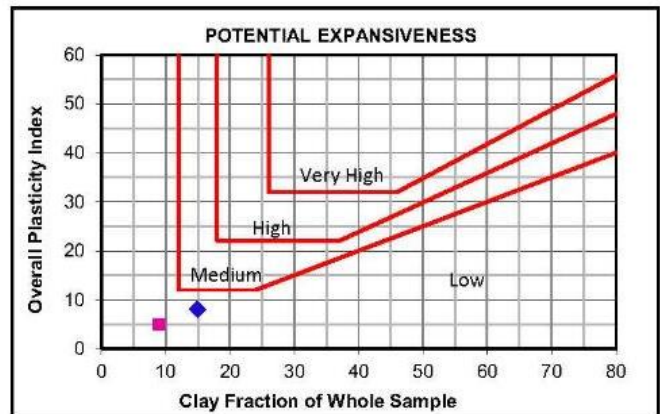


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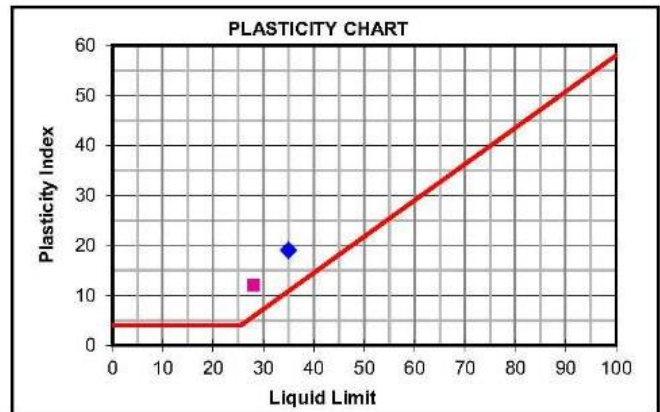
FOUNDATION INDICATOR

Laboratory Number	19	20
Field Number	FTP 13	FTP 5
Client Reference		
Depth (m)	0.3-0.6	0.6-1.1
Position		
Coordinates	X	
	Y	
Description		
Additional Information		
Calcrete / Crushed Stabilizing Agent		



Moisture Content & Relative Density - TMH1 Method A12T

Moisture Content (%)		
Relative Density (S.G.)		



Sieve Analysis (Wet Preparation) - TMH1 Method A1(a)

Percentage Passing	75.0 mm	100	100
	63.0 mm	100	98
	53.0 mm	97	98
	37.5 mm	86	88
	26.5 mm	77	73
	19.0 mm	65	65
	13.2 mm	65	62
	4.75 mm	53	55
	2.00 mm	47	47
	0.425 mm	42	39
	0.075 mm	26	22
Grading Modulus		1.85	1.92

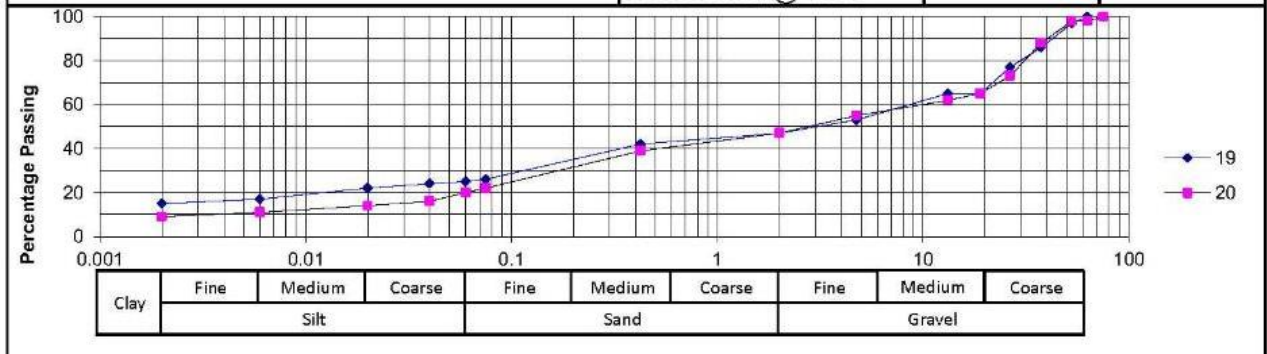
Laboratory Number	19	20
Atterberg Limits - TMH1 Method A2, A3 & A4		
Liquid Limit %	35	28
Plasticity Index %	19	12
Linear Shrinkage %	9.5	6.5
Overall PI %	8	5

Hydrometer Analysis - ASTM Method D422

Percentage Passing	0.060 mm	25	20
	0.040 mm	24	16
	0.020 mm	22	14
	0.006 mm	17	11
	0.002 mm	15	9
Gravel %		53	53
Sand %		22	27
Silt %		10	11
Clay %		15	9

Classifications

HRB	A-2-6(1)	A-2-6(0)
Unified	GC	GC
Weston Swell @ 1 kPa		



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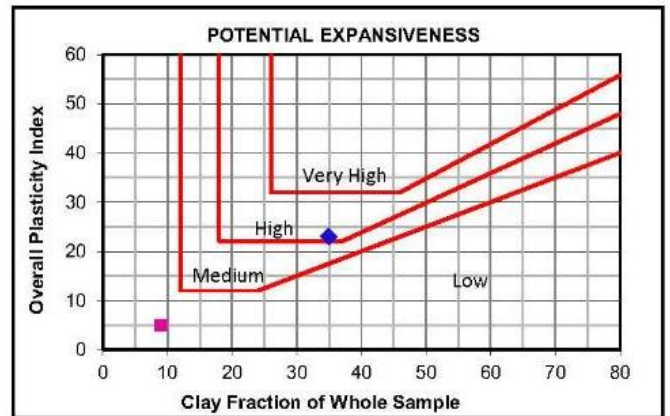


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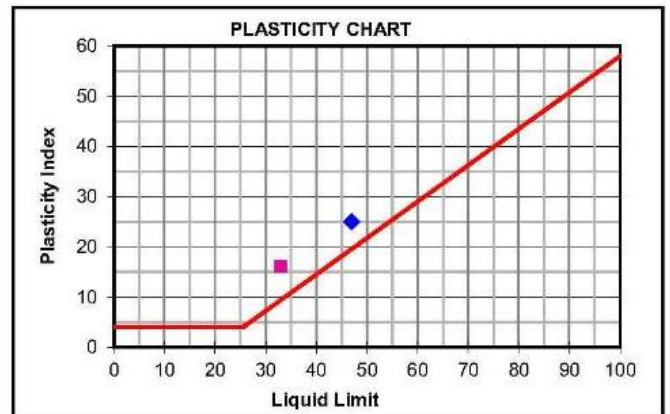
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FOUNDATION INDICATOR

Laboratory Number	21	22
Field Number	CTP :Mix	FTP :Mix
Client Reference		
Depth (m)		
Position		
Coordinates	X	
	Y	
Description		
Additional Information		
Calcrete / Crushed		
Stabilizing Agent		



Moisture Content & Relative Density-TMH1 Method A12T		
Moisture Content (%)		
Relative Density (S.G.)		

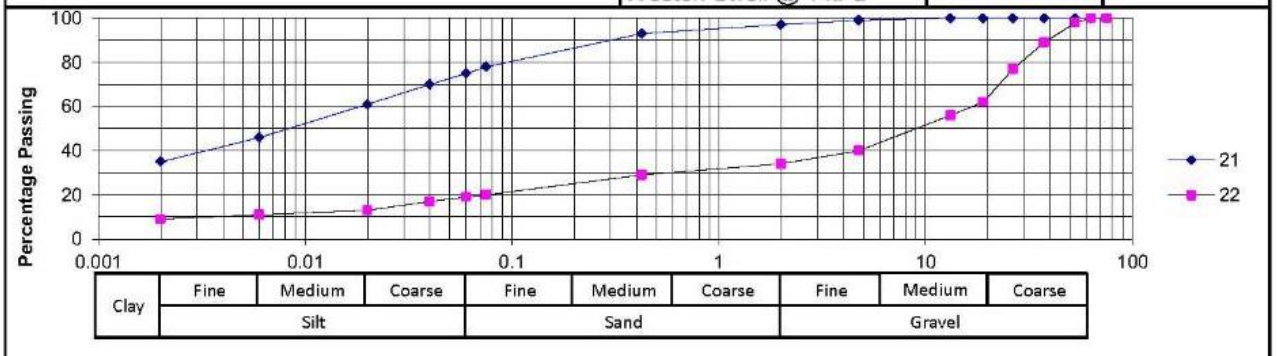


Sieve Analysis (Wet Preparation) - TMH1 Method A1(a)			
Percentage Passing	75.0 mm	100	100
	63.0 mm	100	100
	53.0 mm	100	98
	37.5 mm	100	89
	26.5 mm	100	77
	19.0 mm	100	62
	13.2 mm	100	56
	4.75 mm	99	40
	2.00 mm	97	34
	0.425 mm	93	29
0.075 mm	78	20	
Grading Modulus		0.32	2.17

Hydrometer Analysis - ASTM Method D422			
Percentage Passing	0.060 mm	75	19
	0.040 mm	70	17
	0.020 mm	61	13
	0.006 mm	46	11
	0.002 mm	35	9
Gravel	%	3	66
Sand	%	22	15
Silt	%	40	10
Clay	%	35	9

Laboratory Number	21	22	
Atterberg Limits - TMH1 Method A2, A3 & A4			
Liquid Limit	%	47	33
Plasticity Index	%	25	16
Linear Shrinkage	%	10.5	8.5
Overall PI	%	23	5

Classifications		
HRB	A-7-6(20)	A-2-6(0)
Unified	CL	GC
Weston Swell @ 1 kPa		



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MOISTURE DENSITY RELATIONSHIP

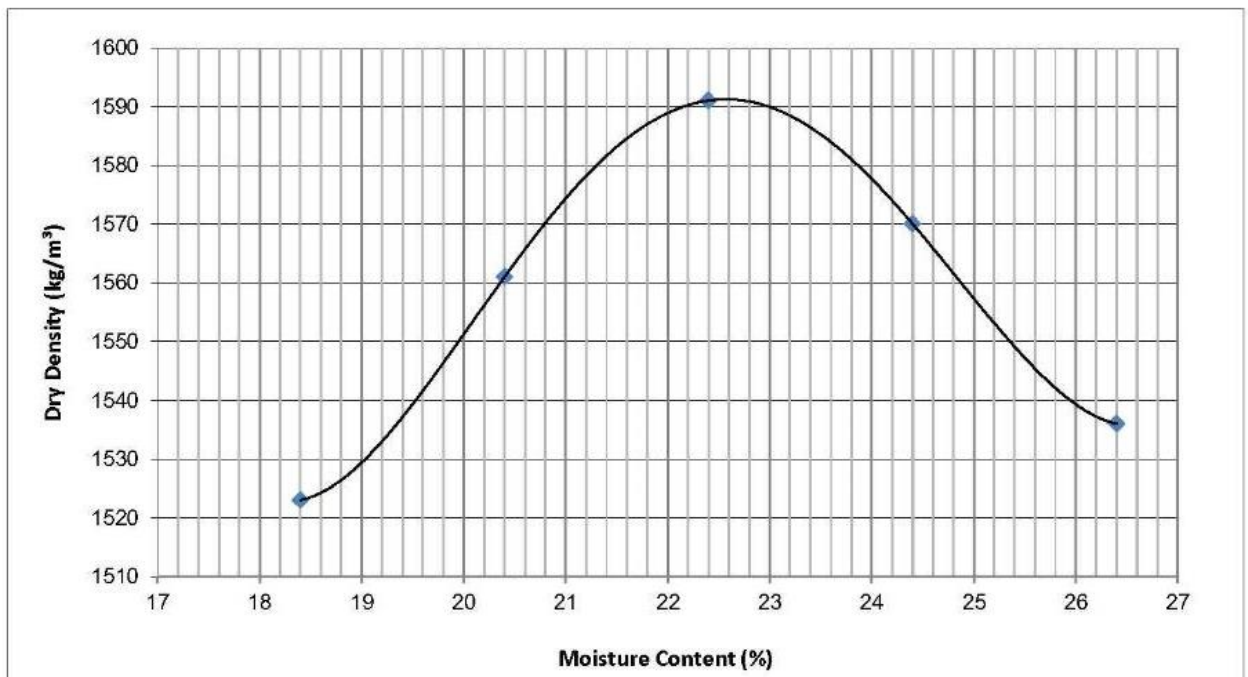
Laboratory Number	21		
Field Number	CTP :Mix		
Client Reference			
Depth (m)			
Position			
Coordinates	X		
	Y		
Description			
Additional Information			
Calcrete / Crushed Stabilizing Agent			

Maximum Dry Density & Optimum Moisture Content - TMH1 Method A7

Compactive Effort:	Standard Proctor		
--------------------	------------------	--	--

Dry Density	kg/m ³	1523	1561	1591	1570	1536	
Moisture Content	%	18.4	20.4	22.4	24.4	26.4	

Max. Dry Density	kg/m ³	1591
Optimum Moisture	%	22.6



36 Fourth Street, Boosens Reserve, Johannesburg 2091
 PO Box 82223, Southdale 2135
 Tel: +27 (0)11 835 3117 • Fax: +27 (0)11 835 2503
 E-mail: jhb@civilab.co.za • Website: www.civilab.co.za



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Client :	JEFFARES & GREEN CONSULTING EN	Date Received:	12/08/2014
Project :	Mzimvubu Water Project	Date Reported:	22/09/2014
Project No:	2014-B-1687	Page No. :	14 of 14

MOISTURE DENSITY RELATIONSHIP

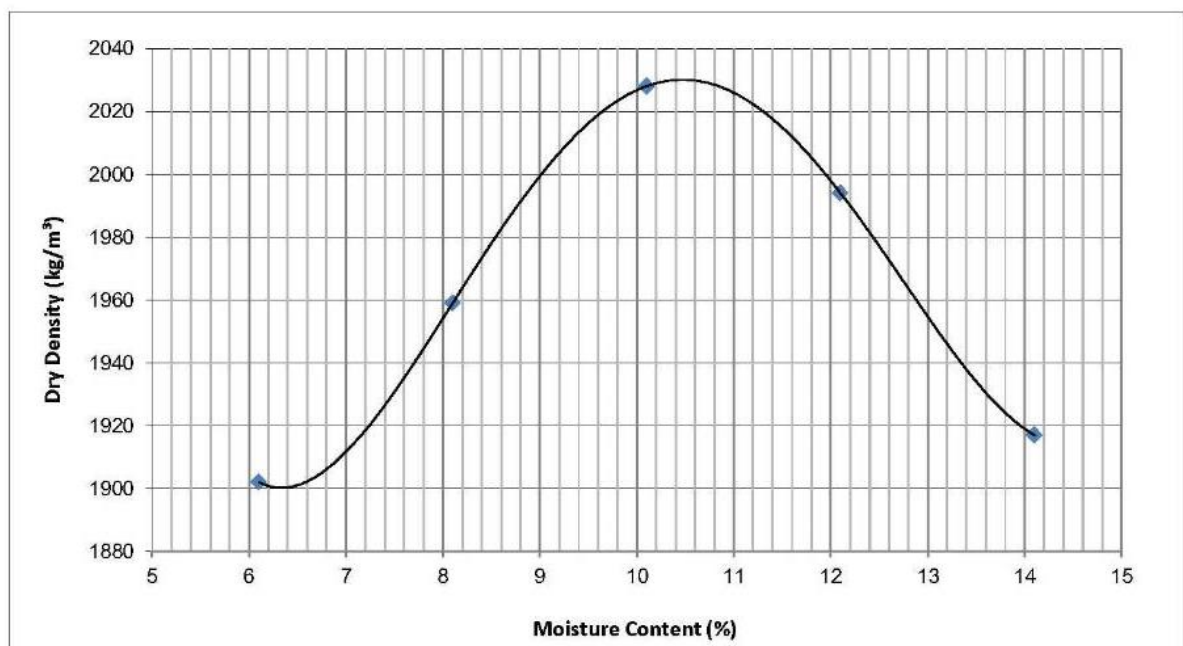
Laboratory Number	22		
Field Number	FTP :Mix		
Client Reference			
Depth (m)			
Position			
Coordinates	X		
	Y		
Description			
Additional Information			
Calcrete / Crushed			
Stabilizing Agent			

Maximum Dry Density & Optimum Moisture Content - TMH1 Method A7

Compactive Effort:	Standard Proctor
--------------------	------------------

Dry Density	kg/m ³	1902	1959	2028	1994	1917	
Moisture Content	%	6.1	8.1	10.1	12.1	14.1	

Max. Dry Density	kg/m ³	2030
Optimum Moisture	%	10.5



E5:

GEOTECHNICAL TESTING – CORE AND SHELL MATERIALS

36/38 Fourth Street, Booyens Reserve, Johannesburg 2091
 PO Box 82223, Southdale 2135
 Tel: +27 (0)11 8353117•Fax: +27 (0)11 835 2503
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Analyses on Potentially Dispersive Soils

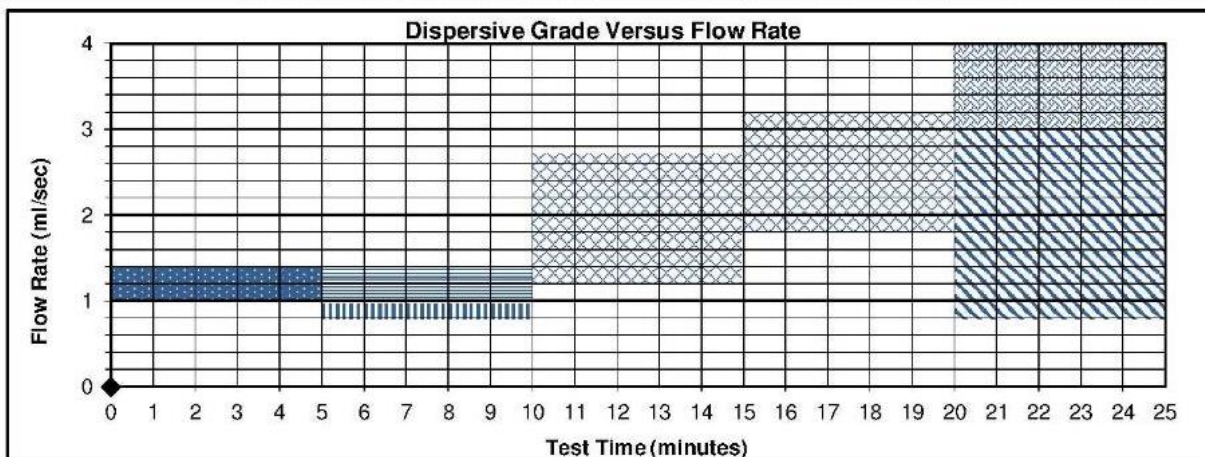
Project Name	MZIMVUBU WATER PROJECT	Lab. No.	1687-21
Job Number	2014-B-1687	Client/Field No.	CTP : MIX
Date Received	20/10/2014	Depth (m)	-

Pinhole Test in accordance with 6.2 of BS 1377:Part 5:1990

Parameters of Test Sample

Fraction tested		Compacted Density	kg/m ³	Bulk Dry	
Liquid Limit	%				
Plastic Limit	%			Moisture Content (%)	
Plasticity Index	%			Hole size after test (mm)	

Head (mm)	50					180					380					1020														
Time (min)	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25					
Flow Rate (ml/sec)																														
Effluent Water	Symbol: PC-Perfectly Clear					C-Clear					SD-Slightly Dark					MD-Moderately Dark					D-Dark					VD-Very Dark				

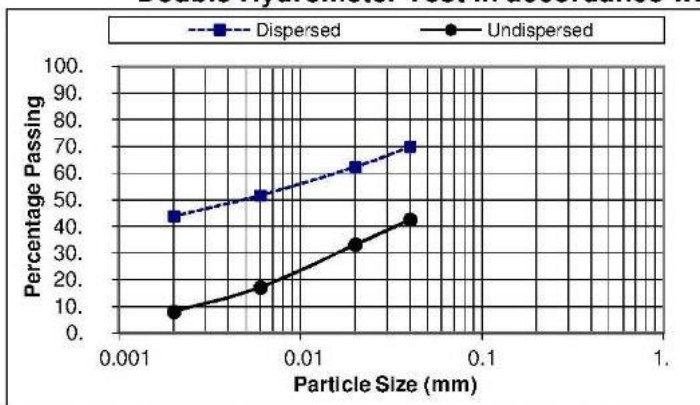


Dispersive Grade Index						Classification of test sample	
Dispersive	Intermediate		Non-dispersive			Not tested	
D1	D2	ND4	ND3	ND2	ND1		

Crumb Test in accordance with 6.3 of BS 1377:Part 5:1990

Reagent used		Classification of test sample	
Dispersive Grade Index		Not tested	
Non-dispersive	Dispersive		
1	2	3	4

Double Hydrometer Test in accordance with 6.4 of BS 1377:Part 5:1990



Dispersive Grade Index
 Extract from the U.S. Department of Agriculture: Soil Conservation service: Soil Mechanics Note No. 13 (1991)

Dispersion %	Class
<30	Non-Dispersive
30-60	Intermediate (additional tests recommended)
>60	Dispersive

Classification of test sample

18 (Non-Dispersive)

Remarks: These methods may not be suited to materials where the clay content < 10% and plasticity index ≤ 4. Hydrometer tests are done in accordance with ASTM D422 and interpreted according to BS 1377: Part 5.

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 E-mail: jhb@civilab.co.za • Website: www.civilab.co.za

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Falling Head Permeability Test Results

Project:	MZIMVUBU WATER PROJECT	
Project No:	2014-B-1687	Date: 21/10/2014

Lab. Sample Reference	Field Sample Reference	Depth (m)	Moisture Contents		Dry density Kg/m ³		Coefficient of Permeability (m/s)		
			Before Test (%)	After Test (%)	Initial	As tested	Range		Average
							Minimum	Maximum	
1687-21	CTP : MIX	-	22.0	25.2	1567	1573	2.5E-08	3.0E-08	2.7E-08

Remarks: Sample remoulded to 98% Proctor.
 Saturated and tested under a load of 100kPa.
 Densities reported are under a load of 100kPa.

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 Tel:+27 (0)11 835 3117
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Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-21
Field Sample Number:	CTP - Mix	Depth (m):	-

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,8

Remarks:	A Consolidated Drained test on a remoulded sample tested saturated.
----------	---

SATURATION DATA

Test No. 1

Saturation method:	Alternating increments of cell- & back pressure		
Pressure increments applied (kPa):	50,70,100,100,100...	Differential pressure (kPa):	10.0
Final cell pressure (kPa):	353.0	Final back pressure (kPa):	343.0
		Final B parameter:	0.99

CONSOLIDATION DATA

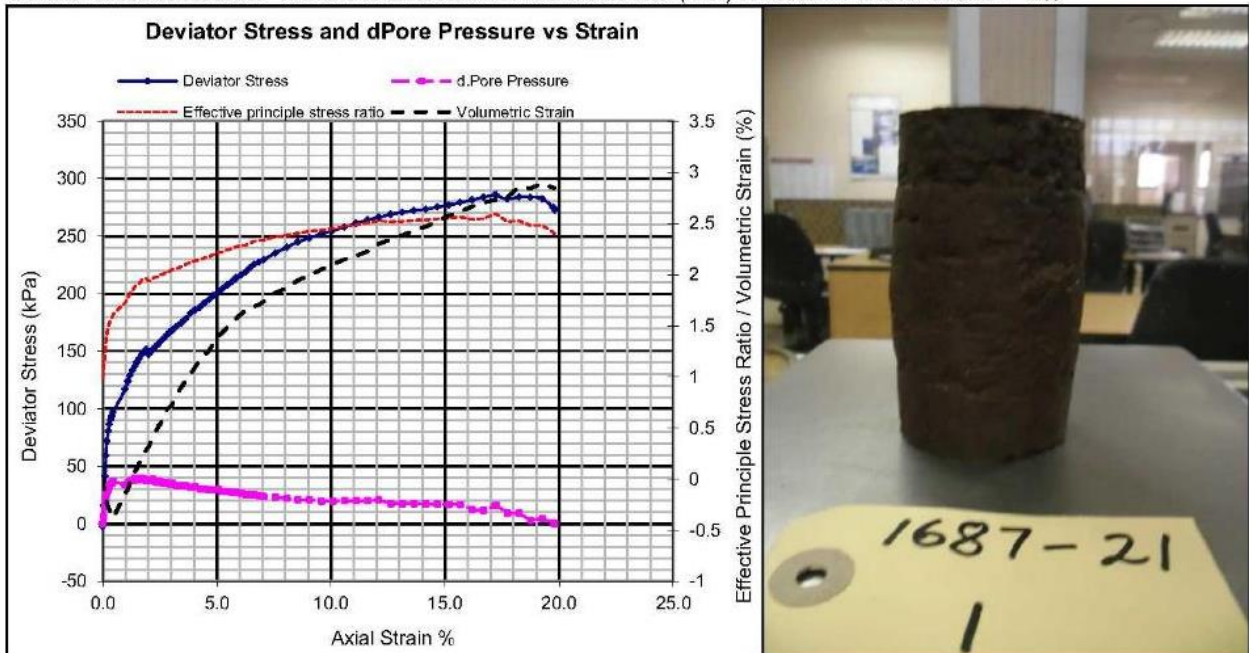
Effective cons. Stress (kPa):	195.4	t100 (minutes):	16	Side drains fitted:	Yes			
	Height mm	Diameter mm	Area mm ²	Moisture Content %	Dry Density kg/m ³	Void Ratio	Saturation %	Specific Gravity
INITIAL (Before saturation)	*100.81	*50.05	1967.42	24.2	1521	0.7742	84	2.698 Determined
CONSOLIDATED	99.15	49.22	1902.68	26.3	1600	0.6867	103	
FINAL (After shear)	79.52	54.17	2304.94	24.5	1646	0.6396	103	
Initial pore pressure (kPa):	533.6	Final pore pressure (kPa):	348.9	PWP dissipation (%):	97			

*: Measured dimensions; all other dimensions are calculated.

SHEAR DATA

Rate of strain (%/hour):	0.50		
Initial pore pressure (kPa):	347.6	Initial effective stress (kPa):	195.4
Parameters at failure:			
Failure Criterion:	Max. Deviator Stress		
Axial strain (%):	17.19	Volumetric strain (%):	2.73
Deviator stress (kPa):	285.9	Principle Stresses (kPa)	
Excess pore pressure (kPa):	15.5	σ_1	σ_1'
Effective principle stress ratio:	2.590	481.3	465.8
		σ_3	σ_3'
		195.4	179.8

Deviator stress corrections: Membrane correction: 1.1 kPa & Side drain(filter) correction: 7 kPa for Strains >= 2%



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 PO Box 82223,Southdale 2135
 Tel:+27 (0)11 835 3117
 E-mail: jhb@civilab.co.za Website: www.civilab.co.za

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Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-21
Field Sample Number:	CTP - Mix	Depth (m):	-

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,8

Remarks:	A Consolidated Drained test on a remoulded sample tested saturated.
----------	---

Test No. 2

SATURATION DATA

Saturation method:	Alternating increments of cell- & back pressure		
Pressure increments applied (kPa):	50,70,100,100,100...	Differential pressure (kPa):	10.0
Final cell pressure (kPa):	353.0	Final back pressure (kPa):	343.0
		Final B parameter:	0.99

CONSOLIDATION DATA

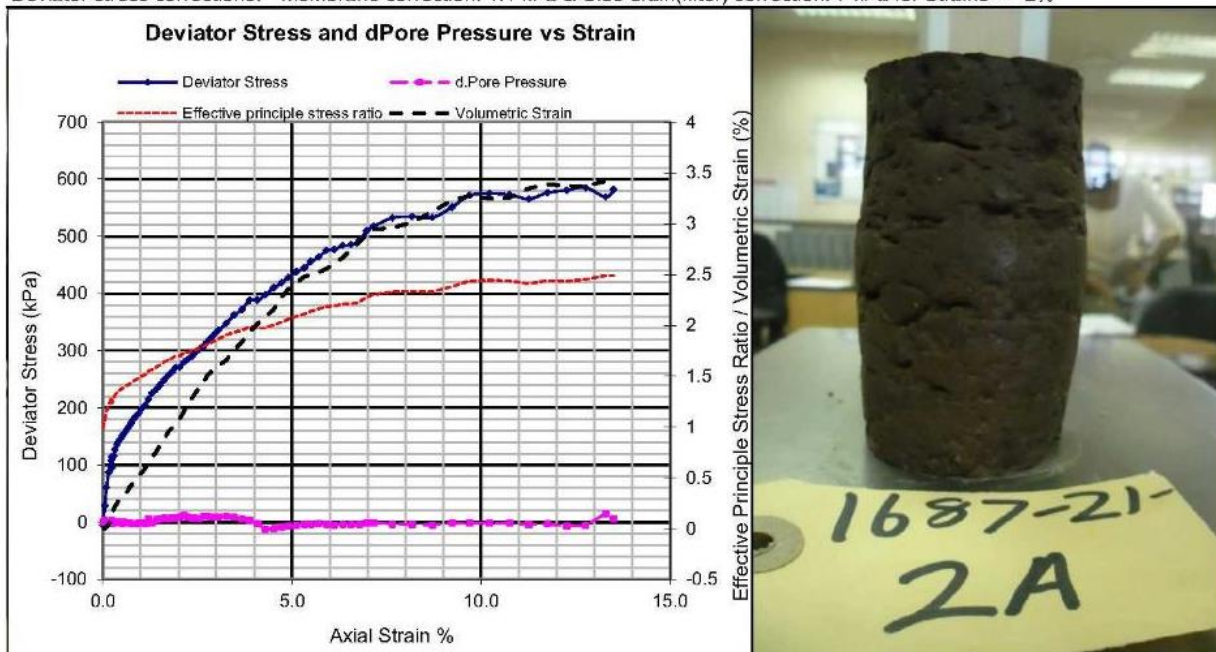
Effective cons. Stress (kPa):	396.7	t100 (minutes):	324	Side drains fitted:	Yes				
	Height mm	Diameter mm	Area mm ²	Moisture Content %	Dry Density kg/m ³	Void Ratio	Saturation %	Specific Gravity	
INITIAL (Before saturation)	*100.03	*50.03	1965.85	25.5	1519	0.7756	89	2.698 Determined	
CONSOLIDATED	97.49	48.74	1865.85	26.7	1645	0.6401	113		
FINAL (After shear)	84.32	51.50	2082.96	24.6	1701	0.5859	113		
Initial pore pressure (kPa):		342.7		Final pore pressure (kPa):		345.8		PWP dissipation (%):	97

*: Measured dimensions; all other dimensions are calculated.

SHEAR DATA

Rate of strain (%/hour):	0.09		
Initial pore pressure (kPa):	346.3	Initial effective stress (kPa):	396.7
Parameters at failure:			
Failure Criterion:	Max. Deviator Stress		
Axial strain (%):	12.77	Volumetric strain (%):	3.37
Deviator stress (kPa):	585.0	Principle Stresses (kPa)	
Excess pore pressure (kPa):	-6.0	σ_1	σ_1'
Effective principle stress ratio:	2.452	981.7	987.7
		σ_3	σ_3'
		396.7	402.8

Deviator stress corrections: Membrane correction: 1.1 kPa & Side drain(filter) correction: 7 kPa for Strains >= 2%



36 Fourth Str.,Booyens Reserve,Johannesburg 2091
 PO Box 82223,Southdale 2135
 Tel:+27 (0)11 835 3117
 E-mail: jhb@civilab.co.za Website: www.civilab.co.za

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Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-21
Field Sample Number:	CTP - Mix	Depth (m):	-

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,8

Remarks:	A Consolidated Drained test on a remoulded sample tested saturated.
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SATURATION DATA

Test No. 3

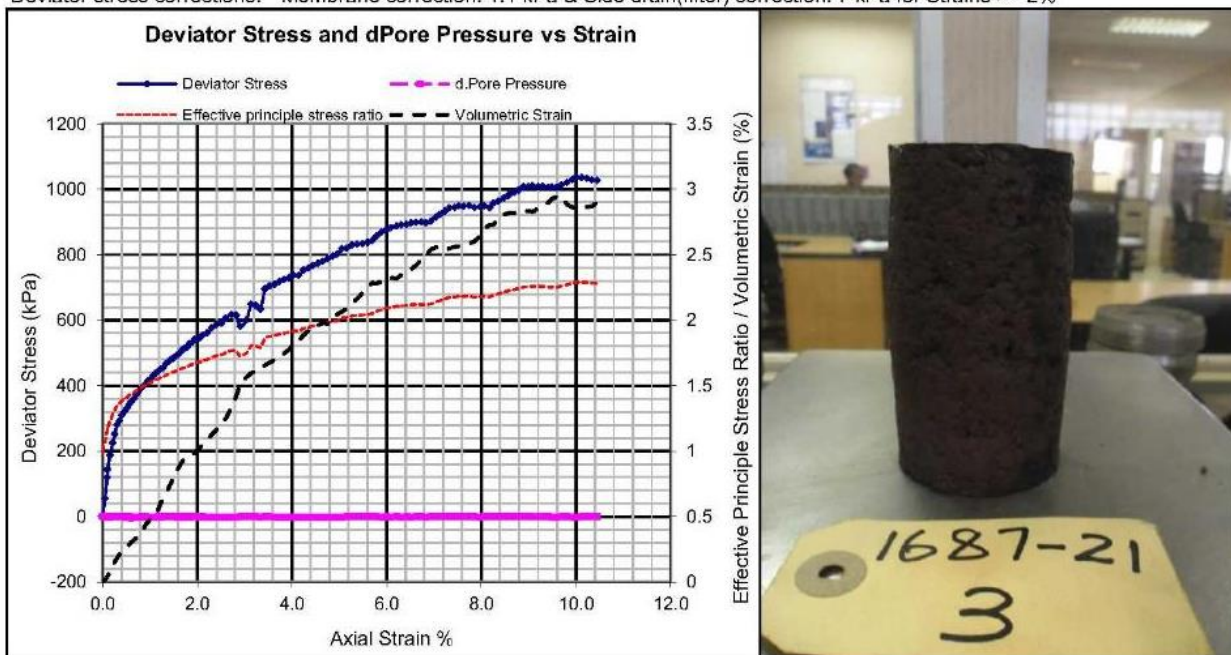
Saturation method:	Alternating increments of cell- & back pressure		
Pressure increments applied (kPa):	50,70,100,100,100...	Differential pressure (kPa):	10.0
Final cell pressure (kPa):	353.0	Final back pressure (kPa):	343.0
		Final B parameter:	0.97

CONSOLIDATION DATA

Effective cons. Stress (kPa):	802.1	t100 (minutes):	400	Side drains fitted:	Yes			
	Height mm	Diameter mm	Area mm ²	Moisture Content %	Dry Density kg/m ³	Void Ratio	Saturation %	Specific Gravity
INITIAL (Before saturation)	*100.57	*50.03	1965.85	23.7	1534	0.6655	91	2.555 Determined
CONSOLIDATED	96.74	48.09	1816.28	22.5	1732	0.4754	121	
FINAL (After shear)	86.64	50.07	1969.24	20.8	1778	0.4373	122	
Initial pore pressure (kPa):	339.0	Final pore pressure (kPa):	345.0	PWP dissipation (%):	100			
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour):	0.05		
Initial pore pressure (kPa):	338.9	Initial effective stress (kPa):	802.1
Parameters at failure:			
Failure Criterion:	Max. Deviator Stress		
Axial strain (%):	10.01	Volumetric strain (%):	2.85
Deviator stress (kPa):	1037.7	Principle Stresses (kPa)	
Excess pore pressure (kPa):	-1.1	σ_1	σ_1'
Effective principle stress ratio:	2.292	1839.8	1840.9
		σ_3	σ_3'
		802.1	803.2
Deviator stress corrections: Membrane correction: 1.1 kPa & Side drain(filter) correction: 7 kPa for Strains >= 2%			



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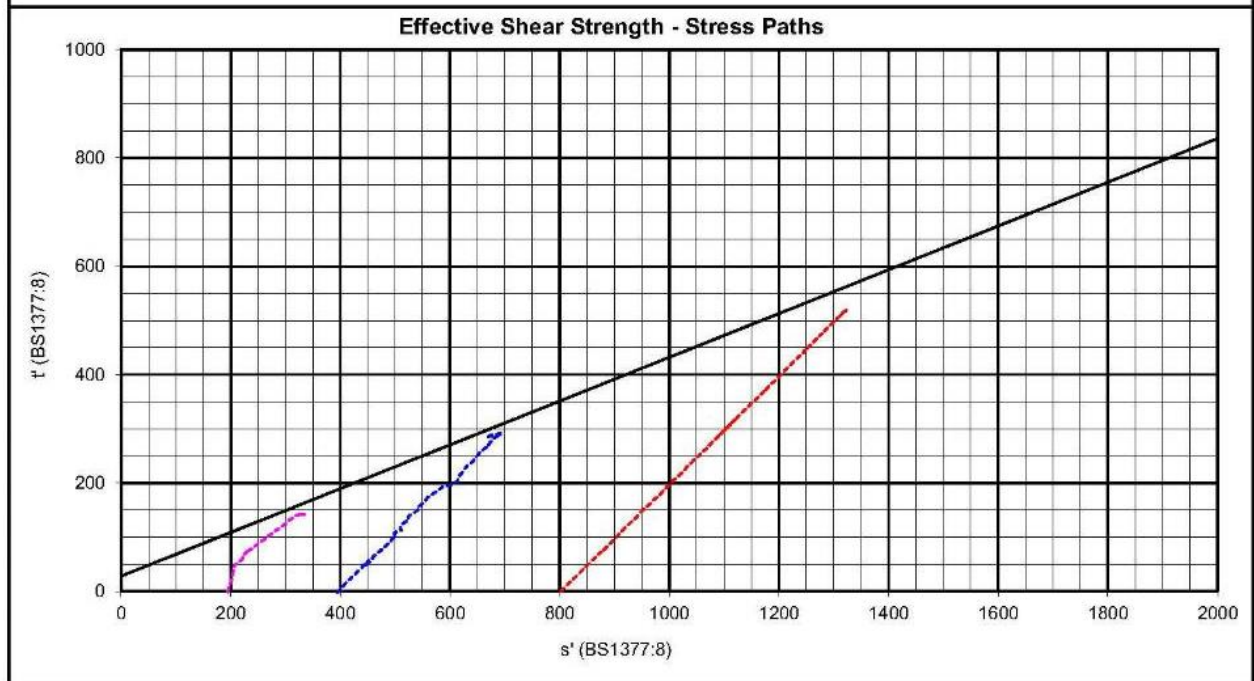
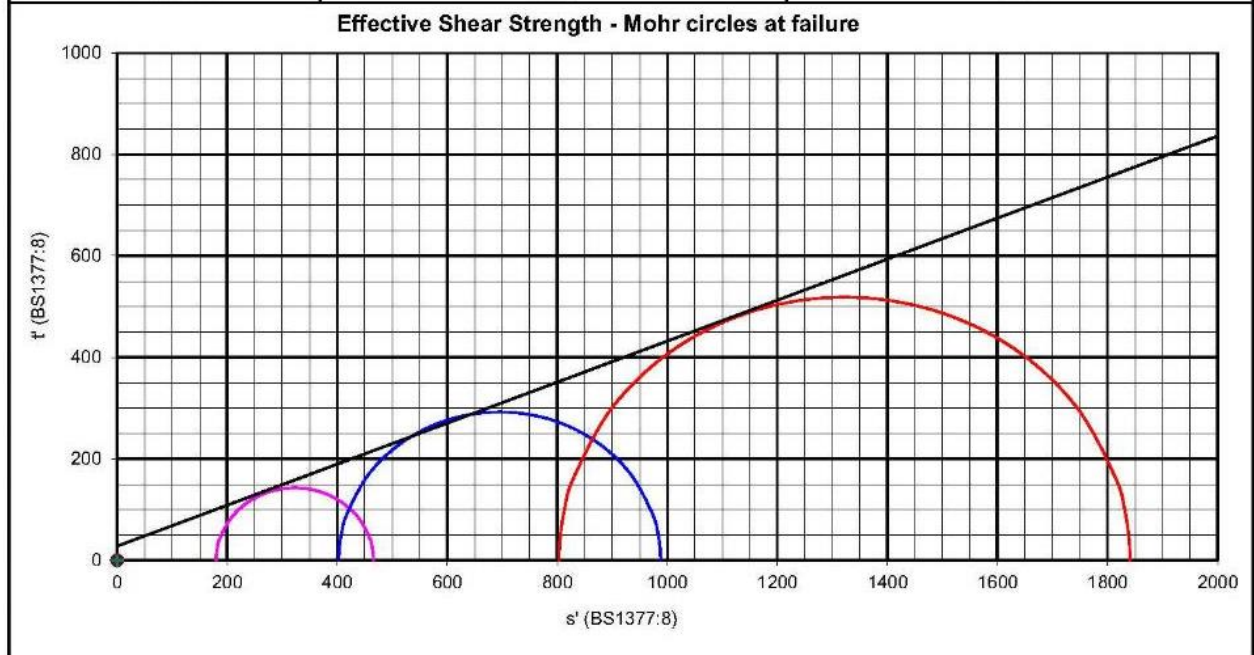
Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-21
Field Sample Reference:	CTP - Mix	Depth (m):	-

Effective Shear Strength Parameters

Stresses	Cohesion (kPa)	Internal friction (Degrees)
Total	22.6	22.3
Effective	27.9	22.0



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 PO Box 82223,Southdale 2135
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 E-mail: jhb@civilab.co.za Website: www.civilab.co.za

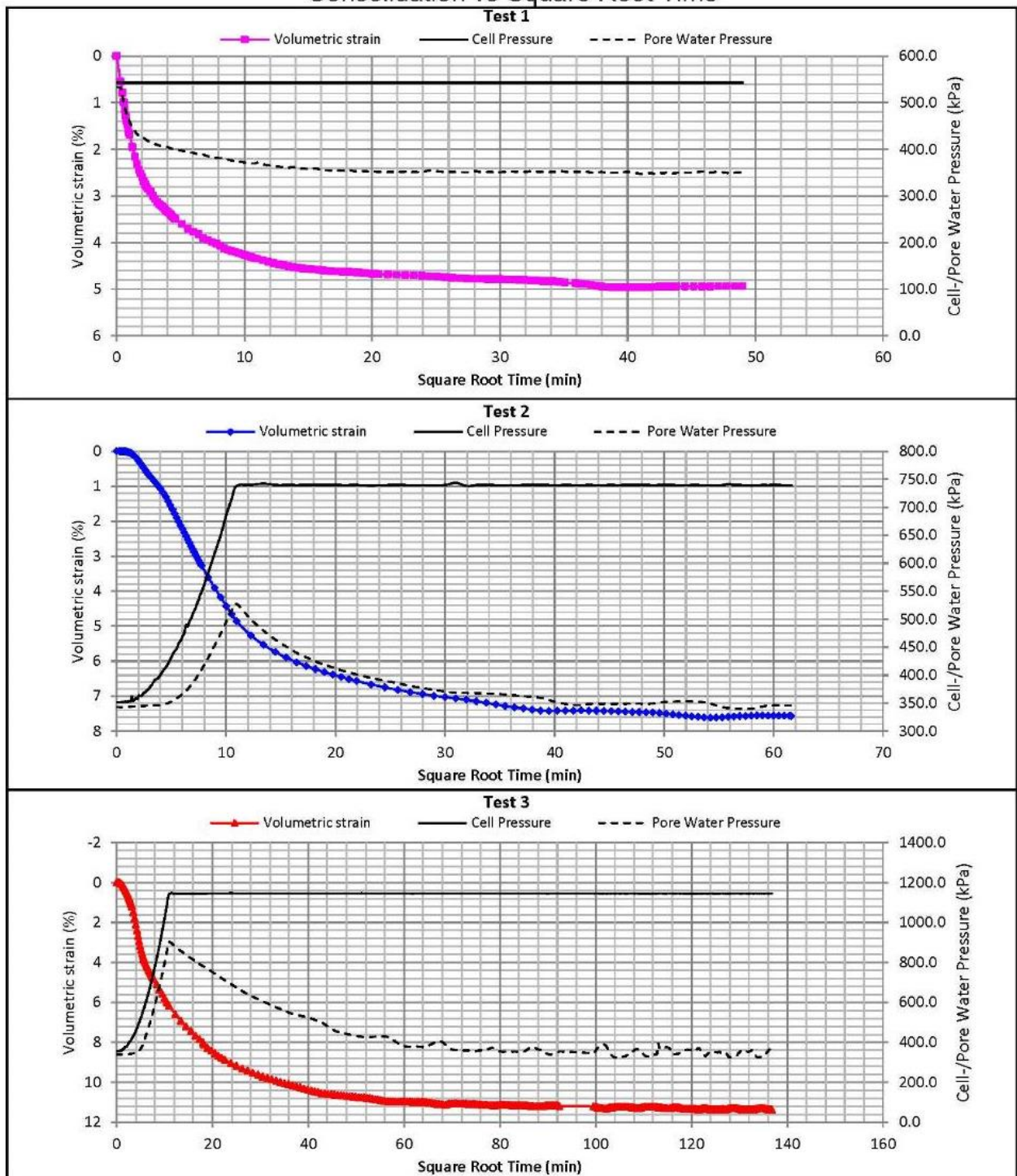


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Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-21
Field Sample Reference:	CTP - Mix	Depth (m):	-

Consolidation vs Square Root Time



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Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	B-1687-21
Field Sample Number:	CTP - Mix	Depth (m):	-

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks:	A Consolidated Undrained test on a remoulded sample tested saturated.
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SATURATION DATA

Test No. 1

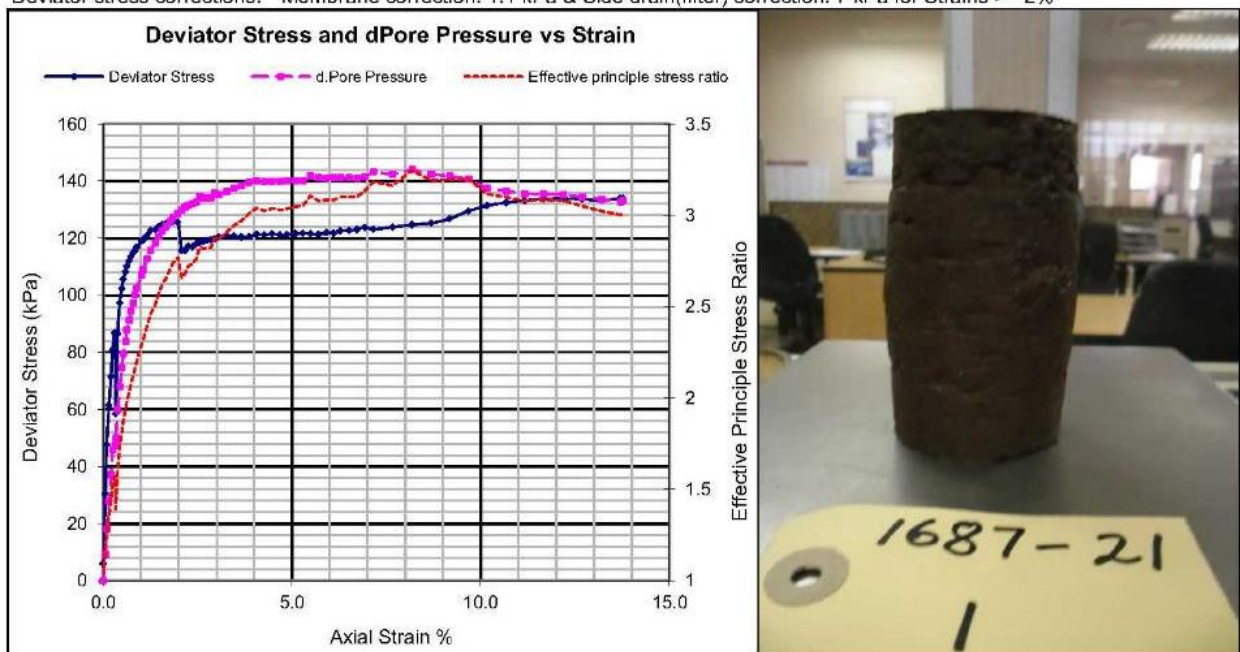
Saturation method:	Alternating increments of cell- & back pressure		
Pressure increments applied (kPa):	50,70,100,100,100...	Differential pressure (kPa):	10.0
Final cell pressure (kPa):	353.0	Final back pressure (kPa):	343.0
		Final B parameter:	0.97

CONSOLIDATION DATA

Effective cons. Stress (kPa):	199.6	t100 (minutes):	64	Side drains fitted:	Yes			
	Height mm	Diameter mm	Area mm ²	Moisture Content %	Dry Density kg/m ³	Void Ratio	Saturation %	Specific Gravity
INITIAL (Before saturation)	*100.74	*49.96	1960.36	23.0	1543	0.7487	83	2.698 Determined
CONSOLIDATED	98.92	49.05	1889.65	25.9	1631	0.6541	107	
FINAL (After shear)	85.29	52.82	2191.64	25.9	1630	0.6552	106	
Initial pore pressure (kPa):	537.8	Final pore pressure (kPa):	351.7	PWP dissipation (%):	100			
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour):	0.30		
Initial pore pressure (kPa):	346.4	Initial effective stress (kPa):	199.6
Parameters at failure:			
Failure Criterion:	Max. Effective Principle Stress Ratio		
Axial strain (%):	8.17		
Deviator stress (kPa):	124.7	Principle Stresses (kPa)	
Excess pore pressure (kPa):	144.0	σ_1	σ_1'
Effective principle stress ratio:	3.246	324.3	180.3
		σ_3	σ_3'
		199.6	55.5
Deviator stress corrections: Membrane correction: 1.1 kPa & Side drain(filter) correction: 7 kPa for Strains >= 2%			



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 PO Box 82223,Southdale 2135
 Tel:+27 (0)11 835 3117
 E-mail: jhb@civilab.co.za Website: www.civilab.co.za

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Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	B-1687-21
Field Sample Number:	CTP - Mix	Depth (m):	-

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks:	A Consolidated Undrained test on a remoulded sample tested saturated.
----------	---

SATURATION DATA

Test No. 2

Saturation method:	Alternating increments of cell- & back pressure		
Pressure increments applied (kPa):	50,70,100,100,100...	Differential pressure (kPa):	10.0
Final cell pressure (kPa):	343.0	Final back pressure (kPa):	333.0
		Final B parameter:	0.99

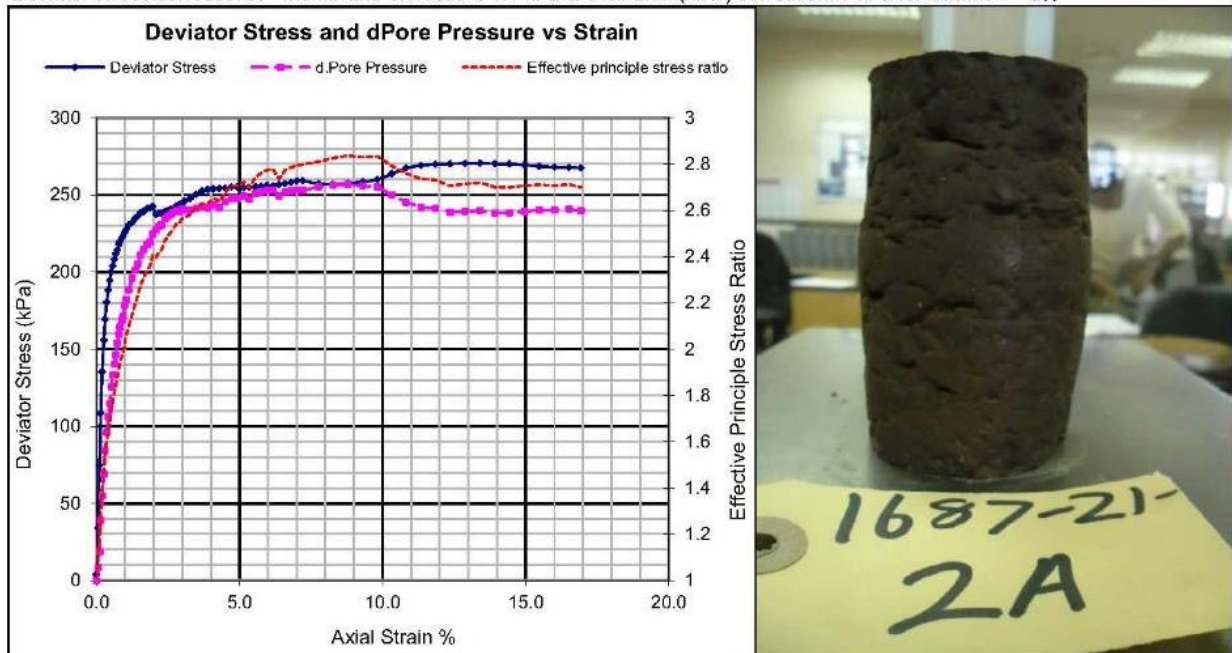
CONSOLIDATION DATA

Effective cons. Stress (kPa):	397.1		t100 (minutes):	81	Side drains fitted:	Yes		
	Height mm	Diameter mm	Area mm ²	Moisture Content %	Dry Density kg/m ³	Void Ratio	Saturation %	Specific Gravity
INITIAL (Before saturation)	*100	*50	1963.50	23.2	1550	0.7409	85	2.698 Determined
CONSOLIDATED	97.13	48.54	1850.78	24.1	1696	0.5910	110	
FINAL (After shear)	80.69	53.26	2227.83	24.1	1693	0.5939	109	
Initial pore pressure (kPa):	728.9		Final pore pressure (kPa):	343.1		PWP dissipation (%): 100		
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour):	0.30		
Initial pore pressure (kPa):	341.9	Initial effective stress (kPa):	397.1
Parameters at failure:			
Failure Criterion:	Max. Effective Principle Stress Ratio		
Axial strain (%):	8.79		
Deviator stress (kPa):	257.6	Principle Stresses (kPa)	
Excess pore pressure (kPa):	256.8	σ_1	σ_1'
Effective principle stress ratio:	2.836	654.7	397.9
		σ_3	σ_3'
		397.1	140.3

Deviator stress corrections: Membrane correction: 1.1 kPa & Side drain(filter) correction: 7 kPa for Strains >= 2%



36 Fourth Str.,Booyens Reserve,Johannesburg 2091
 PO Box 82223,Southdale 2135
 Tel:+27 (0)11 835 3117
 E-mail: jhb@civilab.co.za Website: www.civilab.co.za

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Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	B-1687-21
Field Sample Number:	CTP - Mix	Depth (m):	-

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks:	A Consolidated Undrained test on a remoulded sample tested saturated.
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SATURATION DATA

Test No. 3

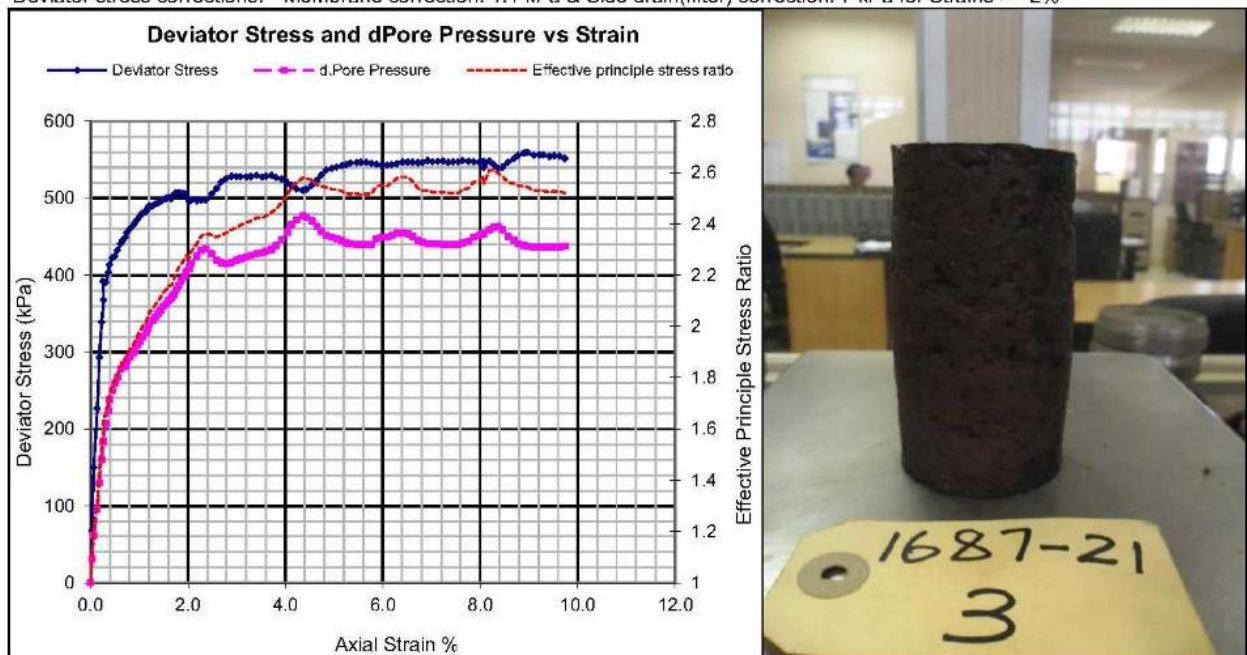
Saturation method:	Alternating increments of cell- & back pressure		
Pressure increments applied (kPa):	50,70,100,100,100...	Differential pressure (kPa):	10.0
Final cell pressure (kPa):	353.0	Final back pressure (kPa):	343.0
		Final B parameter:	0.98

CONSOLIDATION DATA

Effective cons. Stress (kPa):	799.8	t100 (minutes):	400	Side drains fitted:	Yes			
	Height mm	Diameter mm	Area mm ²	Moisture Content %	Dry Density kg/m ³	Void Ratio	Saturation %	Specific Gravity
INITIAL (Before saturation)	*100	*50	1963.50	22.9	1556	0.7335	84	2.698 Determined
CONSOLIDATED	96.20	48.06	1814.28	22.0	1757	0.5359	111	
FINAL (After shear)	86.83	50.59	2009.97	22.0	1751	0.5409	110	
Initial pore pressure (kPa):	341.7		Final pore pressure (kPa):	362.9		PWP dissipation (%): 100		
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour):	0.08		
Initial pore pressure (kPa):	342.2	Initial effective stress (kPa):	799.8
Parameters at failure:			
Failure Criterion:	Max. Effective Principle Stress Ratio		
Axial strain (%):	8.28		
Deviator stress (kPa):	543.4	Principle Stresses (kPa)	
Excess pore pressure (kPa):	462.3	σ_1	σ_1'
Effective principle stress ratio:	2.611	1343.2	880.9
		σ_3	σ_3'
		799.8	337.4
Deviator stress corrections: Membrane correction: 1.1 kPa & Side drain(filter) correction: 7 kPa for Strains >= 2%			



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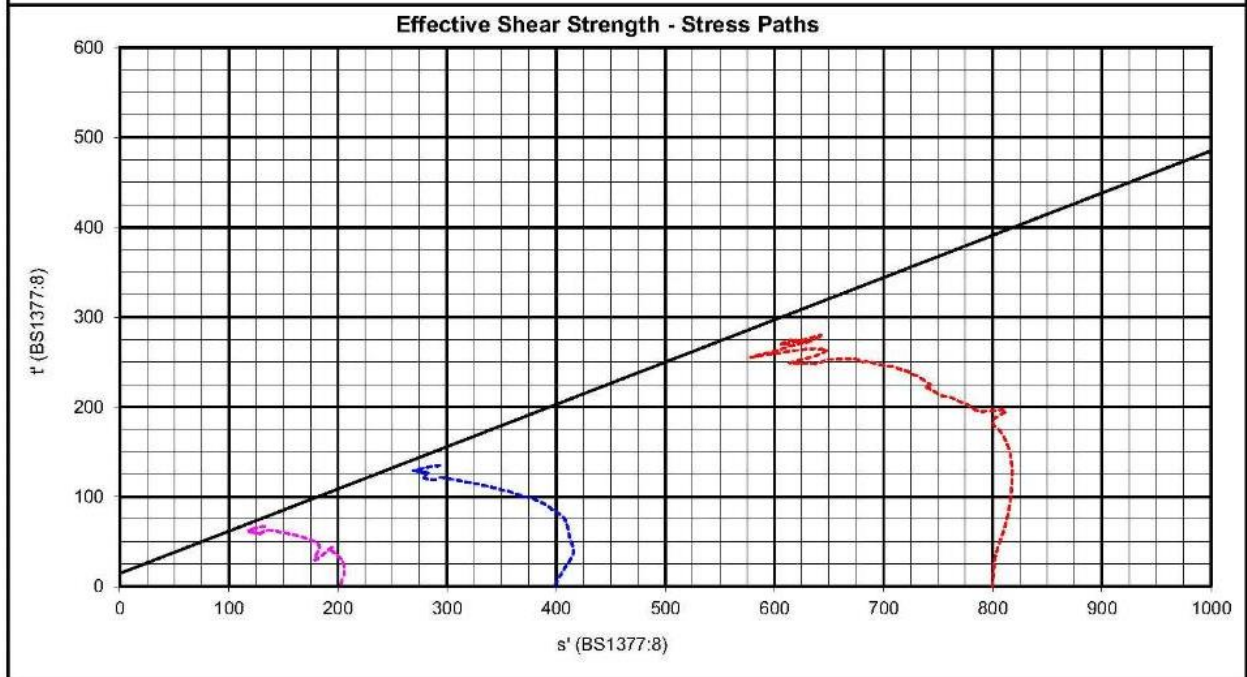
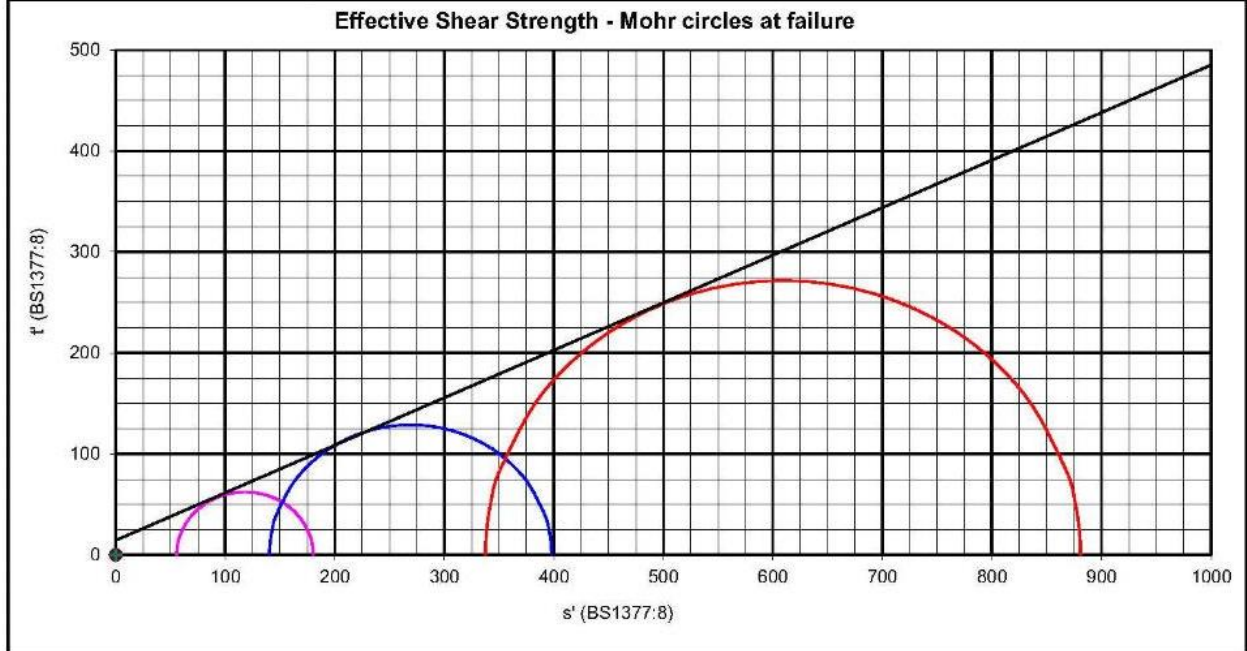
Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	B-1687-21
Field Sample Reference:	CTP - Mix	Depth (m):	-

Effective Shear Strength Parameters

Stresses	Cohesion (kPa)	Internal friction (Degrees)
Total	0.0	14.6
Effective	14.5	25.2



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 Tel: +27 (0)11 835 3117
 E-mail: jhb@civilab.co.za Website: www.civilab.co.za

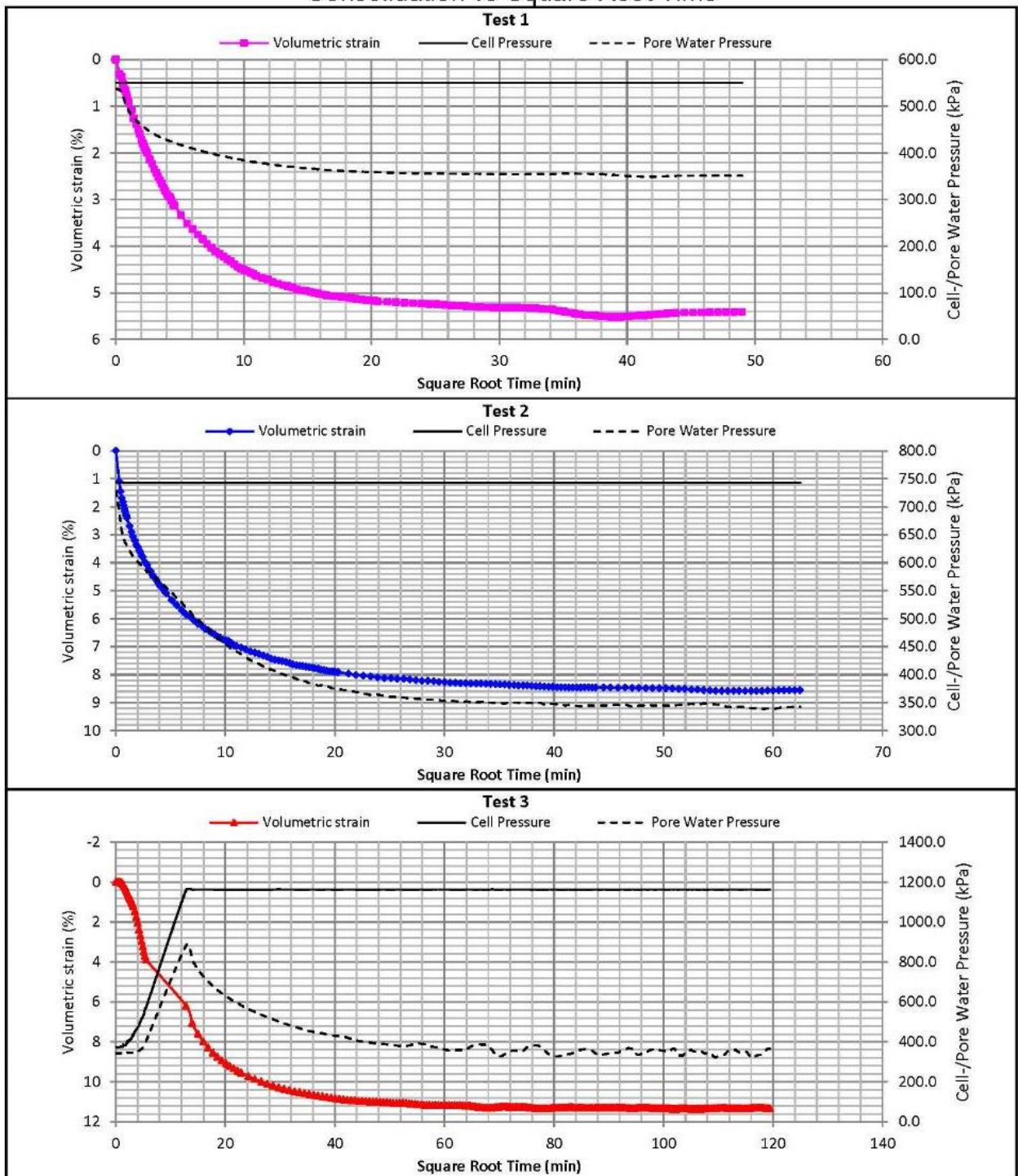


Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	B-1687-21
Field Sample Reference:	CTP - Mix	Depth (m):	-

Consolidation vs Square Root Time



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 PO Box 82223,Southdale 2135
 Tel:+27 (0)11 835 3117
 E-mail: jhb@civilab.co.za Website: www.civilab.co.za

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Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-22
Field Sample Number:	FTP - Mix	Depth (m):	-

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,8

Remarks:	A Consolidated Drained test on a remoulded sample tested saturated.
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SATURATION DATA

Test No. 1

Saturation method:	Alternating increments of cell- & back pressure		
Pressure increments applied (kPa):	50,70,100,100,100...	Differential pressure (kPa):	10.0
Final cell pressure (kPa):	353.0	Final back pressure (kPa):	343.0
		Final B parameter:	0.98

CONSOLIDATION DATA

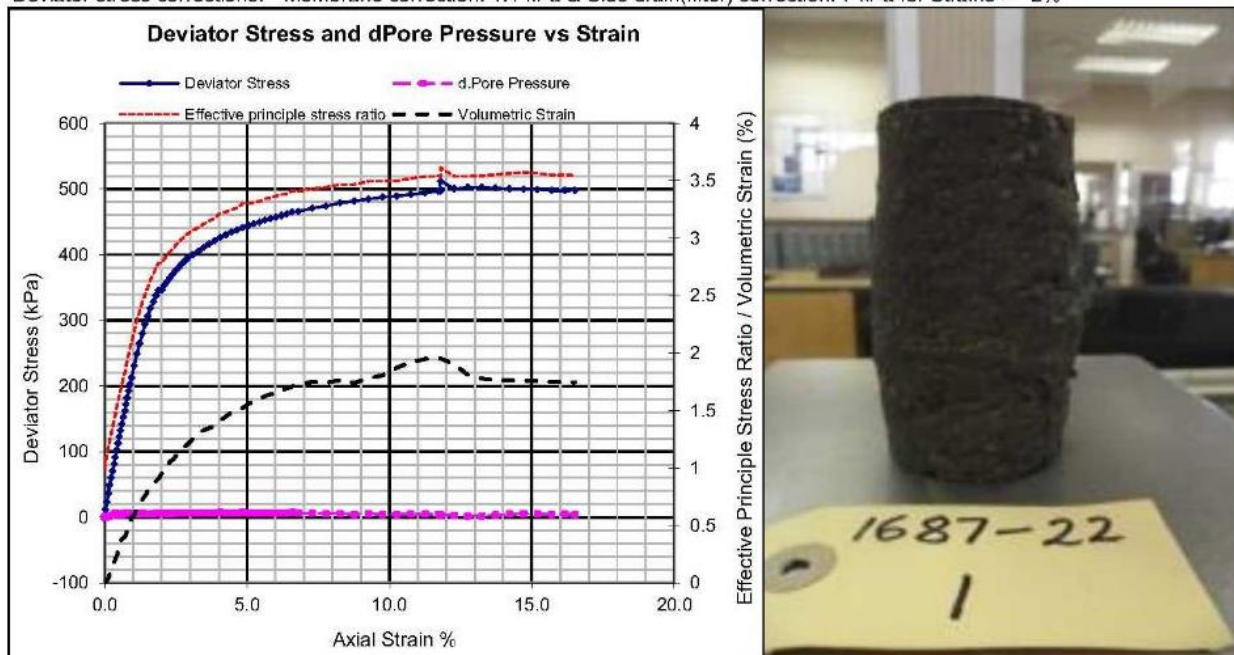
Effective cons. Stress (kPa):	199.6	t100 (minutes):	1	Side drains fitted:	Yes			
	Height mm	Diameter mm	Area mm ²	Moisture Content %	Dry Density kg/m ³	Void Ratio	Saturation %	Specific Gravity
INITIAL (Before saturation)	*103.98	*50.11	1972.14	10.0	1909	0.4324	63	2.734 Determined
CONSOLIDATED	102.37	49.33	1911.19	15.7	2001	0.3660	117	
FINAL (After shear)	85.47	53.52	2249.30	14.8	2036	0.3428	118	
Initial pore pressure (kPa):	539.2	Final pore pressure (kPa):	343.4	PWP dissipation (%):	100			

*: Measured dimensions; all other dimensions are calculated.

SHEAR DATA

Rate of strain (%/hour):	0.50		
Initial pore pressure (kPa):	341.4	Initial effective stress (kPa):	199.6
Parameters at failure:			
Failure Criterion:	Max. Deviator Stress		
Axial strain (%):	11.80	Volumetric strain (%):	1.95
Deviator stress (kPa):	511.4	Principle Stresses (kPa)	
Excess pore pressure (kPa):	3.8	σ_1	σ_1'
Effective principle stress ratio:	3.611	711.1	707.3
		σ_3	σ_3'
		199.6	195.9

Deviator stress corrections: Membrane correction: 1.1 kPa & Side drain(filter) correction: 7 kPa for Strains >= 2%



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Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-22
Field Sample Number:	FTP - Mix	Depth (m):	-

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,8

Remarks:	A Consolidated Drained test on a remoulded sample tested saturated.
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SATURATION DATA

Test No. 2

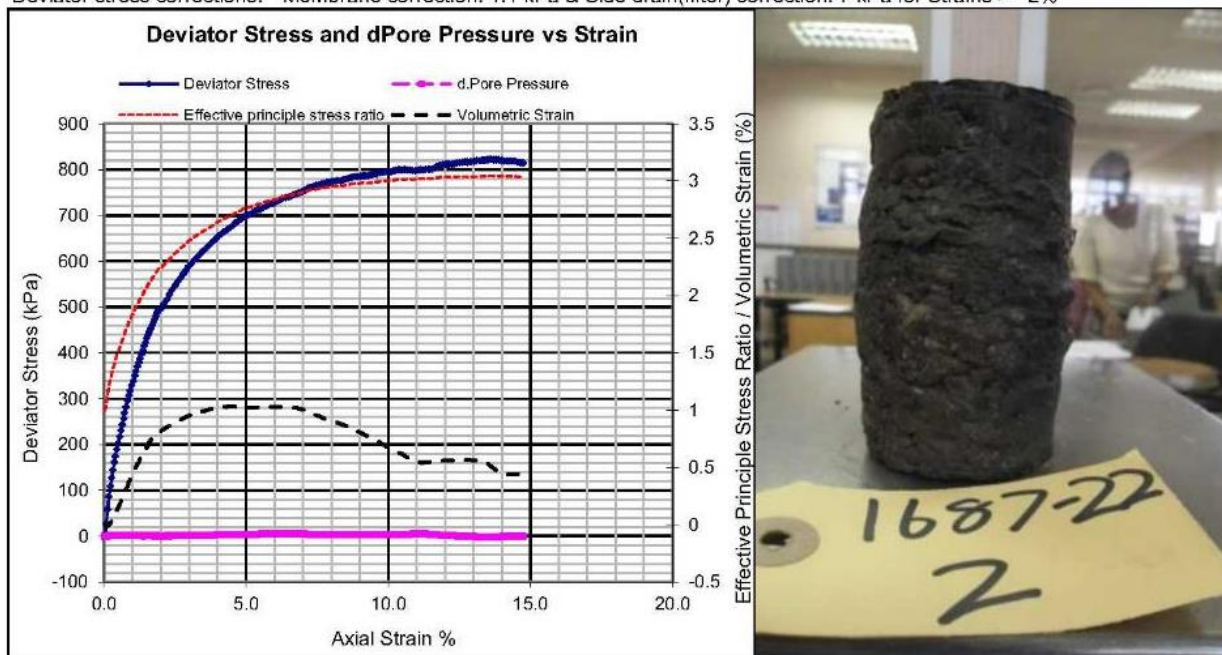
Saturation method:	Alternating increments of cell- & back pressure		
Pressure increments applied (kPa):	50,70,100,100,100...	Differential pressure (kPa):	10.0
Final cell pressure (kPa):	353.0	Final back pressure (kPa):	343.0
		Final B parameter:	0.96

CONSOLIDATION DATA

Effective cons. Stress (kPa):	400.4	t100 (minutes):	121	Side drains fitted:	Yes			
	Height mm	Diameter mm	Area mm ²	Moisture Content %	Dry Density kg/m ³	Void Ratio	Saturation %	Specific Gravity
INITIAL (Before saturation)	*104.49	*50.36	1991.87	10.2	1880	0.4542	62	2.734 Determined
CONSOLIDATED	102.63	49.45	1920.87	14.8	1986	0.3764	108	
FINAL (After shear)	87.50	53.44	2243.10	14.6	1994	0.3713	107	
Initial pore pressure (kPa):	339.8	Final pore pressure (kPa):	339.6	PWP dissipation (%):	104			
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour):	0.21		
Initial pore pressure (kPa):	337.6	Initial effective stress (kPa):	400.4
Parameters at failure:			
Failure Criterion:	Max. Deviator Stress		
Axial strain (%):	13.60	Volumetric strain (%):	0.51
Deviator stress (kPa):	822.4	Principle Stresses (kPa)	
Excess pore pressure (kPa):	-1.3	σ_1	σ_3
Effective principle stress ratio:	3.047	1222.8	400.4
		σ_1'	σ_3'
		1224.1	401.7
Deviator stress corrections: Membrane correction: 1.1 kPa & Side drain(filter) correction: 7 kPa for Strains >= 2%			



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Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-22
Field Sample Number:	FTP - Mix	Depth (m):	-

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,8

Remarks:	A Consolidated Drained test on a remoulded sample tested saturated.
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SATURATION DATA

Test No. 3

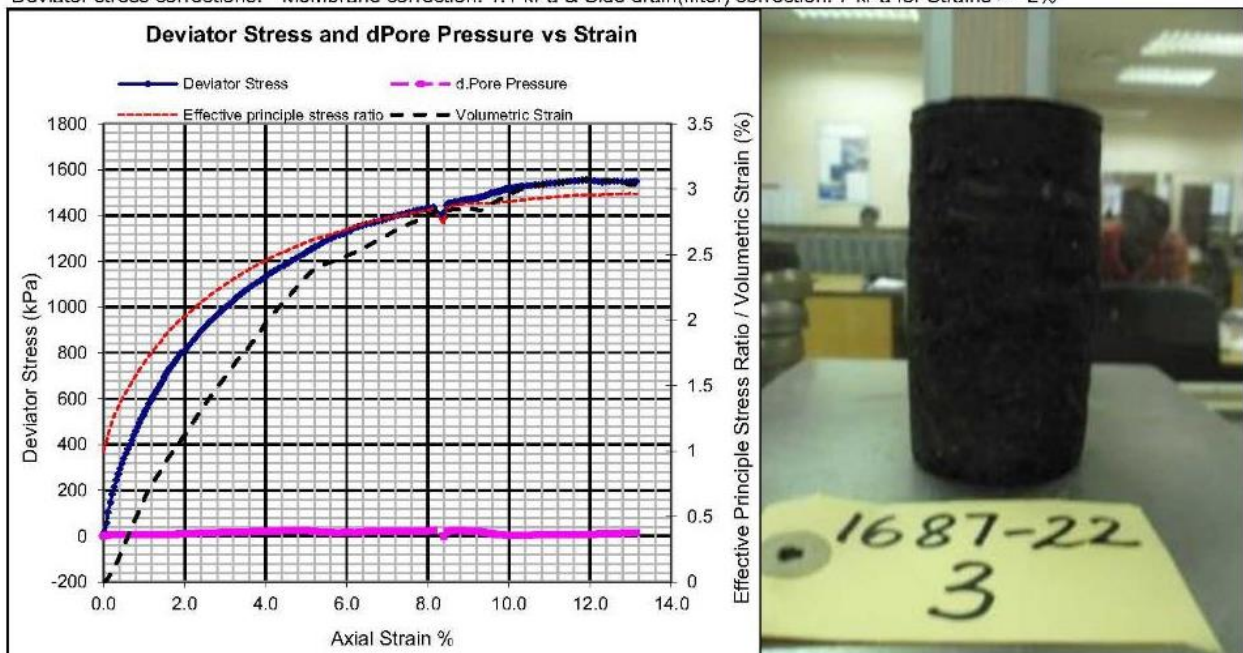
Saturation method:	Alternating increments of cell- & back pressure		
Pressure increments applied (kPa):	50,70,100,100,100...	Differential pressure (kPa):	10.0
Final cell pressure (kPa):	353.0	Final back pressure (kPa):	343.0
		Final B parameter:	0.97

CONSOLIDATION DATA

Effective cons. Stress (kPa):	800.2		t100 (minutes):	121	Side drains fitted:	Yes		
	Height mm	Diameter mm	Area mm ²	Moisture Content %	Dry Density kg/m ³	Void Ratio	Saturation %	Specific Gravity
INITIAL (Before saturation)	*104.82	*50.1	1971.36	10.0	1898	0.4408	62	2.734 Determined
CONSOLIDATED	102.14	48.80	1870.44	14.3	2055	0.3302	118	
FINAL (After shear)	88.72	51.56	2088.11	12.8	2117	0.2917	120	
Initial pore pressure (kPa):	337.4		Final pore pressure (kPa):	337.9		PWP dissipation (%):	100	
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour):	0.18		
Initial pore pressure (kPa):	336.8	Initial effective stress (kPa):	800.2
Parameters at failure:			
Failure Criterion:	Max. Deviator Stress		
Axial strain (%):	11.90	Volumetric strain (%):	3.08
Deviator stress (kPa):	1554.2		Principle Stresses (kPa)
Excess pore pressure (kPa):	6.2	σ_1	σ_1'
Effective principle stress ratio:	2.957	2354.4	2348.2
		σ_3	800.2
		σ_3'	794.0
Deviator stress corrections: Membrane correction: 1.1 kPa & Side drain(filter) correction: 7 kPa for Strains >= 2%			



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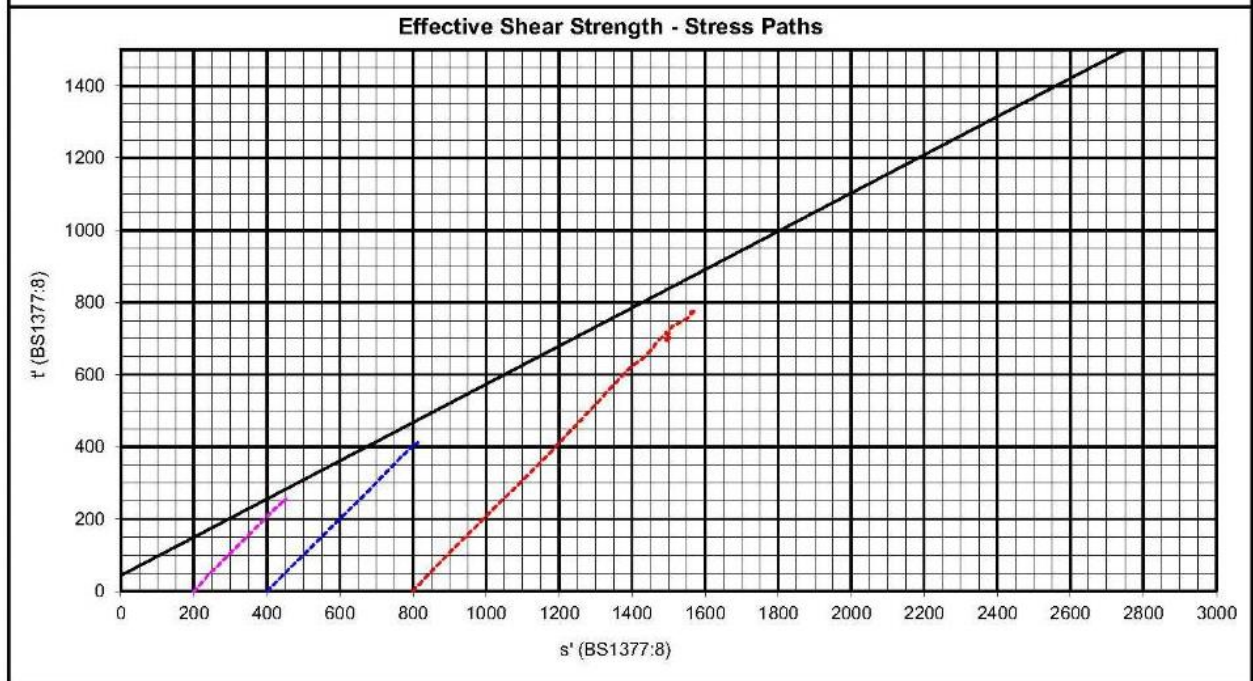
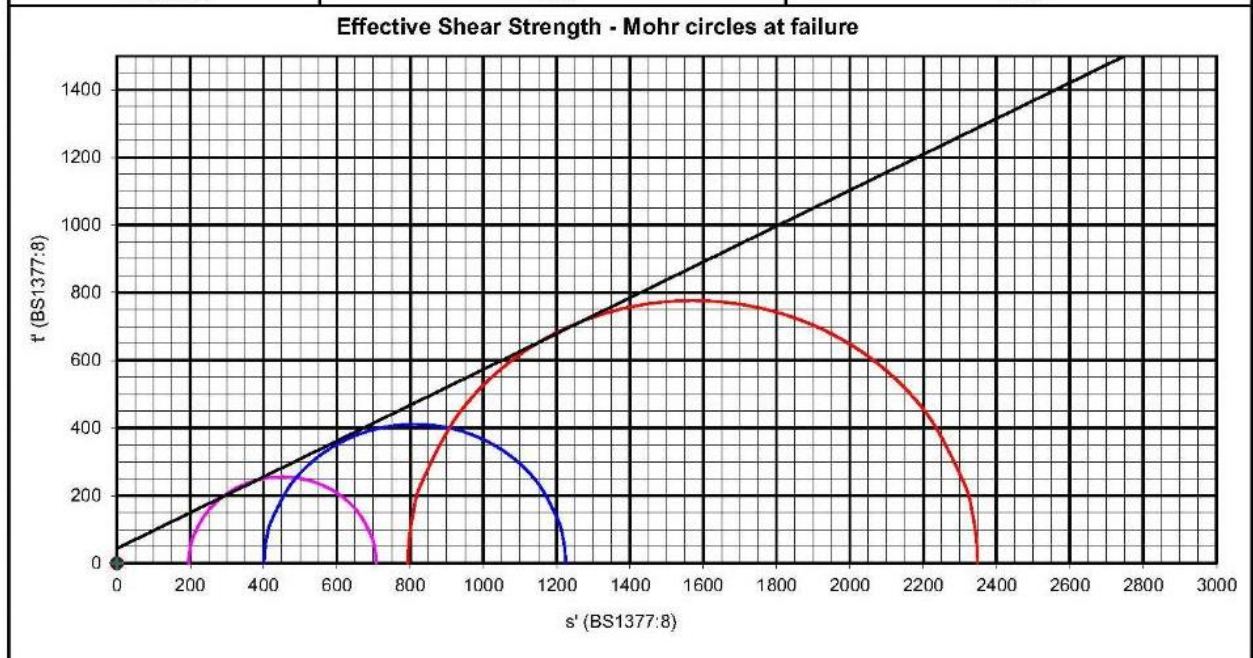
Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-22
Field Sample Reference:	FTP - Mix	Depth (m):	-

Effective Shear Strength Parameters

Stresses	Cohesion (kPa)	Internal friction (Degrees)
Total	43.8	27.8
Effective	43.8	27.9



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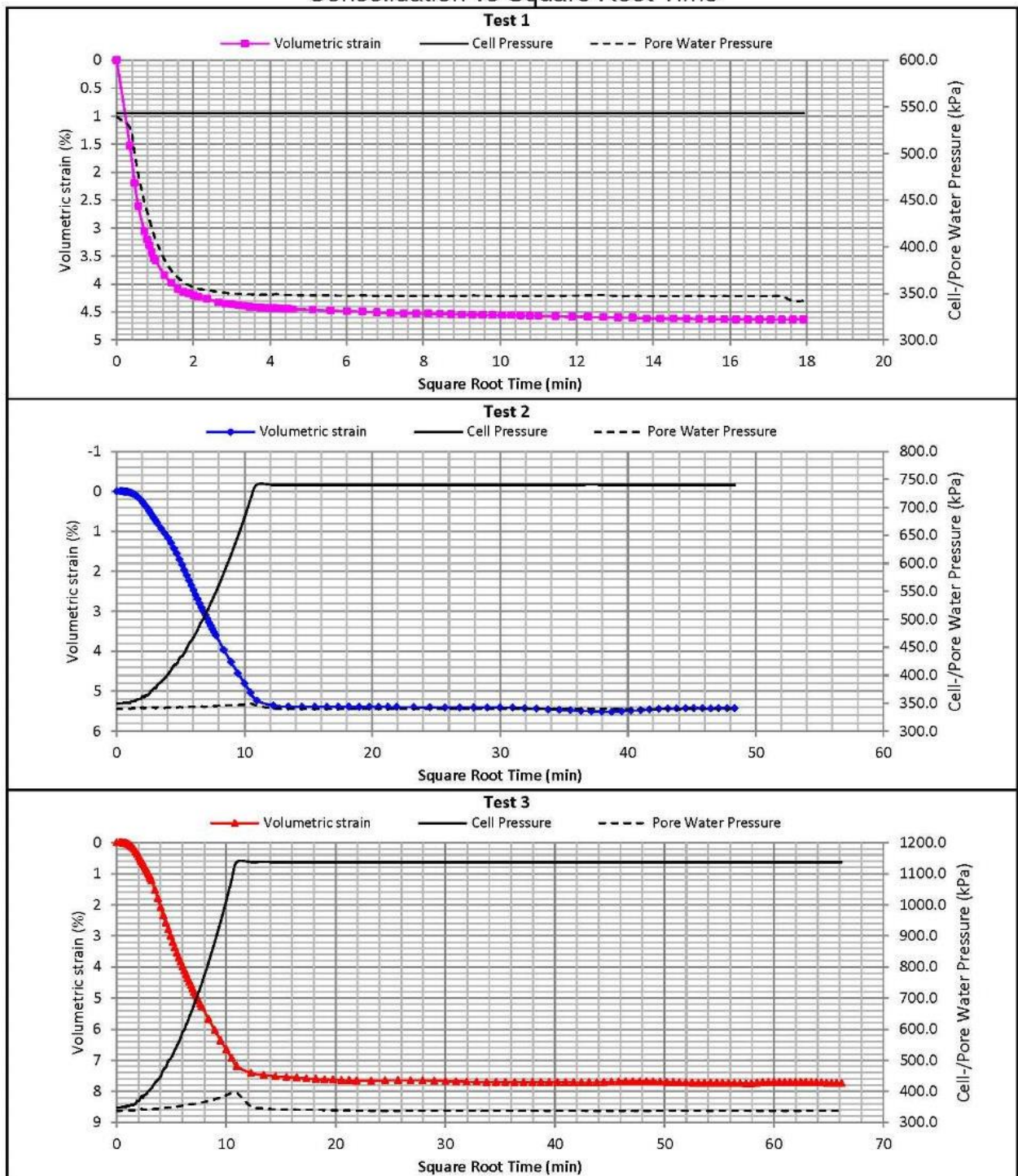


Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-22
Field Sample Reference:	FTP - Mix	Depth (m):	-

Consolidation vs Square Root Time



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 Tel:+27 (0)11 835 3117
 E-mail: jhb@civilab.co.za Website: www.civilab.co.za

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Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-22
Field Sample Number:	FTP - Mix	Depth (m):	-

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks:	A Consolidated Undrained test on a remoulded sample tested saturated.
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SATURATION DATA

Test No. 1

Saturation method:	Alternating increments of cell- & back pressure		
Pressure increments applied (kPa):	50,70,100,100,100...	Differential pressure (kPa):	10.0
Final cell pressure (kPa):	393.0	Final back pressure (kPa):	383.0
		Final B parameter:	1.00

CONSOLIDATION DATA

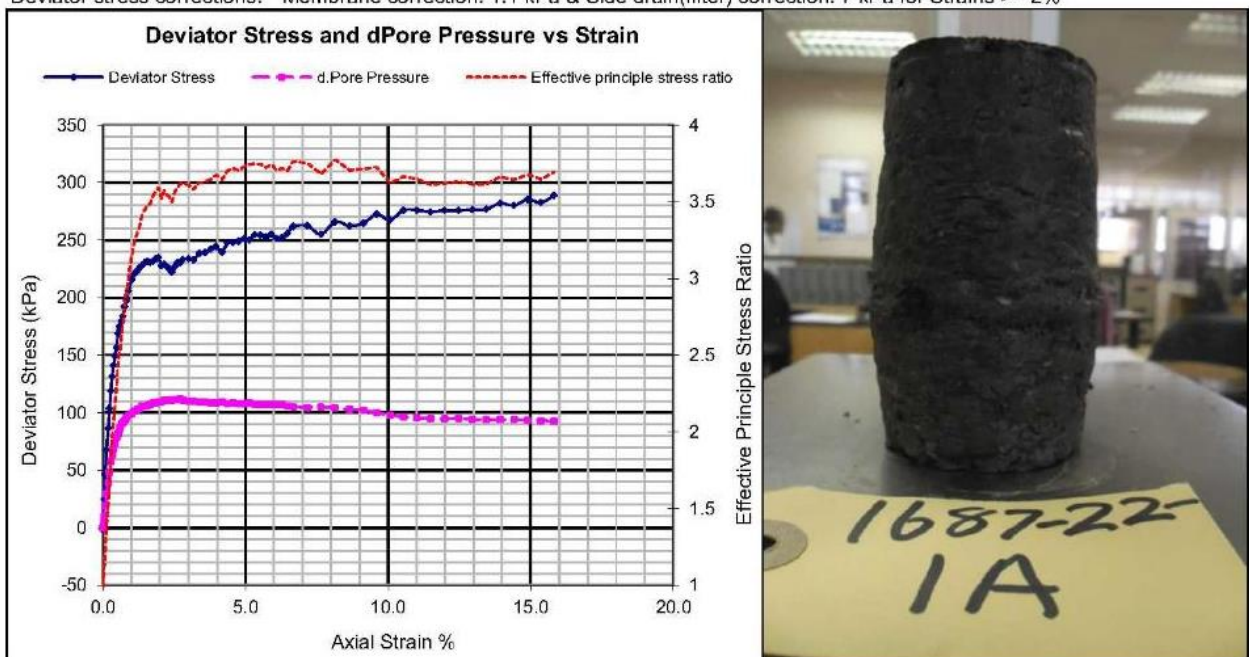
Effective cons. Stress (kPa):	199.8	t100 (minutes):	1	Side drains fitted:	Yes			
	Height mm	Diameter mm	Area mm ²	Moisture Content %	Dry Density kg/m ³	Void Ratio	Saturation %	Specific Gravity
INITIAL (Before saturation)	*105.19	*50.65	2014.88	10.0	1849	0.4786	57	2.734 Determined
CONSOLIDATED	104.13	50.14	1974.32	15.7	1907	0.4339	99	
FINAL (After shear)	87.67	54.64	2345.09	15.7	1906	0.4342	99	
Initial pore pressure (kPa):	573.7	Final pore pressure (kPa):	387.4	PWP dissipation (%):	100			

*: Measured dimensions; all other dimensions are calculated.

SHEAR DATA

Rate of strain (%/hour):	0.30		
Initial pore pressure (kPa):	386.2	Initial effective stress (kPa):	199.8
Parameters at failure:			
Failure Criterion:	Max. Effective Principle Stress Ratio		
Axial strain (%):	8.15		
Deviator stress (kPa):	265.9	Principle Stresses (kPa)	
Excess pore pressure (kPa):	104.0	σ_1	σ_3
Effective principle stress ratio:	3.774	465.7	199.8
		σ_1'	σ_3'
		361.7	95.9

Deviator stress corrections: Membrane correction: 1.1 kPa & Side drain(filter) correction: 7 kPa for Strains >= 2%



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Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-22
Field Sample Number:	FTP - Mix	Depth (m):	-

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks: A Consolidated Undrained test on a remoulded sample tested saturated.

SATURATION DATA

Test No. 2

Saturation method:	Alternating increments of cell- & back pressure		
Pressure increments applied (kPa):	50,70,100,100,100...	Differential pressure (kPa):	10.0
Final cell pressure (kPa):	333.0	Final back pressure (kPa):	323.0
		Final B parameter:	0.96

CONSOLIDATION DATA

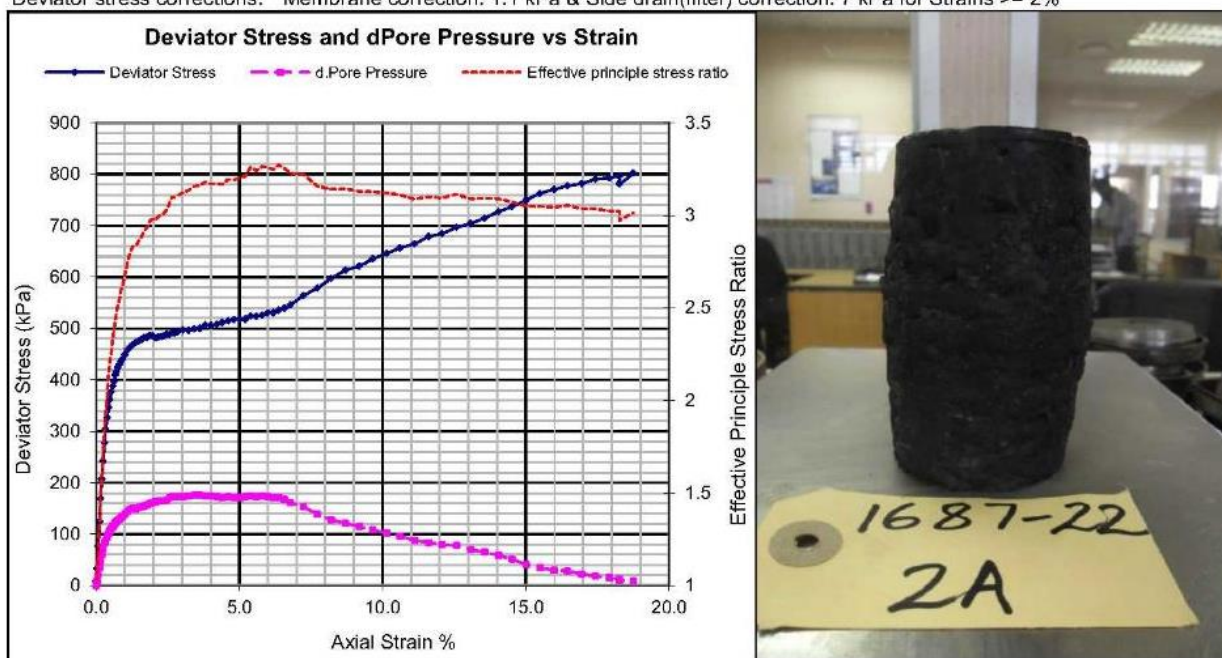
Effective cons. Stress (kPa):	406.5		t100 (minutes):	0		Side drains fitted:	Yes		
	Height mm	Diameter mm	Area mm ²	Moisture Content %	Dry Density kg/m ³	Void Ratio	Saturation %	Specific Gravity	
INITIAL (Before saturation)	*104.42	*50.62	2012.49	10.0	1867	0.4645	59	2.734 Determined	
CONSOLIDATED	102.88	49.87	1953.12	13.9	1953	0.3997	95		
FINAL (After shear)	83.59	55.32	2403.96	13.9	1952	0.4004	95		
Initial pore pressure (kPa):	704.8		Final pore pressure (kPa):	322.2		PWP dissipation (%):	100		

*: Measured dimensions; all other dimensions are calculated.

SHEAR DATA

Rate of strain (%/hour):	0.50		
Initial pore pressure (kPa):	319.5	Initial effective stress (kPa):	406.5
Parameters at failure:			
Failure Criterion:	Max. Effective Principle Stress Ratio		
Axial strain (%):	6.37		
Deviator stress (kPa):	535.1	Principle Stresses (kPa)	
Excess pore pressure (kPa):	170.9	σ_1	σ_1'
Effective principle stress ratio:	3.271	941.6	770.7
		σ_3	σ_3'
		406.5	235.6

Deviator stress corrections: Membrane correction: 1.1 kPa & Side drain(filter) correction: 7 kPa for Strains >= 2%



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Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-22
Field Sample Number:	FTP - Mix	Depth (m):	-

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks:	A Consolidated Undrained test on a remoulded sample tested saturated.
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SATURATION DATA

Test No. 3

Saturation method:	Alternating increments of cell- & back pressure		
Pressure increments applied (kPa):	50,70,100,100,100...	Differential pressure (kPa):	10.0
Final cell pressure (kPa):	353.0	Final back pressure (kPa):	343.0
		Final B parameter:	0.98

CONSOLIDATION DATA

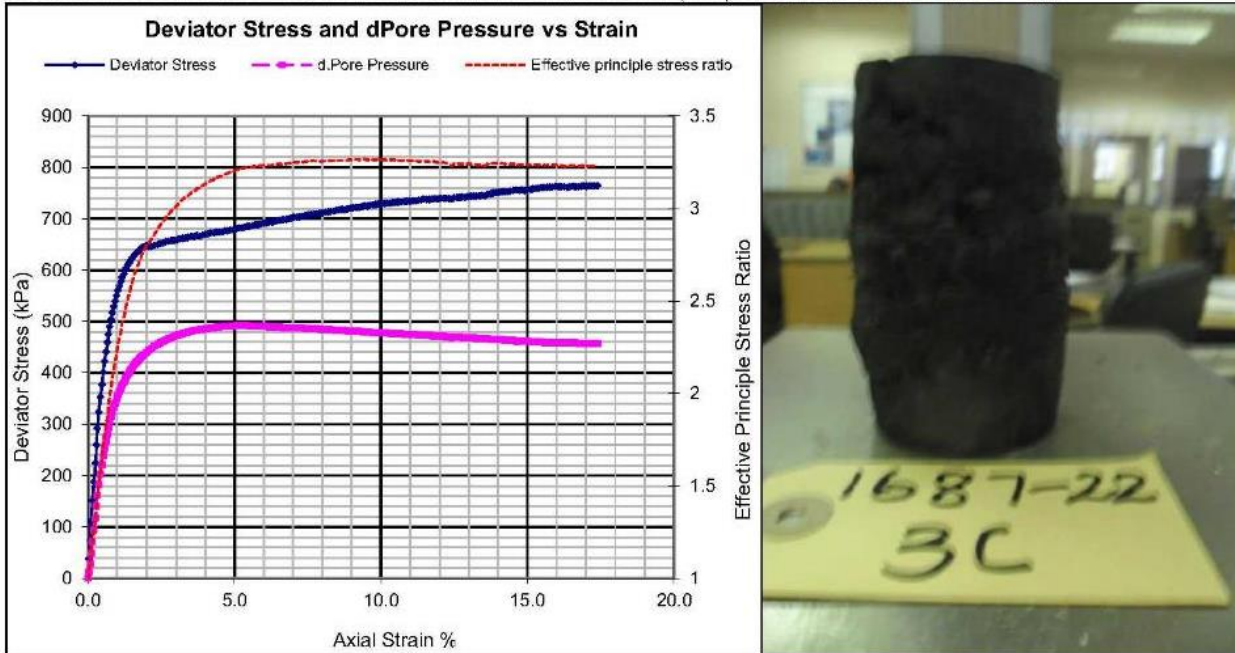
Effective cons. Stress (kPa):	799.7	t100 (minutes):	169	Side drains fitted:	Yes			
	Height mm	Diameter mm	Area mm ²	Moisture Content %	Dry Density kg/m ³	Void Ratio	Saturation %	Specific Gravity
INITIAL (Before saturation)	*105.72	*50.84	2030.02	9.8	1831	0.4934	55	2.734 Determined
CONSOLIDATED	104.13	50.07	1968.78	13.8	1918	0.4258	89	
FINAL (After shear)	86.01	55.09	2383.39	13.8	1917	0.4265	88	
Initial pore pressure (kPa):	341.8		Final pore pressure (kPa):	342.4		PWP dissipation (%): 99		

*: Measured dimensions; all other dimensions are calculated.

SHEAR DATA

Rate of strain (%/hour):	0.79		
Initial pore pressure (kPa):	341.3	Initial effective stress (kPa):	799.7
Parameters at failure:			
Failure Criterion:	Max. Effective Principle Stress Ratio		
Axial strain (%):	9.44		
Deviator stress (kPa):	725.4	Principle Stresses (kPa)	
Excess pore pressure (kPa):	479.8	σ_1	σ_1'
Effective principle stress ratio:	3.267	1525.1	1045.3
		799.7	319.9

Deviator stress corrections: Membrane correction: 2.2 kPa & Side drain(filter) correction: 7 kPa for Strains >= 2%



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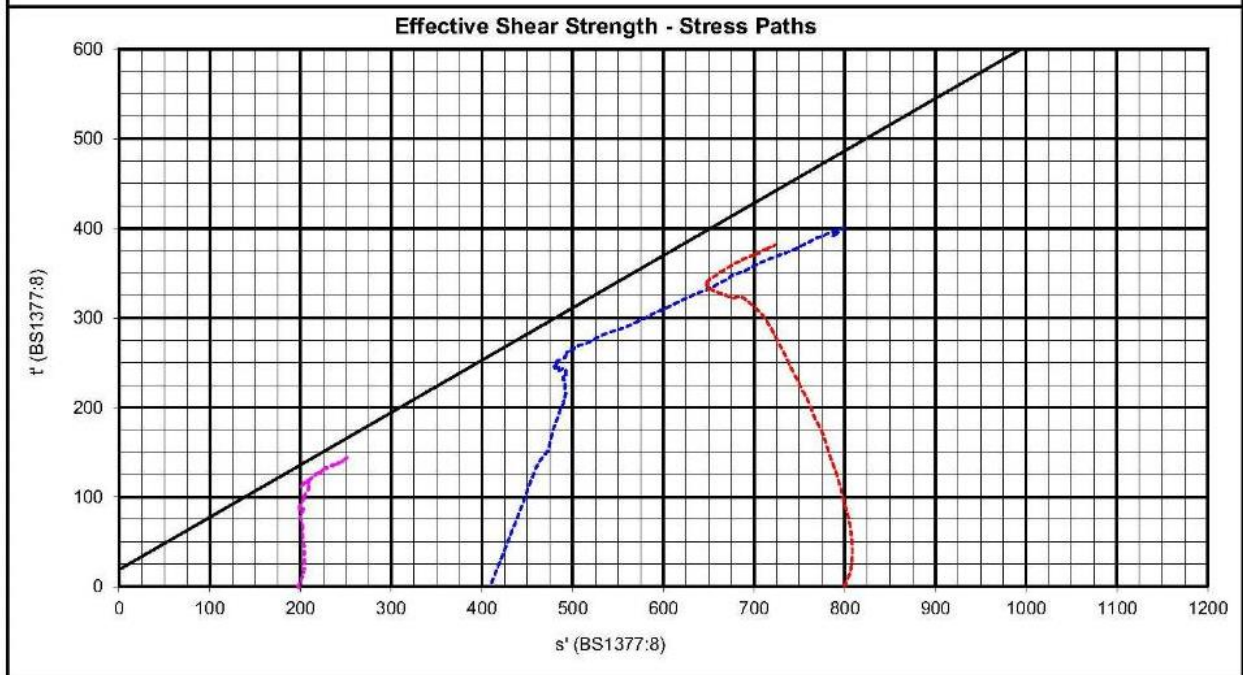
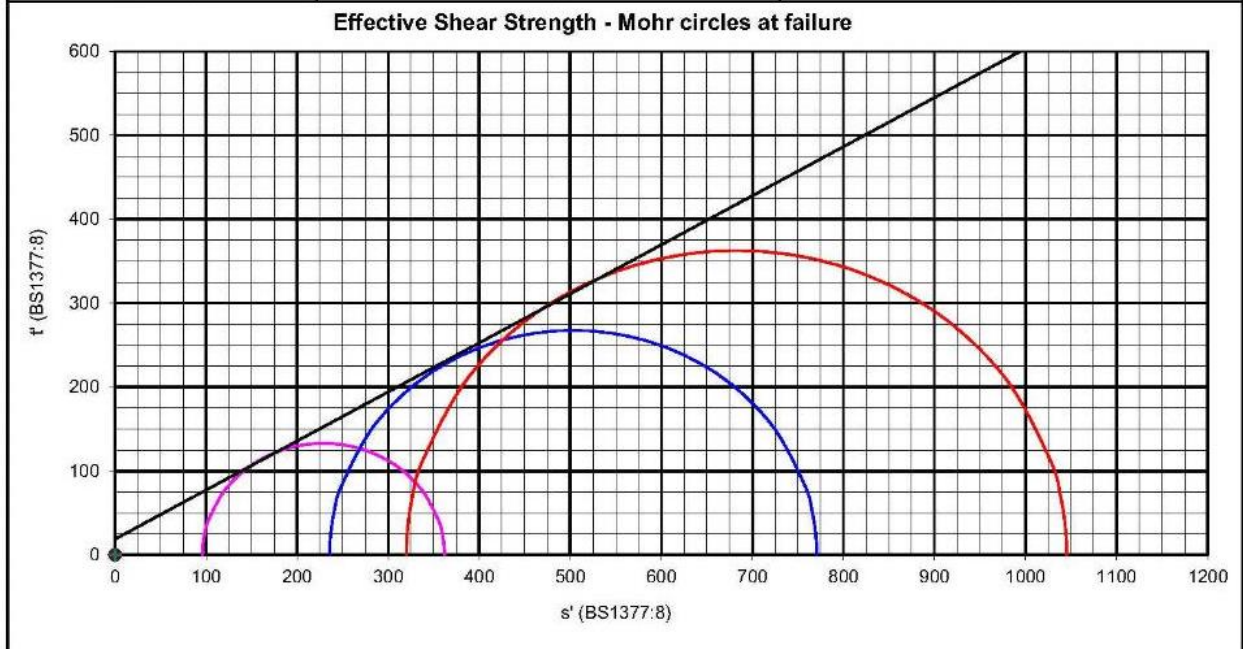
Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-22
Field Sample Reference:	FTP - Mix	Depth (m):	-

Effective Shear Strength Parameters

Stresses	Cohesion (kPa)	Internal friction (Degrees)
Total	60.5	15.7
Effective	18.9	30.3



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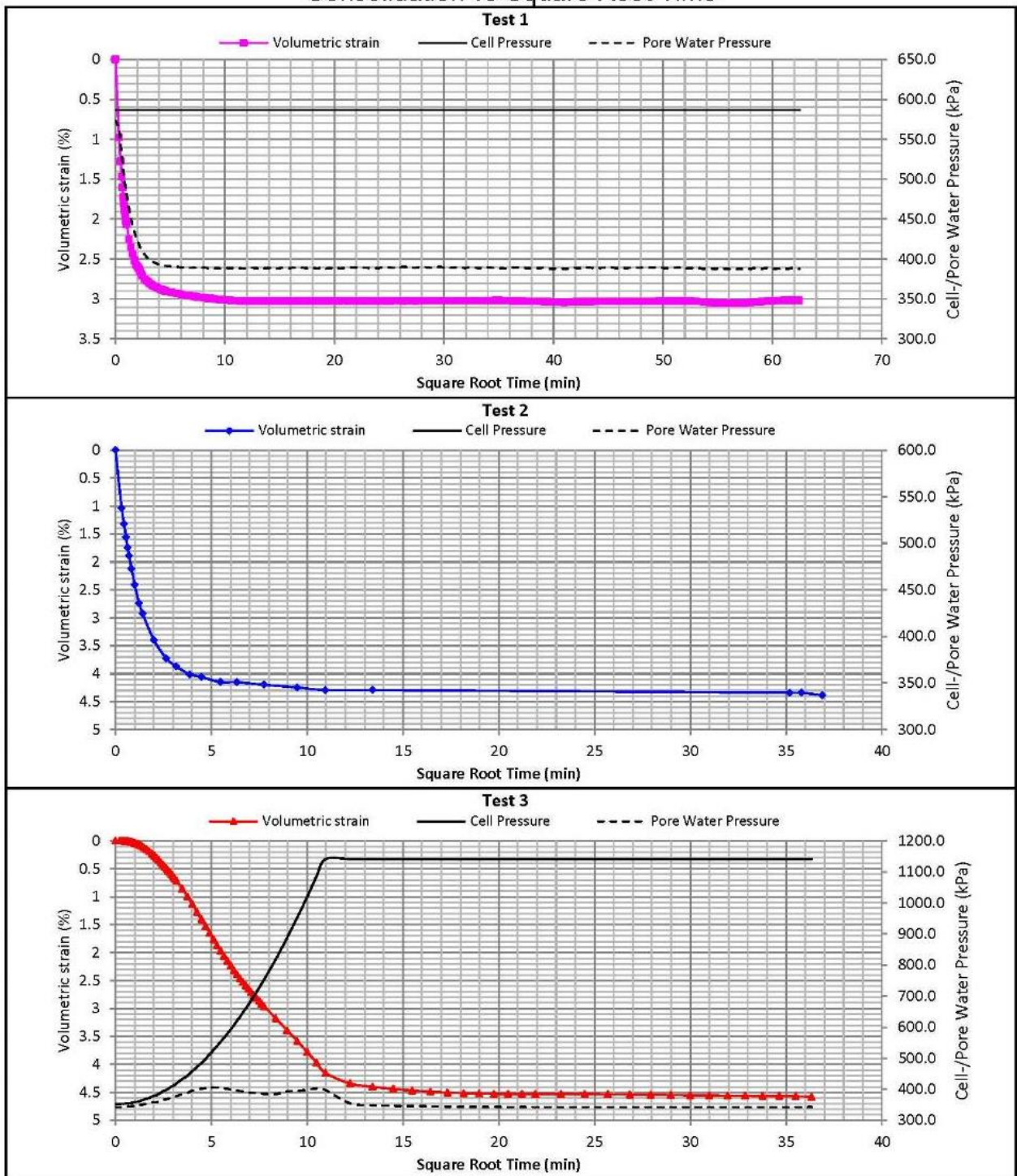


Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Mzimvubu Water Project	Date Received:	2014/08/12
Job Number:	2014-B-1687	Laboratory Number:	1687-22
Field Sample Reference:	FTP - Mix	Depth (m):	-

Consolidation vs Square Root Time



Civilab (Proprietary) Limited. Registration No.: 1998/019071/07

36/38 Fourth Street, Booysens Reserve, Johannesburg 2091
 P O Box 82223, Southdale 2135
 Tel: +27 (0)11 835-3117 • Fax: +27 (0)11 835-2503
 Email: jhb@civilab.co.za • Website: www.civilab.co.za



Civil Engineering Testing Laboratories

Consolidation Tests

Project:	MZIMVUBU WATER PROJECT						Test 1
Project No.:	2014-B-1687	Sample No.:	1687-21				
Borehole No:	CTP : MIX		Depth:	-			
Date Received:	12/08/2014		Date Tested:	22/10/2014			
Remarks:	An undisturbed sample tested soaked.						
Machine No.	6	Ring No.	35	Height (mm)	20.7	Diameter (mm)	76.2

Masses for Water Content Determination (g)

Wet Sample and Ring		Dry Sample and Ring	Ring Only	Water Content	
Before Test	After Test			Before Test	After Test
265.5	266.0	232.1	84.97	22.7%	23.0%

Pre-Determined Particle Specific Gravity | 2.555

Initial Parameters

Void Ratio	0.6393	Degree of Saturation (%)	90.7	Dry Density (Kg/m ³)	1559
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Effect. Stress (kPa)	10	50	100	200	400	800	1600	400	100	10	0
Dial Correction (u)	0	50	82	122	189	299	417	231	117	42	0
HH:MM:SS	Minutes	Dial Readings in Microns									Initial Dial Reading
00:00:00	0.00	13592									13592
12:00:00	26.83			13246							
18:00:00	32.86									12602	
19:00:00	33.76								12239		
20:00:00	34.64				12980		11635				
24:00:00	37.95					12425					
76:00:00	67.53		13654	13489				11977			
96:00:00	75.89	13776									
End of Primary Cons		13776	13654	13489	13246	12980	12425	11635	11977	12239	12602
Number of Readings:		2	1	1	1	1	1	1	1	1	1

36/38 Fourth Street, Booysens Reserve, Johannesburg 2091
 P O Box 82223, Southdale 2135
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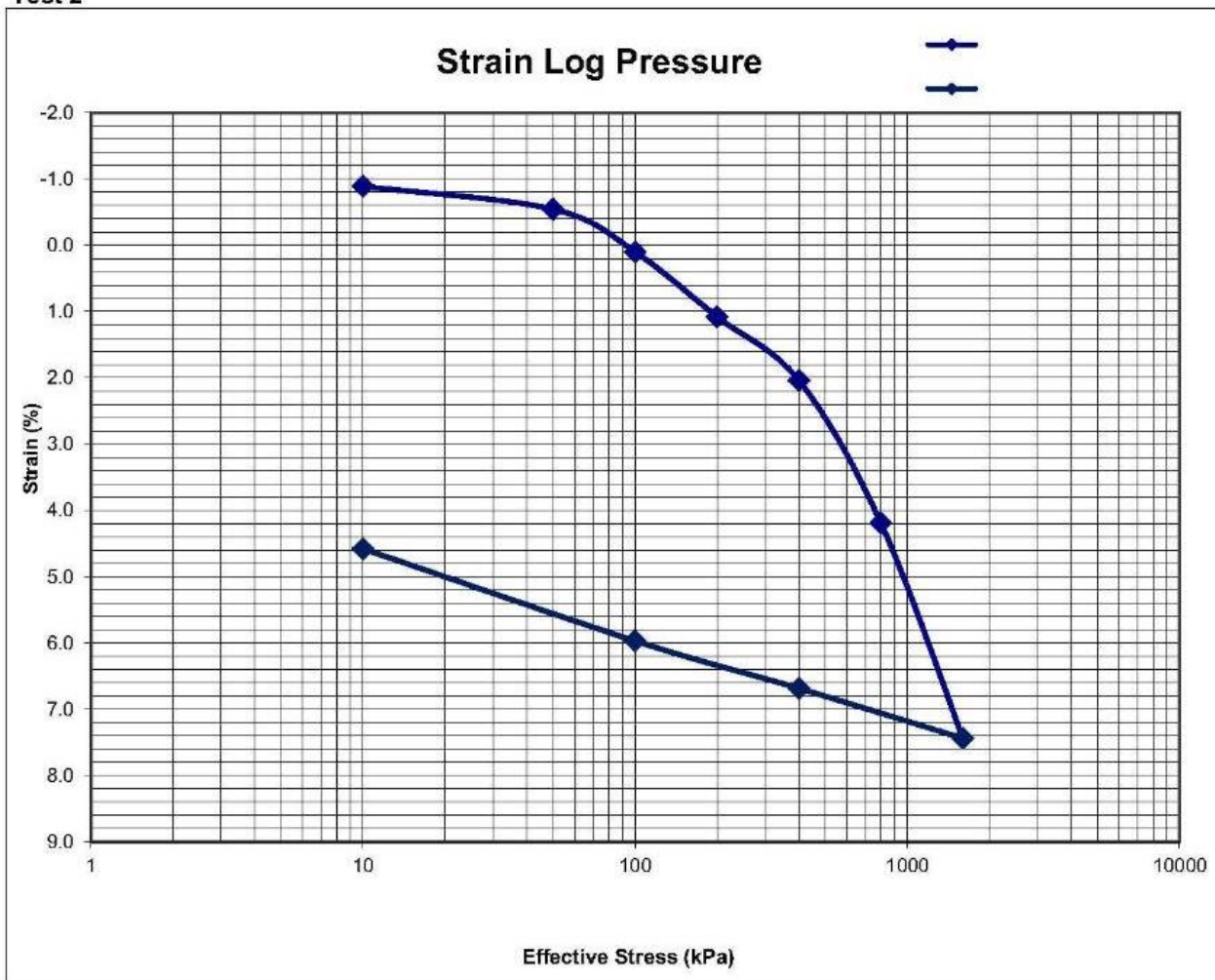
Consolidation Tests

Project: MZIMVUBU WATER PROJECT	
Project No.: 2014-B-1687	Sample No.: 1687-21
Borehole No: CTP : MIX	Depth: -
Date Received: 12/08/2014	Date Tested: 22/10/2014

Test 1

Effect.Stress (kPa)	10	50	100	200	400	800	1600	400	100	10
Strain (%)	-0.89	-0.54	0.10	1.08	2.04	4.19	7.44	6.69	5.97	4.58
Mv (1/MPa)		0.0870	0.1285	0.0981	0.0481	0.0537	0.0406	0.0063	0.0238	0.1546
Void Ratio	0.6539	0.6482	0.6376	0.6216	0.6058	0.5706	0.5173	0.5297	0.5414	0.5642

Test 2



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36/38 Fourth Street, Booyens Reserve, Johannesburg 2091
 P O Box 82223, Southdale 2135
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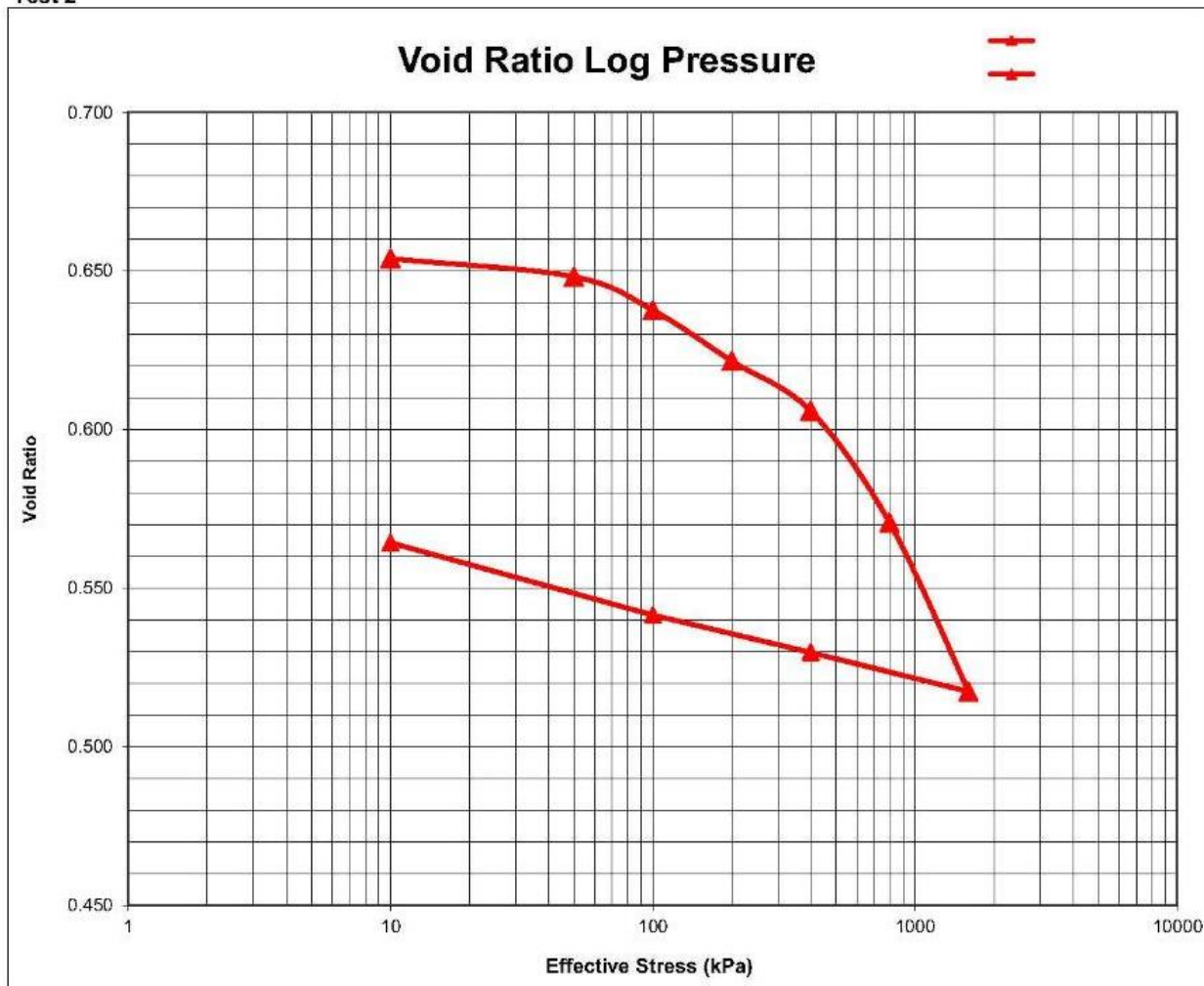
Consolidation Tests

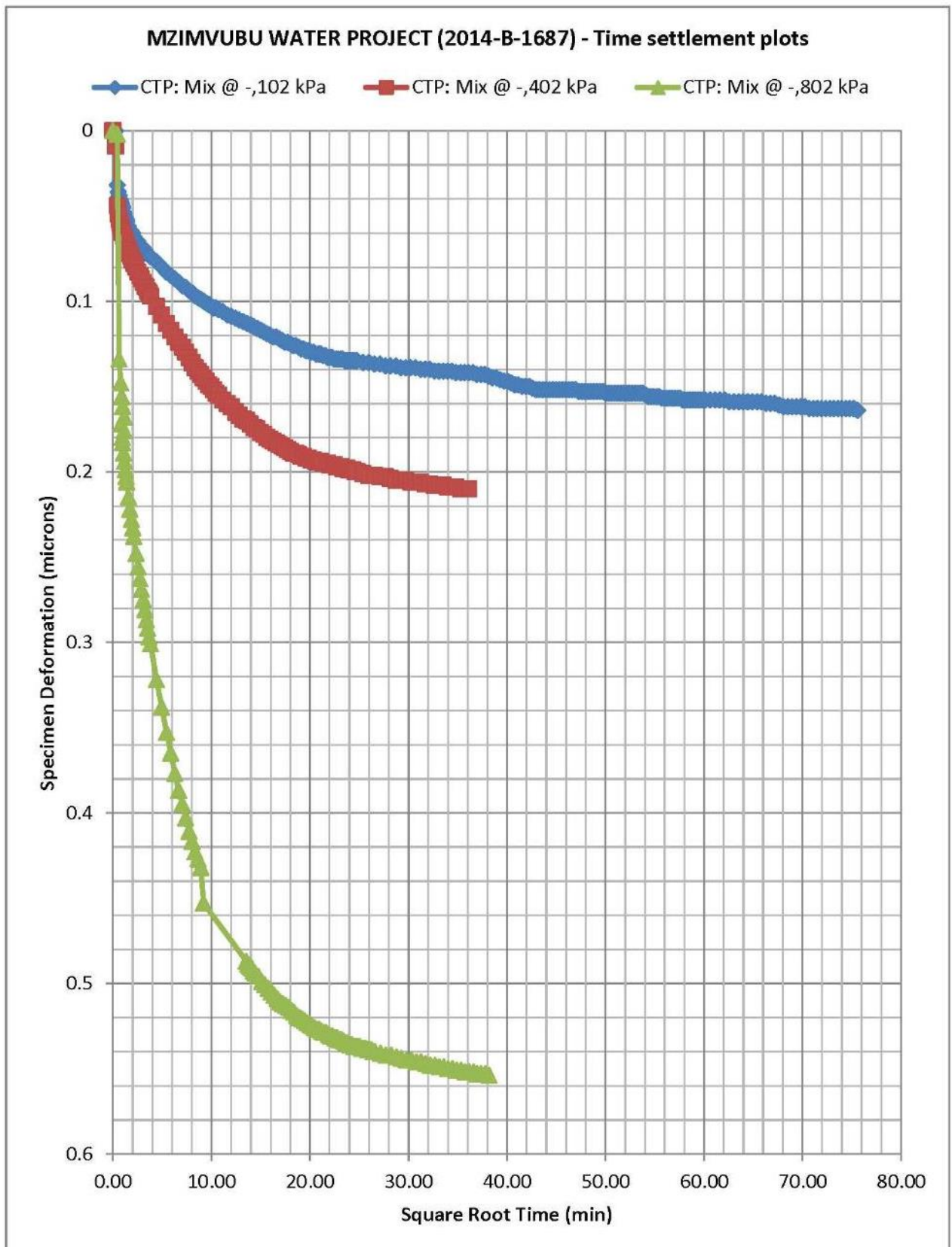
Project: MZIMVUBU WATER PROJECT	
Project No.: 2014-B-1687	Sample No.: 1687-21
Borehole No: CTP : MIX	Depth: -
Date Received: 12/08/2014	Date Tested: 22/10/2014

Test 1

Effect. Stress (kPa)	10	50	100	200	400	800	1600	400	100	10
Strain (%)	-0.89	-0.54	0.10	1.08	2.04	4.19	7.44	6.69	5.97	4.58
Mv (1/MPa)		0.0870	0.1285	0.0981	0.0481	0.0537	0.0406	0.0063	0.0238	0.1546
Void Ratio	0.6539	0.6482	0.6376	0.6216	0.6058	0.5706	0.5173	0.5297	0.5414	0.5642

Test 2





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Civil Engineering Testing Laboratories

Consolidation Tests

Project:	MZIMVUBU WATER PROJECT					Test 1	
Project No.:	2014-B-1687	Sample No.:	1687-22				
Borehole No:	FTP : MIX	Depth:	-				
Date Received:	12/08/2014	Date Tested:	22/10/2014				
Remarks:	An undisturbed sample tested soaked. Initial swell was verified.						
Machine No.	5	Ring No.	36	Height (mm)	19.4	Diameter (mm)	76.1

Masses for Water Content Determination (g)

Wet Sample and Ring		Dry Sample and Ring	Ring Only	Water Content	
Before Test	After Test			Before Test	After Test
279.2	286.0	260.4	85.2	10.7%	14.6%

Pre-Determined Particle Specific Gravity | 2.578

Initial Parameters

Void Ratio	0.2984	Degree of Saturation (%)	92.7	Dry Density (Kg/m ³)	1986
------------	--------	--------------------------	------	----------------------------------	------

Effect. Stress (kPa)	10	50	100	200	400	800	1600	400	100	10	0
Dial Correction (u)	0	37	68	101	170	220	321	196	112	45	0
HH:MM:SS	√Minutes	Dial Readings in Microns								Initial Dial Reading	13053
00:00:00	0.00	13053									
03:00:00	13.42							12507			
08:00:00	21.91				12965						
17:00:00	31.94					12696					
18:00:00	32.86								12722		
22:00:00	36.33						12358				
24:00:00	37.95			13198							
72:00:00	65.73	13612	13497							13031	
78:00:00	68.41			13373							
End of Primary Cons		13612	13497	13373	13198	12965	12696	12358	12507	12722	13031
Number of Readings:		2	1	1	1	1	1	1	1	1	1

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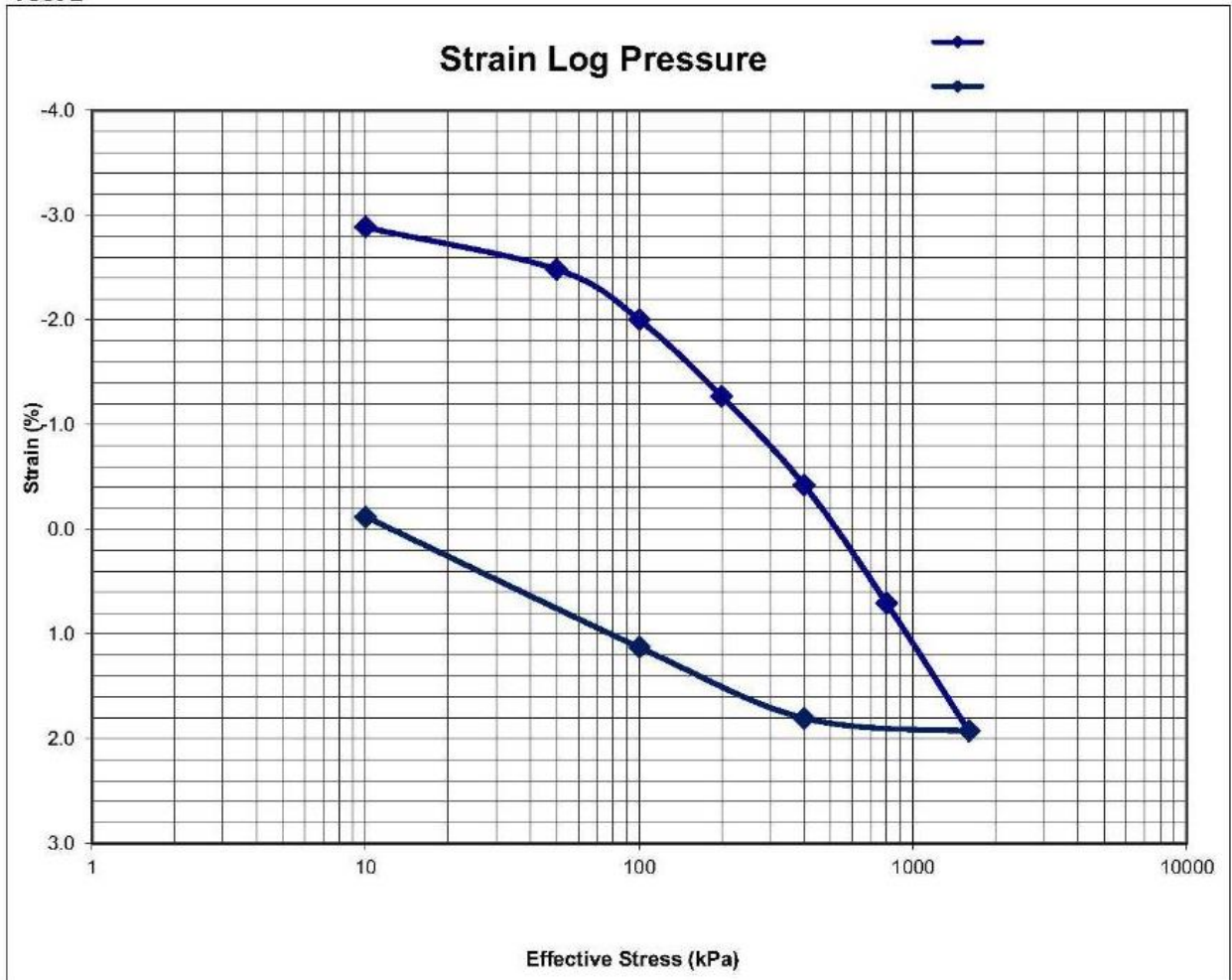
Consolidation Tests

Project: MZIMVUBU WATER PROJECT	
Project No.: 2014-B-1687	Sample No.: 1687-22
Borehole No: FTP : MIX	Depth: -
Date Received: 12/08/2014	Date Tested: 22/10/2014

Test 1

Effect Stress (kPa)	10	50	100	200	400	800	1600	400	100	10
Strain (%)	-2.88	-2.48	-2.00	-1.27	-0.42	0.71	1.93	1.80	1.13	-0.12
Mv (1/MPa)		0.1005	0.0959	0.0732	0.0423	0.0282	0.0153	0.0010	0.0225	0.1386
Void Ratio	0.3358	0.3306	0.3244	0.3149	0.3039	0.2892	0.2734	0.275	0.2837	0.2999

Test 2



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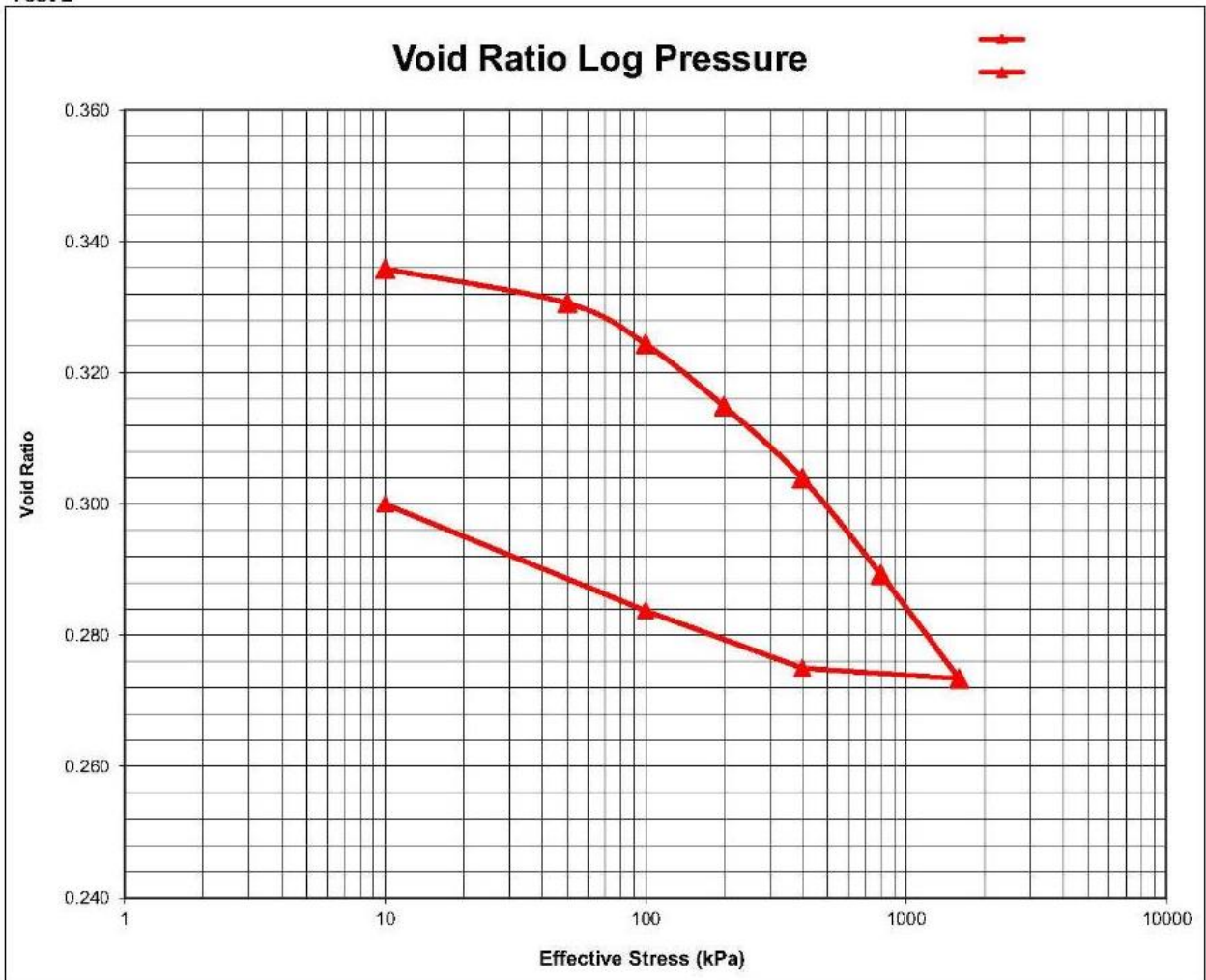
Consolidation Tests

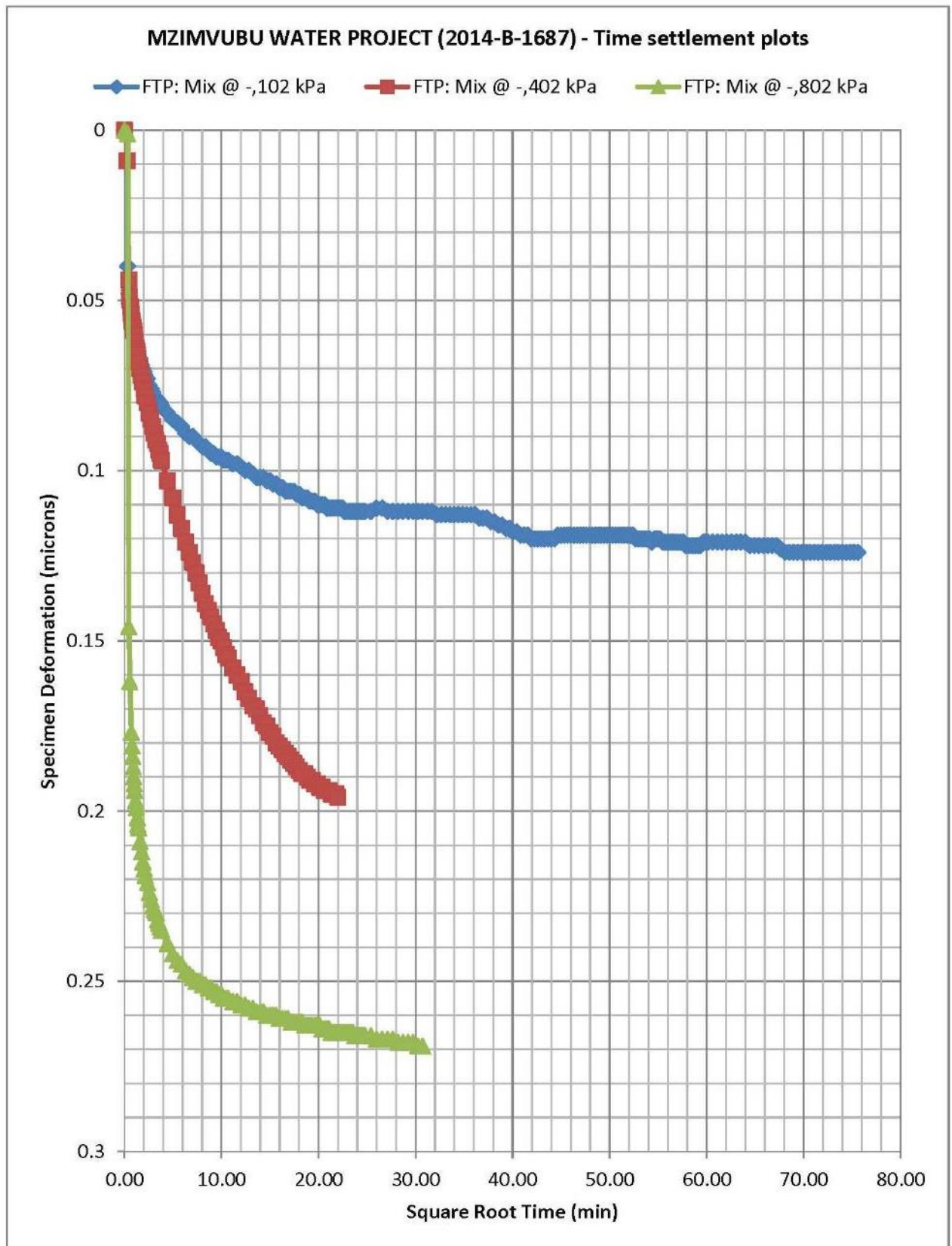
Project: MZIMVUBU WATER PROJECT	
Project No.: 2014-B-1687	Sample No.: 1687-22
Borehole No: FTP : MIX	Depth: -
Date Received: 12/08/2014	Date Tested: 22/10/2014

Test 1

Effect. Stress (kPa)	10	50	100	200	400	800	1600	400	100	10
Strain (%)	-2.88	-2.48	-2.00	-1.27	-0.42	0.71	1.93	1.80	1.13	-0.12
Mv (1/MPa)		0.1005	0.0959	0.0732	0.0423	0.0282	0.0153	0.0010	0.0225	0.1386
Void Ratio	0.3358	0.3306	0.3244	0.3149	0.3039	0.2892	0.2734	0.275	0.2837	0.2999

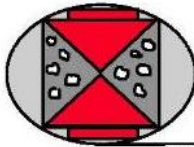
Test 2





E6:

WATER TEST RESULTS



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Ref: CJ14/08/1015 7 October 2014
Client: Jeffares & Green
Subject: Water Testing
Project: Tsitsa River
O/N:

LABORATORY REPORT

CLIENT

Jeffares & Green (Pty) Ltd, P O Box 794, HILTON, 3245

SYNOPSIS:

Physical and chemical tests on a sample of water have been provided.

BRIEF FROM CLIENT:

Contest were requested to analyse the samples for;

- Total dissolved solids
- Chloride content
- Calcium hardness as CaCO₃
- pH
- Sulphates (as SO₃)
- Comparative Cubes

SAMPLES:

A 11 litre sample, Tsitsa River, was received on 15.08.2014.

TESTING:

The following test methods were used;

- Total dissolved solids - SABS Standard Method 213.
- Chloride content - Volhard method typically as described in Quantitative Analytical Chemistry by Fritz & Schenk.
- Calcium Hardness of water - SABS Standard Method 216. This does not give the total carbonates and bicarbonates, but simply the calcium hardness or equivalent calcium carbonate in mg/l.
- pH - as described in the operating manual of our pH meter - a WTW inoLab pH720 meter and using Hanna buffer solutions for calibration.

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Managing Member: R.J.L Raw B Tech (Civil Eng)
Members: MT Clark, JS Dunnett, MC Mzobe, VA Horton
Consultant: A J M Horton Pr Tech (Eng), Dip ACT, HND (Chem), HNC (Civ. Eng), FICT, MSA Corr I

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 Subject: Water Testing
 Project: Tsitsa River
 O/N:

- Sulphate - SABS Standard Method 212.
- Comparative Cubes

All tests were carried out in duplicate i.e. an A and B sample and the average values determined and reported.

RESULTS:

Chemical Testing:

		Client Sample
TDS	(mg/l)	105
Chloride (Cl ⁻)	(mg/l)	16
Calcium Hardness as CaCO ₃	(mg/l)	71
pH		8.51
Sulphate (SO ₃)	(mg/l)	None detected

Physical Testing:

Comparative cubes:

	Control	Sample	Sample as % of Control
	MPa	MPa	%
24hr	5.0	4.5	90
3d	12.4	11.7	94
7d	19.7	18.6	94
28d			

COMMENT:

The results indicate that the water sample is suitable for concrete manufacture.

We have used the following limits based on various sources for a number of years.

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Subject: Water Testing
Project: Tsitsa River
O/N:

	(mg/l)
Total dissolved solids	2000
Chloride (as Cl ⁻)	500
Calcium Hardness(as CaCO ₃)	400
pH	6 to 8
Sulphate as SO ₃	1000

These values seem to be conservative based on some of the latest publications; in particular the upper range of pH seems to have been increased substantially, probably with the experience of using wash water in RMC plants.

The ninth edition of Fulton's Concrete Technology states that the mean compressive strength at **seven** days of the test specimen, prepared with the test water, shall be 90% of the mean compressive strength of the corresponding specimens prepared with distilled or deionised water.

However, the eight edition of Fulton's Concrete Technology states that the compressive strength of 'test' cubes at 28 days should be no less than 90% of the control cubes.

BS 3148 has suggested that if the comparative strength is between 80 and 90%, then the water can still be used if the concrete mix proportions are adjusted.

Compressive strength of cubes at 24 hours and 3 days was carried out to determine any early age effect on the concrete strength.



R J L Raw
B Tech (Civil Eng)



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 Unit 5, 89 King Dinuzulu (Berea) Road,
 Durban, 4001, South Africa
 Post Net Suite #290 Private Bag X04
 Dalbridge, 4014, South Africa
 Tel: +27 31 301 1257
 Fax: +27 31 301 1256
 Email: info.dbn@alsglobal.com
 www.alsglobal.com

REFERENCE No: ALSD 3757

DATE: 02 October 2014

CERTIFICATE OF ANALYSIS

Report On: 3 (Three) Samples I.D.: Water
 Date & Time Received: 29/09/14 – 14:00 Taken By: Yourselfes
 Date & Time Analysis Started: 01/10/14 – 14:00 From: Monika
 Date & Time Analysis Finished: 02/10/14 – 15:05
 MARKED: **AS PER BELOW.**

Results marked with “*” refers to tests that are “Not SANAS Accredited” in this report and are not included in the SANAS Schedule of Accreditation for this Laboratory.

Analysis on an as received basis:

	*Ammonia, as NH ₄ mg/l [Kjeldahl Distillation]	Magnesium, as Mg, mg/l [ICP]	Dissolved Sulphates, as SO ₄ mg/l [Gravimetric]
1. 1015/2 – BH T4.....	<1	25	21
2. 1015/3 – BH T5.....	<1	35	17
3. 1015/4 – BH T6.....	<1	33	20

Technical Signatory: Chemistry Mr P. Ramdeen Microbiology Ms N. Kassim

Management Signatory: Mr P. Ramdeen

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 3650

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Ref: CJ14/08/1015a 27 October 2014
Client: Jeffares & Green
Subject: Water Testing
Project: Tsitsa River
O/N:

LABORATORY REPORT - ADDENDUM

CLIENT

Jeffares & Green (Pty) Ltd, P O Box 794, HILTON, 3245

SYNOPSIS:

Physical tests on a sample of water have been provided.

BRIEF FROM CLIENT:

Contest were requested to analyse the samples for;

- Setting Times

SAMPLES:

A 11 litre sample, Tsitsa River, was received on 15.08.2014.

TESTING:

The following test methods were used;

- Setting Times based on ASTM C403-99

RESULTS:

Physical Testing:

SETTING TIMES:

Mix Reference	Initial Set (mins)	Final Set (mins)
(Test) Client Water	367	479
(Control) Lab Water	328	378

ASTM C403-99 states that setting time of concrete is an arbitrary value and has been taken to be as follows, as given in ASTM C403-99:

Initial set: 3.5MPa (measured as penetration resistance)

Final set: 27.6MPa (measured as penetration resistance)

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Project: Tsitsa River
O/N:

The test results given here have been determined under laboratory conditions using the test method described in ASTM C403-99.

These results obtained have therefore been achieved in laboratory controlled environmental conditions. The temperature of the laboratory (and therefore the test specimens) was maintained between 22 and 25 deg. C. The humidity in the laboratory during the testing was between 50 and 55%. There was no exposure of the test specimens to the sun, precipitation or any wind.

Concrete setting times and rates of gain of strength (amongst other concrete characteristics) are significantly affected by temperature, humidity, precipitation, exposure to the sun and wind speed (amongst other factors). However, the test specimens are covered by an impervious material (as required by the test method) to prevent evaporation therefore, the humidity at the surface of the specimens in the curing environment is most likely higher than the laboratory humidity due to the micro climate that exists in the area confined in the space above the concrete and beneath the impervious covering, of the test specimen. This higher humidity would be created by the evaporated water at the surface of the concrete specimen.

Taken into account of the factors mentioned above that effect the setting time and rate of gain of strength of concrete, it is likely that the setting time and rate of gain of strength of concrete under site conditions could be significantly different to those obtained using standard laboratory test methods. This will depend on the site conditions at the time when these factors have an influence on the concrete.

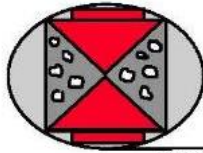
It would be advisable to make some attempt to determine by some method, the setting time and rate of gain of concrete strength for concrete on site at the relevant time. This will give a realistic value for these characteristics under the prevailing site conditions at that time.

COMMENT:

It can be seen from the results given that the initial and final setting times of the test water extended by 39 minutes and 101 minutes respectively, when compared to those of the control water. The control water is normal municipal supply as received at the laboratory in Westmead.



R J L Raw
B Tech (Civil Eng)



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Ref: CJ14/08/1015 6 October 2014
 Client: Jeffares & Green
 Subject: Basson Index
 Project: Laleni Dam & Tunnel

Water Analysis – Basson Index

Four bore hole water samples received on 15.08.2014 were analysed, as requested, to determine the Basson Index.

The results of the chemical analyses may be found appended. (The ammonium ion, magnesium and sulphate contents were determined by ALS).

The following indices were calculated as set out in the PCI publication "Deterioration of Concrete in aggressive waters – measuring aggressiveness and taking counter measures" and the subsequent addendum.

Reference	B/H T4	B/H T5	B/H T6
Leaching Corrosion sub index LSCI	305	224	252
Spalling Corrosion sub index SCSi	7	9	9
Aggressiveness Index LSCI + SCSi	312	233	261

The table of recommendations from the publication has been provided below;

TABLE 4: Guidelines for assessing final index (FI)

Final index	Aggressiveness	Recommendation
Under 350	Non- to mildly aggressive	Use concrete class as required for structural design, but see Remarks in Table 9.
350 to 750	Mildly to fairly aggressive	Good concrete design and construction essential. Read Remarks in Table 9.
750 to 1000	Highly aggressive	Identify dominant corrosion sub-index and follow applicable recommendations.
Over 1000	Very highly aggressive	Do not use in contact with unprotected concrete unless recommended anti-corrosive measures can be carried out in full.

No optional corrections have been applied as there are essentially environmental factors and must be provided by yourselves.

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 Members: MT Clark, JS Dunnett, MC Mzobe
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Ref: CJ14/08/1015

6 October 2014

Client: Jeffares & Green

Subject: Basson Index

Project: Laleni Dam & Tunnel

Analytical results: Chemical Analysis

Borehole Reference:

	T4	T5	T6
pH	8.24	7.55	7.64
Calcium carbonate saturated pH	8.11	7.30	7.45
Calcium hardness as CaCO ₃ (mg/l)	79	146	144
Total ammonium as NH ₄ (mg/l)	< 1	< 1	< 1
Magnesium as Mg (mg/l)	25	35	33
Sulphate as SO ₄	21	17	20
Chloride as Cl (mg/l)	101	169	176
Total dissolved solids (mg/l)	607	876	767



A J M Horton
Pr Tech Eng

APPENDIX F

TRIAL PIT LOGS

F1:

PIPELINE TRIAL PIT LOGS

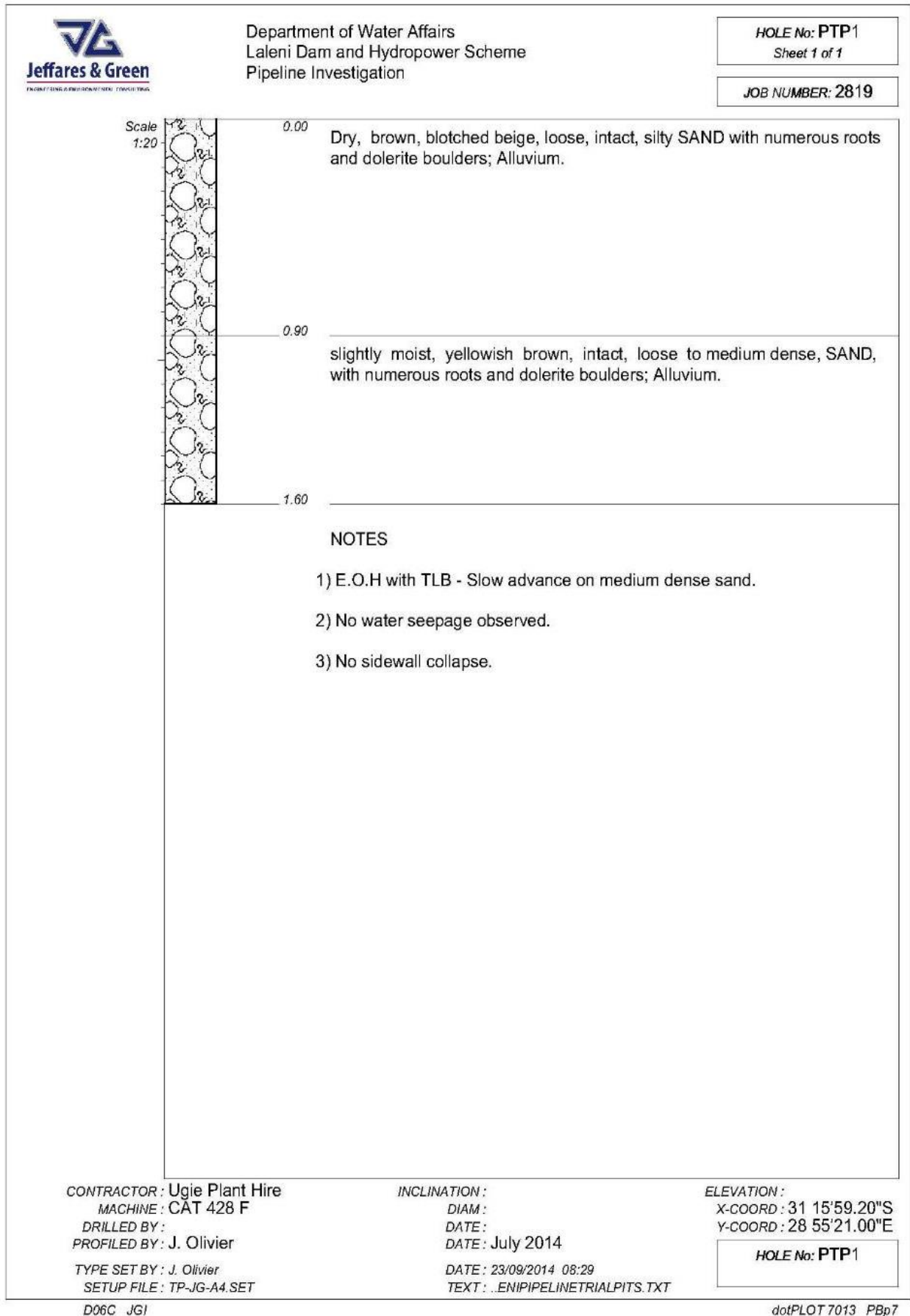


Fig F-1.1: Pipeline Trial Pit 1

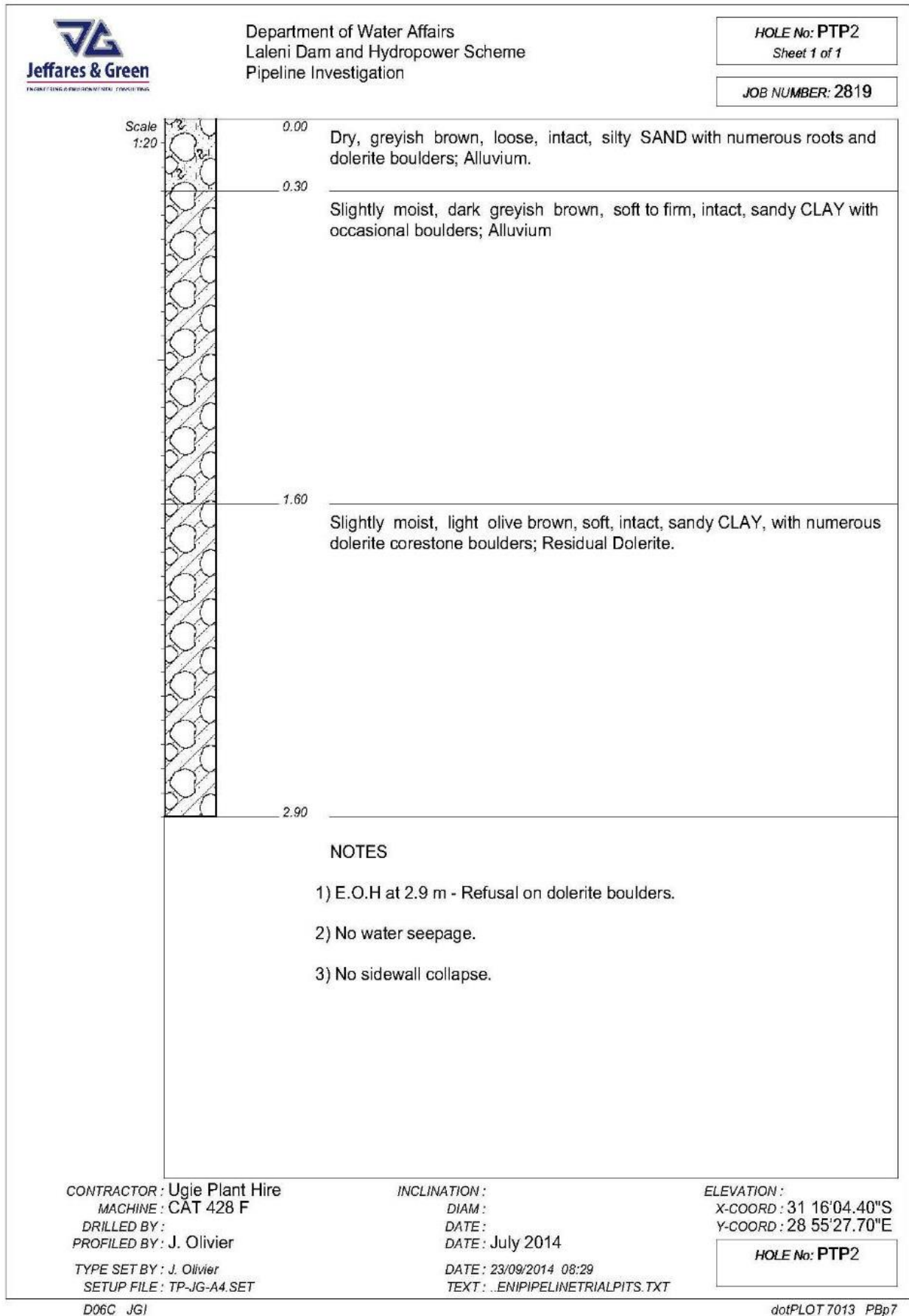


Fig F-1.2: Pipeline Trial Pit 2

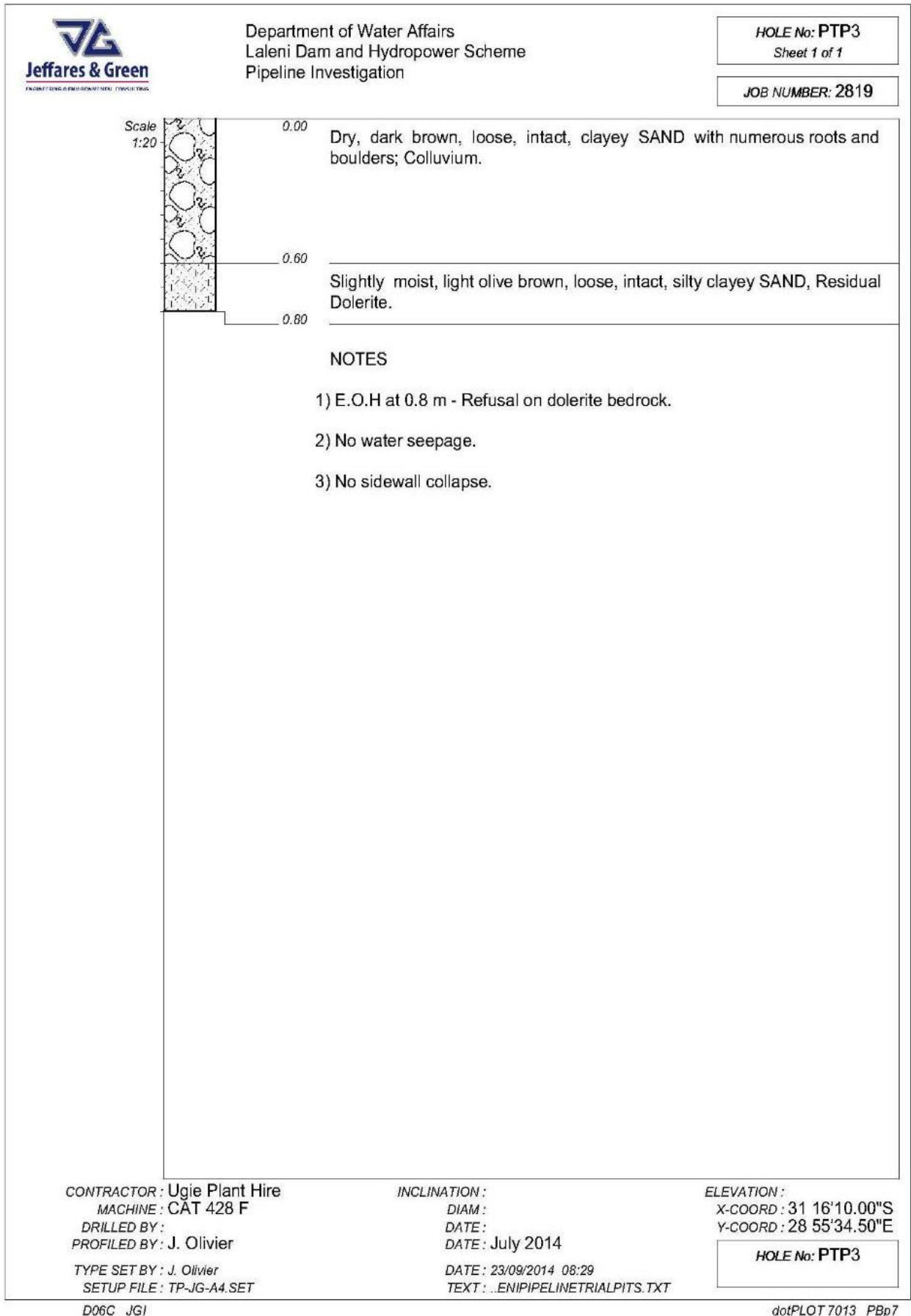


Fig F-1.3: Pipeline Trial Pit 3

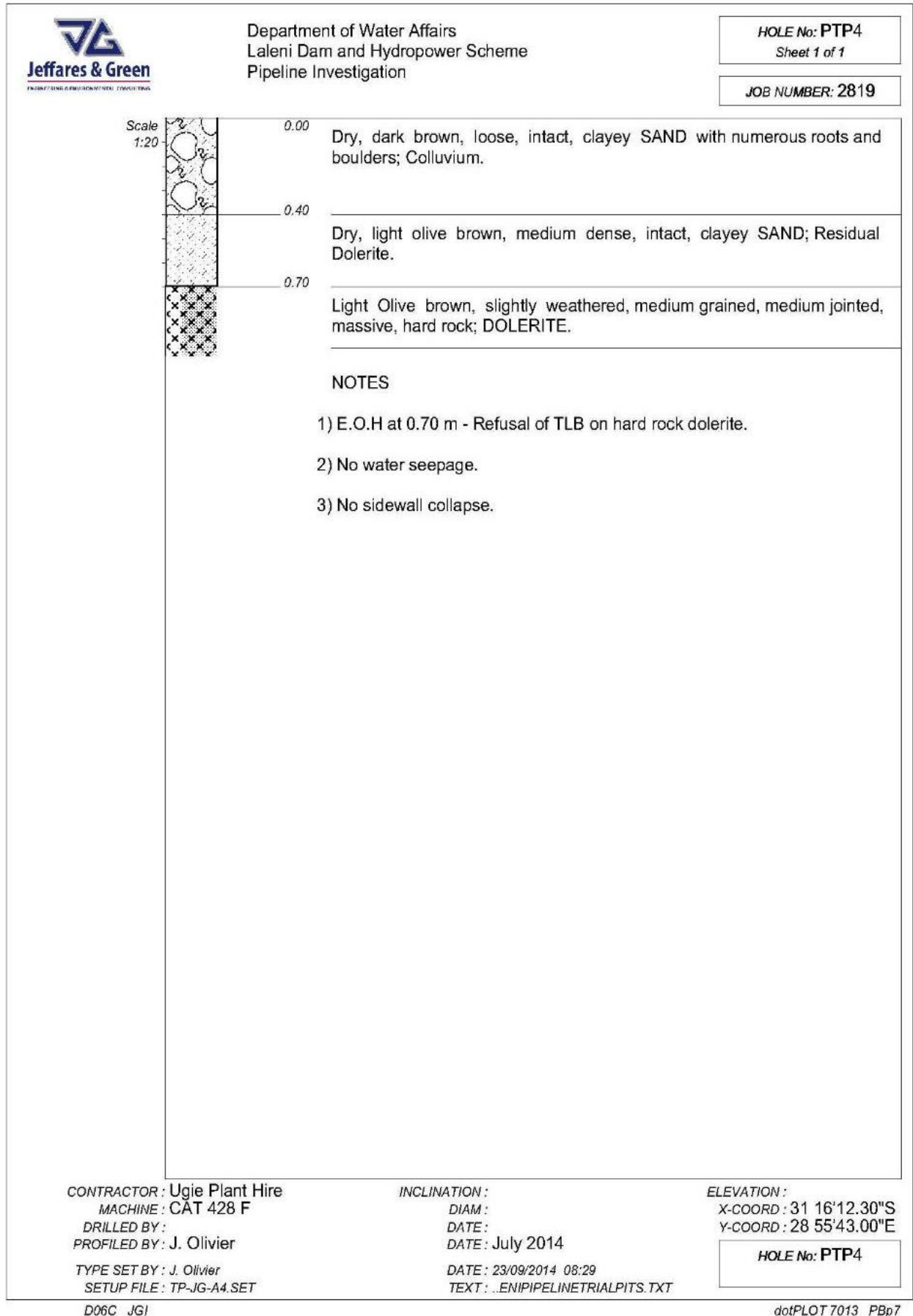


Fig F-1.4: Pipeline Trial Pit 4

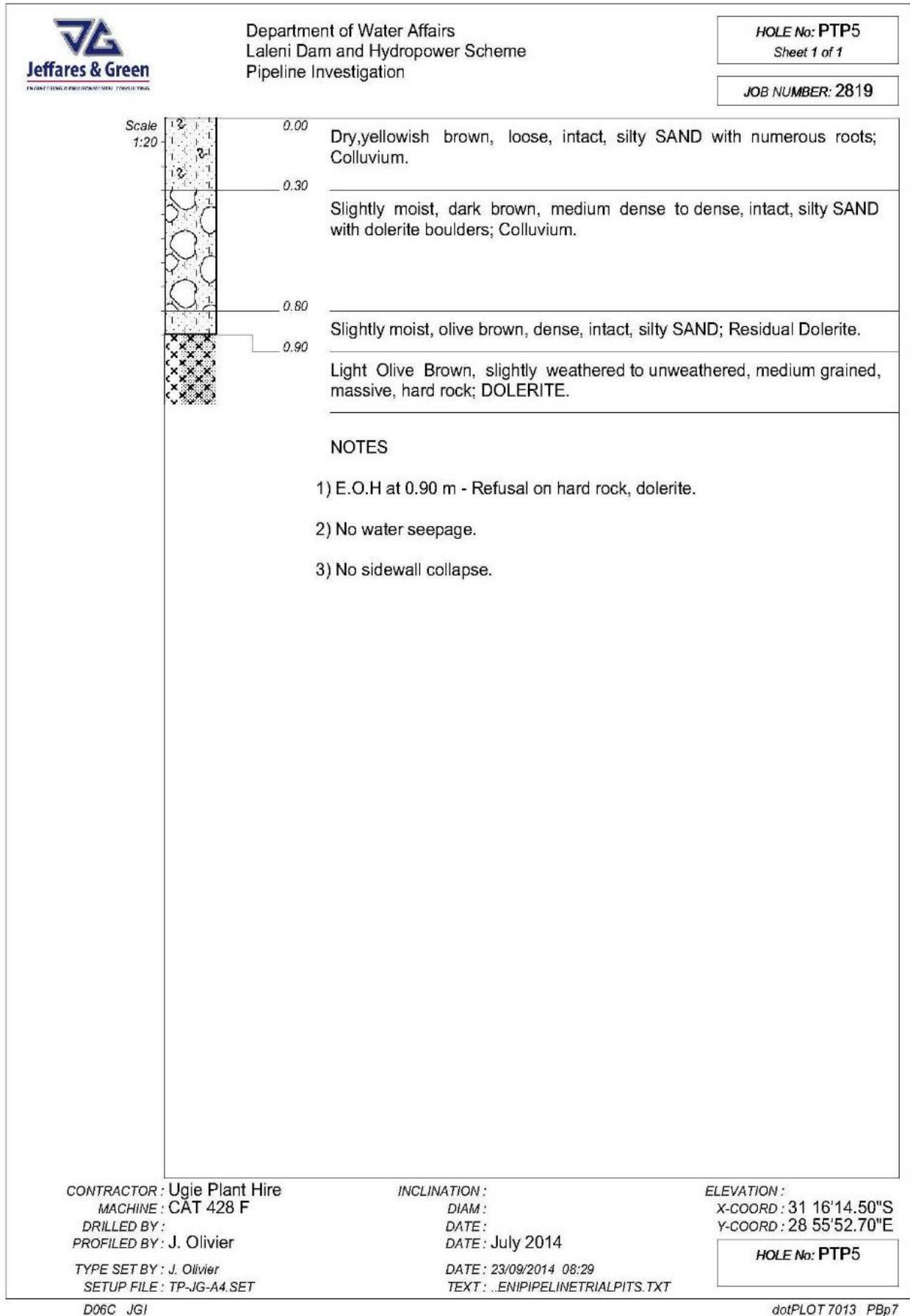


Fig F-1.5: Pipeline Trial Pit 5

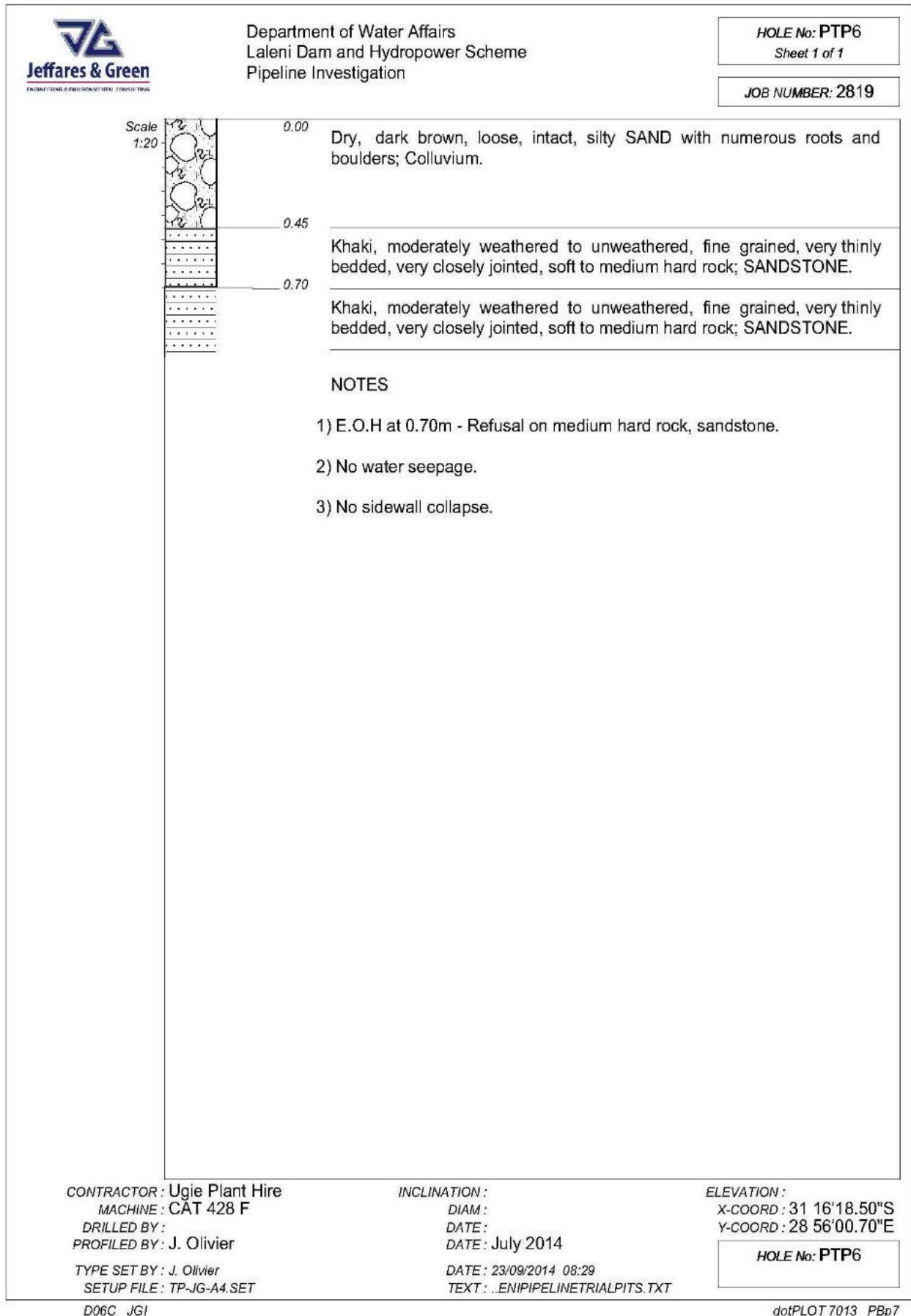


Fig F-1.6: Pipeline Trial Pit 6

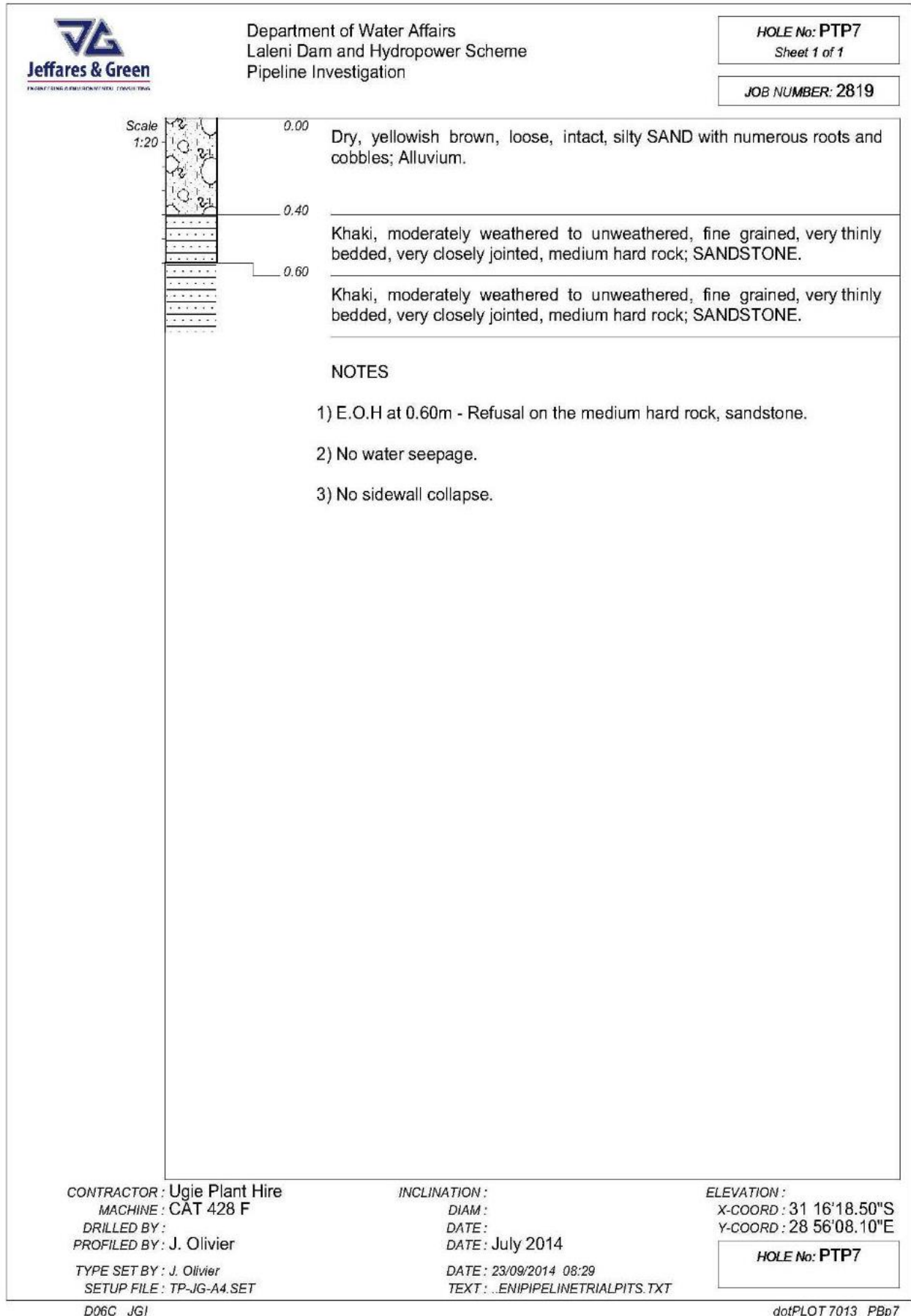


Fig F-1.7: Pipeline Trial Pit 7

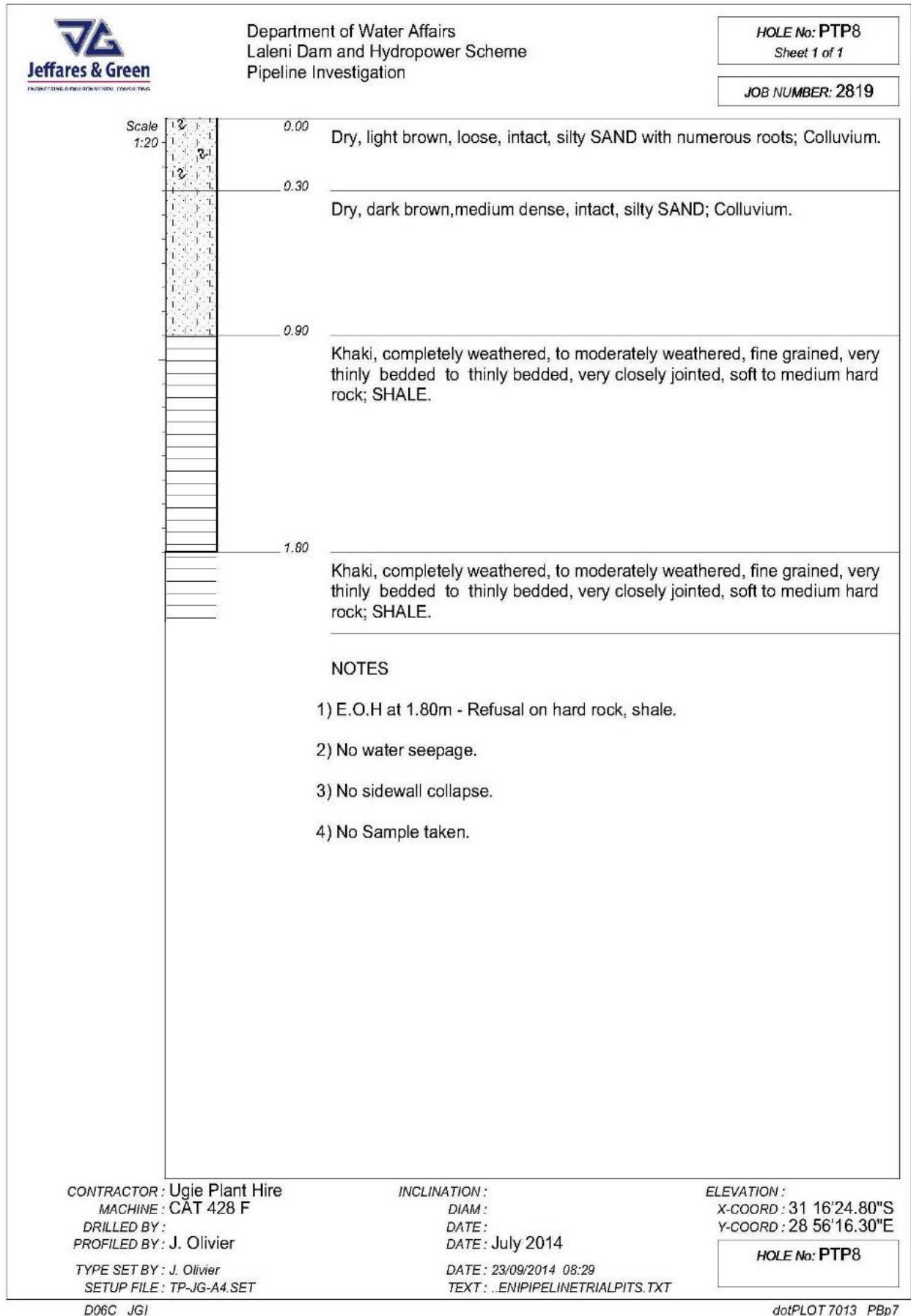


Fig F-1.8: Pipeline Trial Pit 8

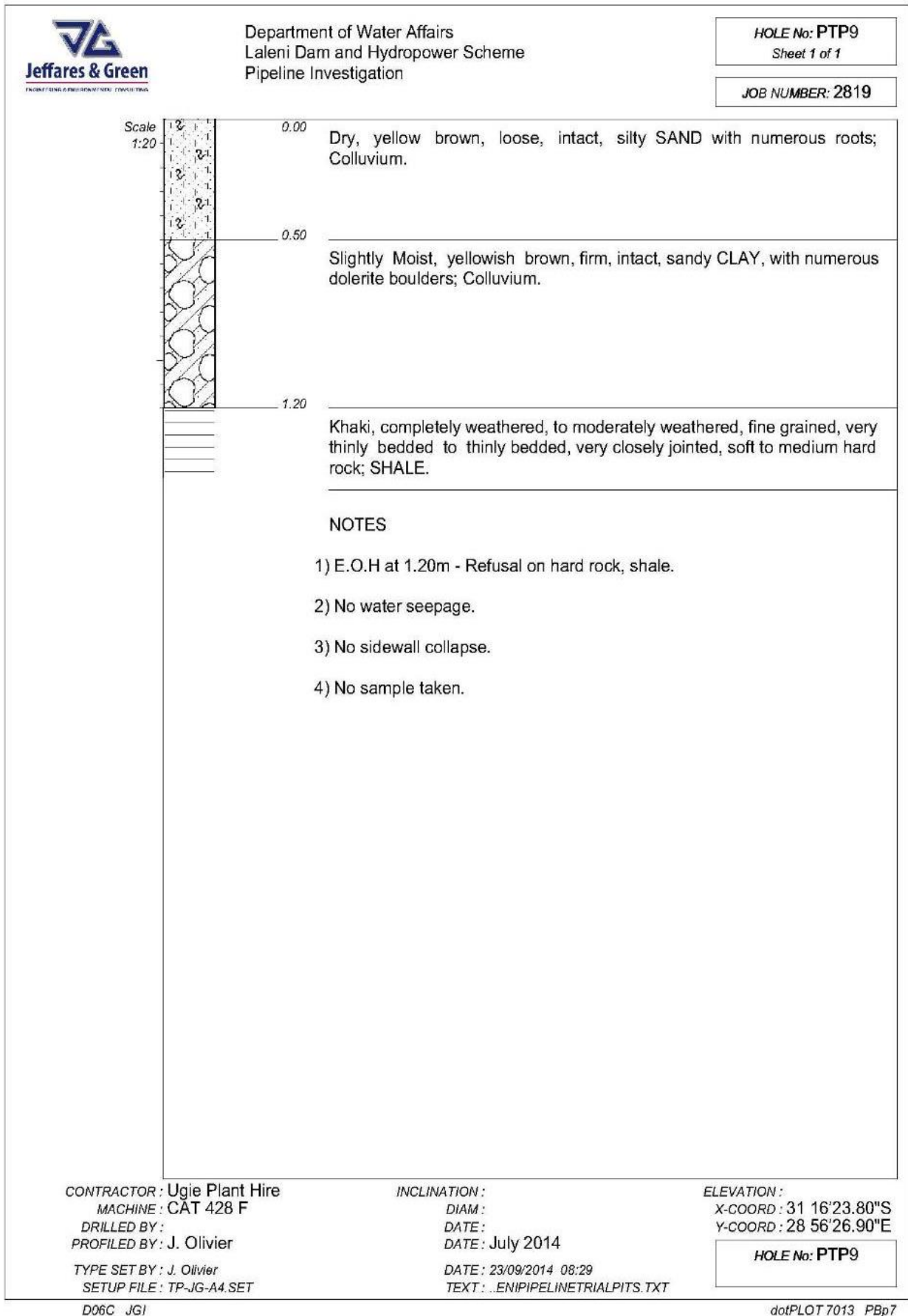


Fig F-1.9: Pipeline Trial Pit 9

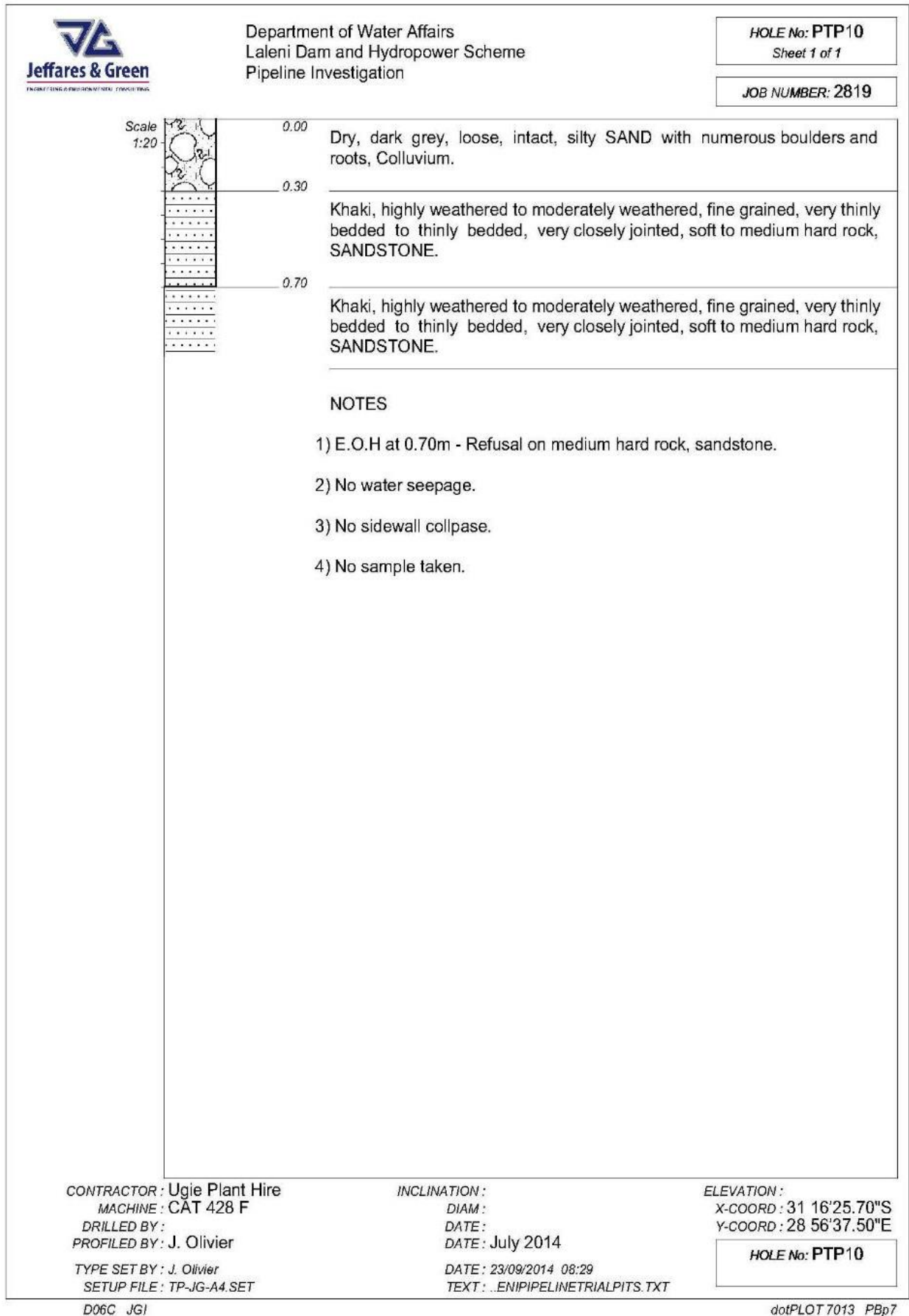


Fig F-1.10: Pipeline Trial Pit 10

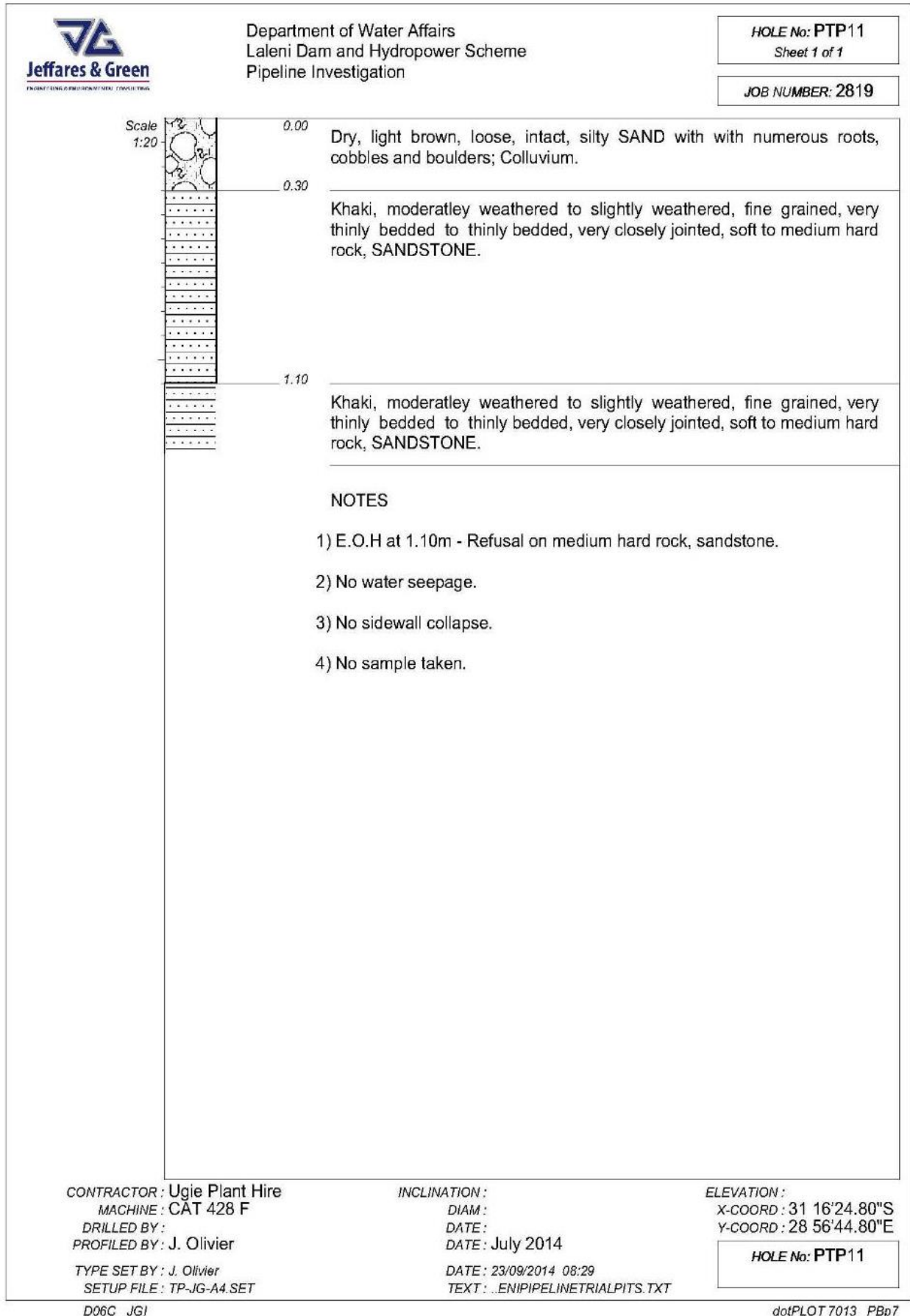


Fig F-1.11: Pipeline Trial Pit 11

F2:

CORE BORROW PIT TRIAL PIT LOGS

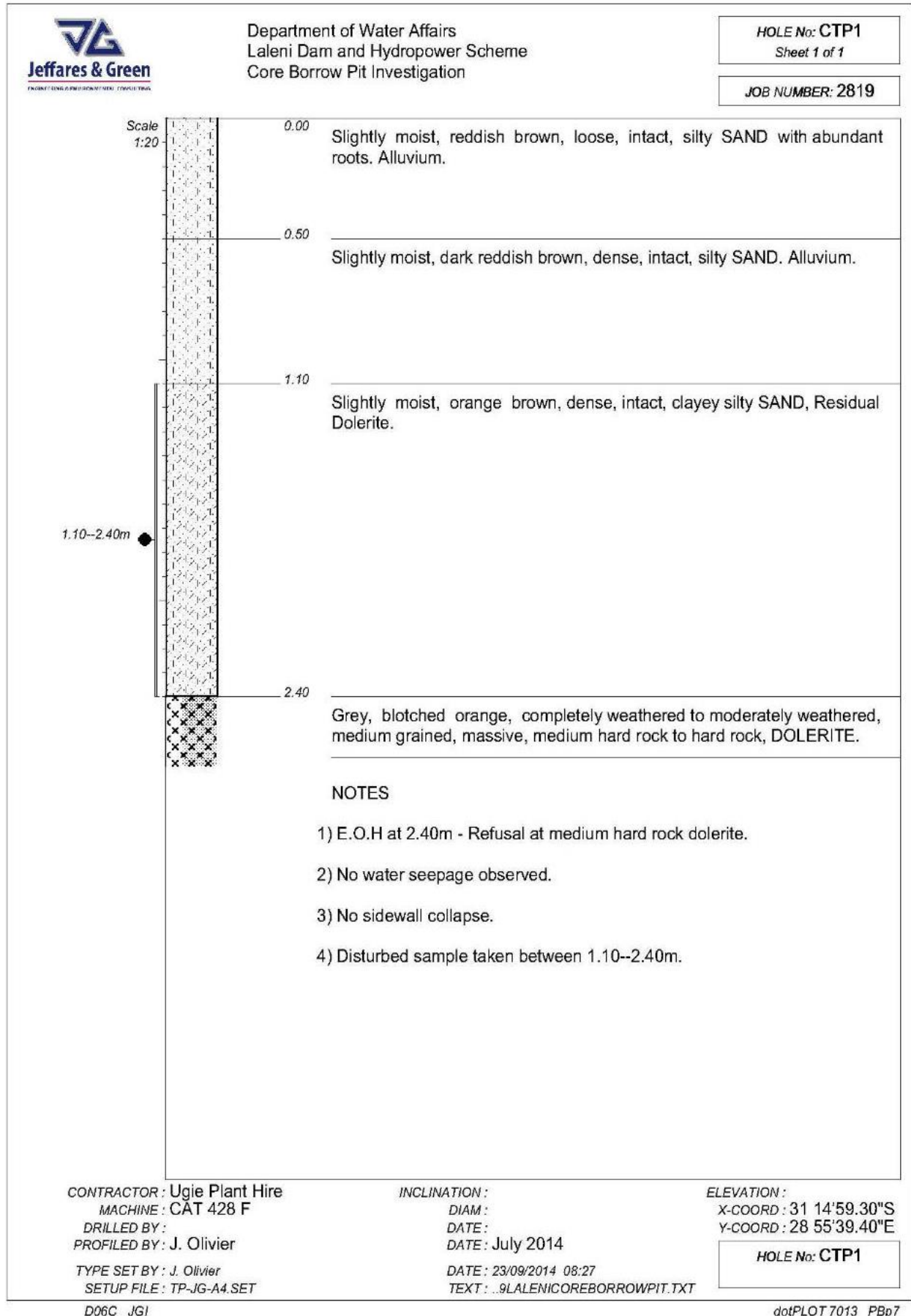


Fig F-2.1: Core Borrow Pit 1

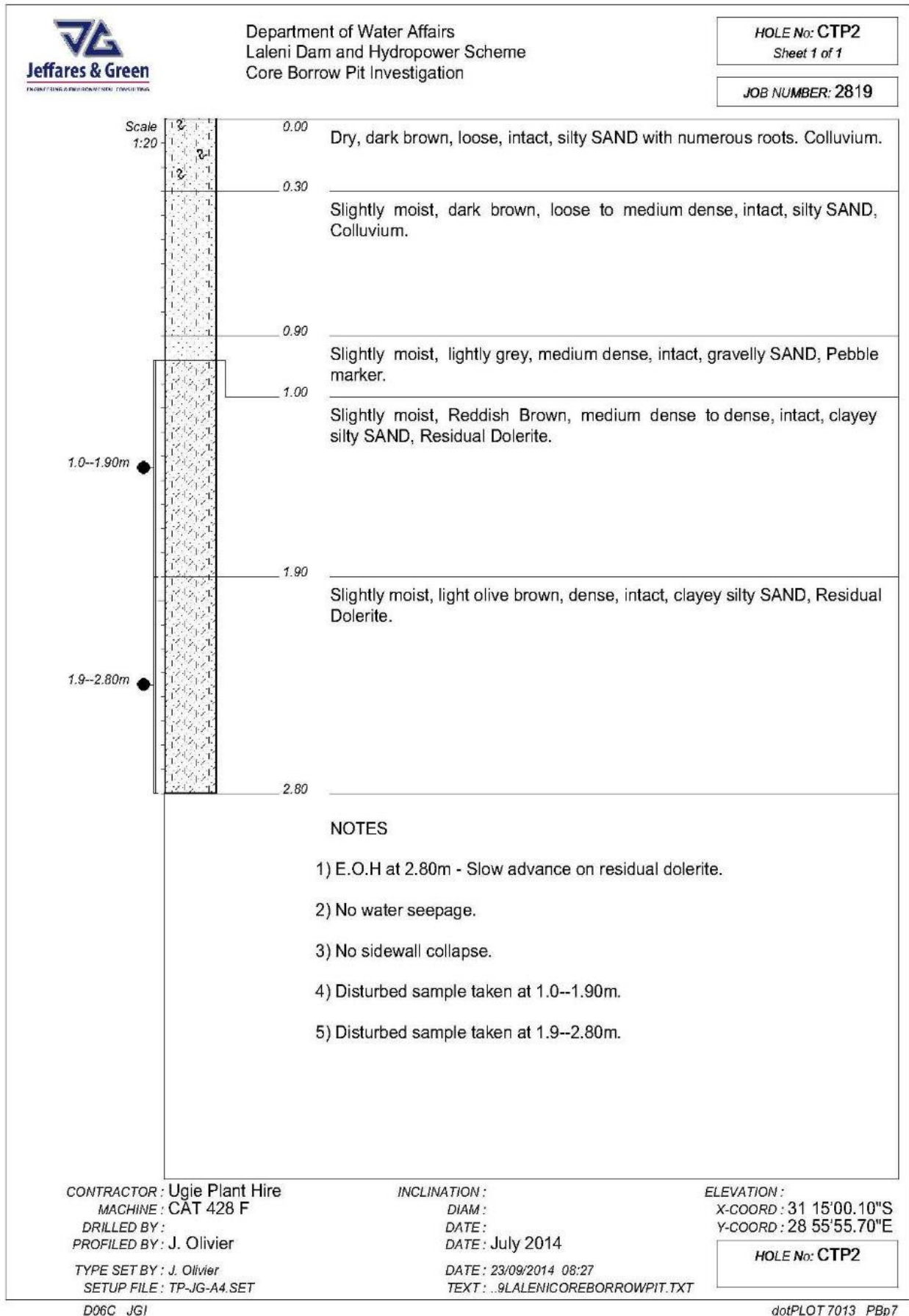


Fig F-2.2: Core Borrow Pit 2

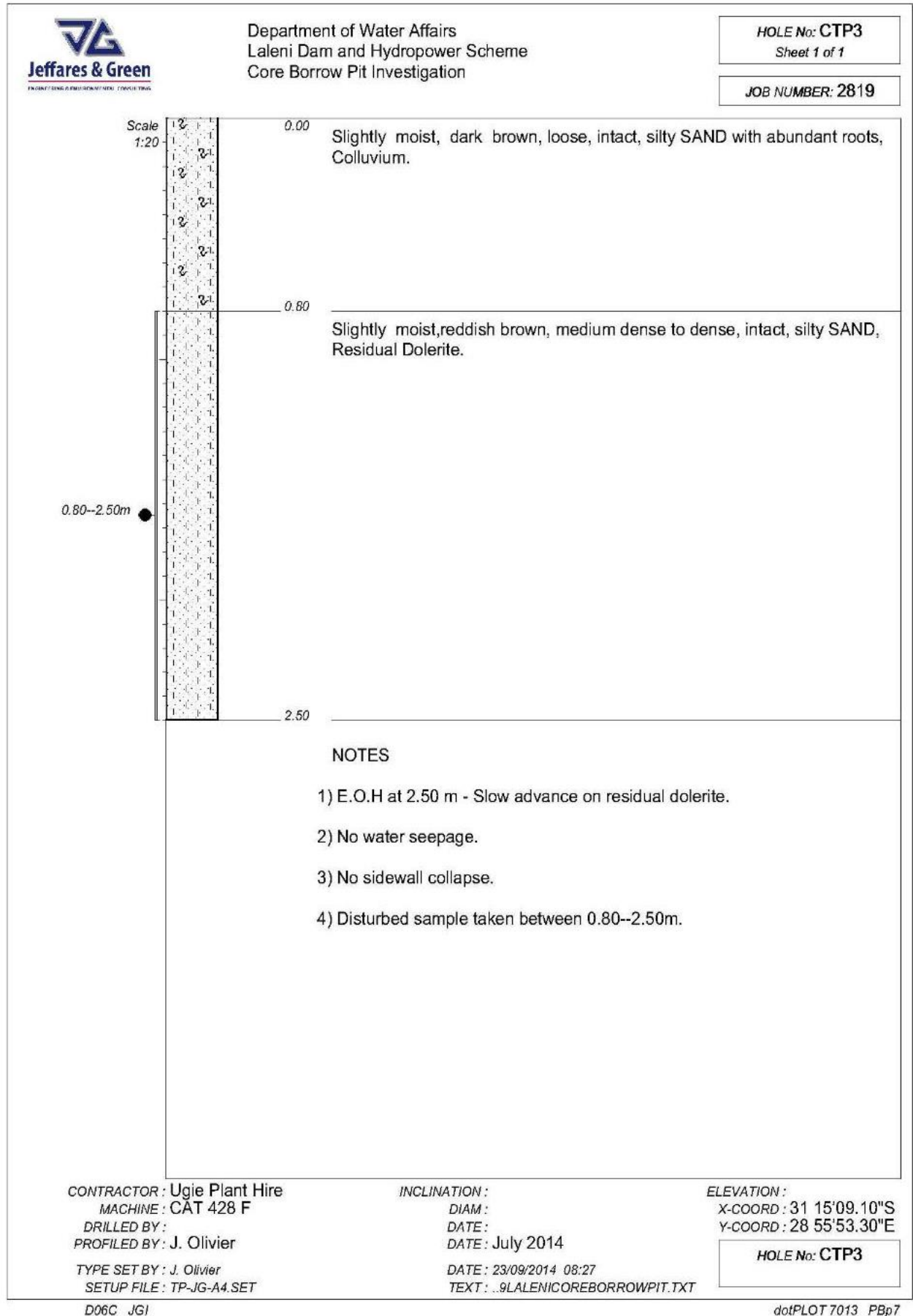


Fig F-2.3: Core Borrow Pit 3

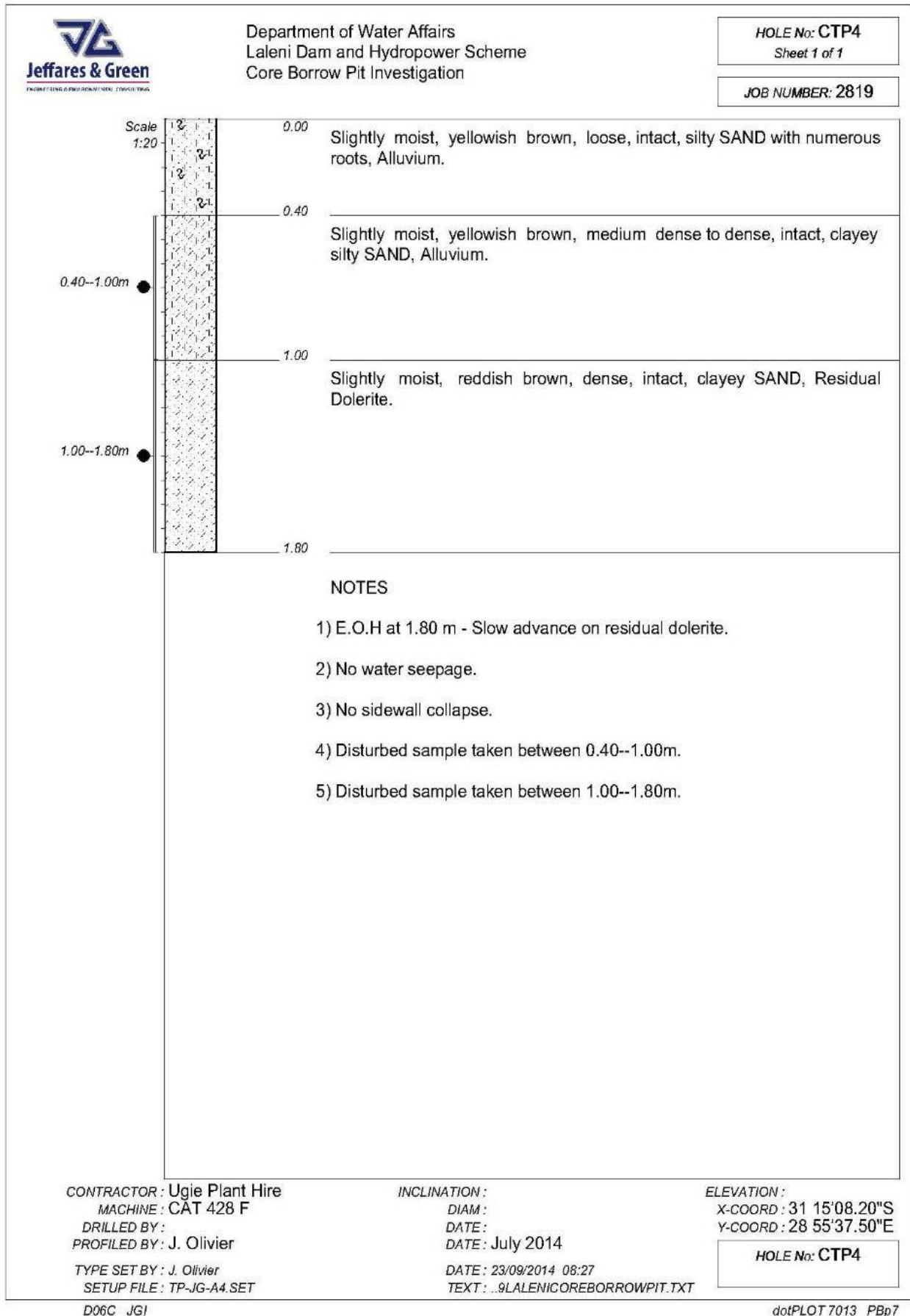


Fig F-2.4: Core Borrow Pit 4

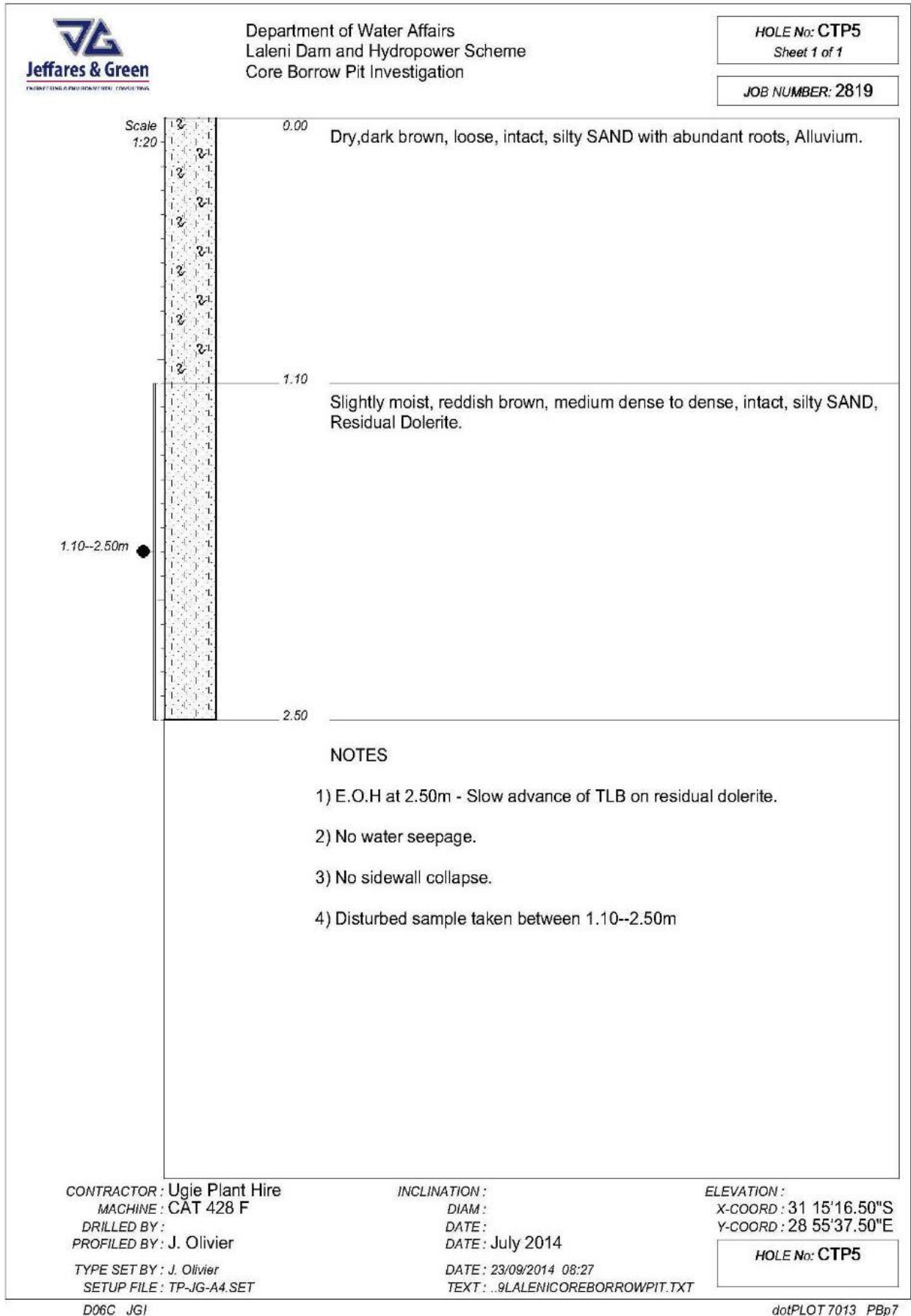


Fig F-2.5: Core Borrow Pit 5

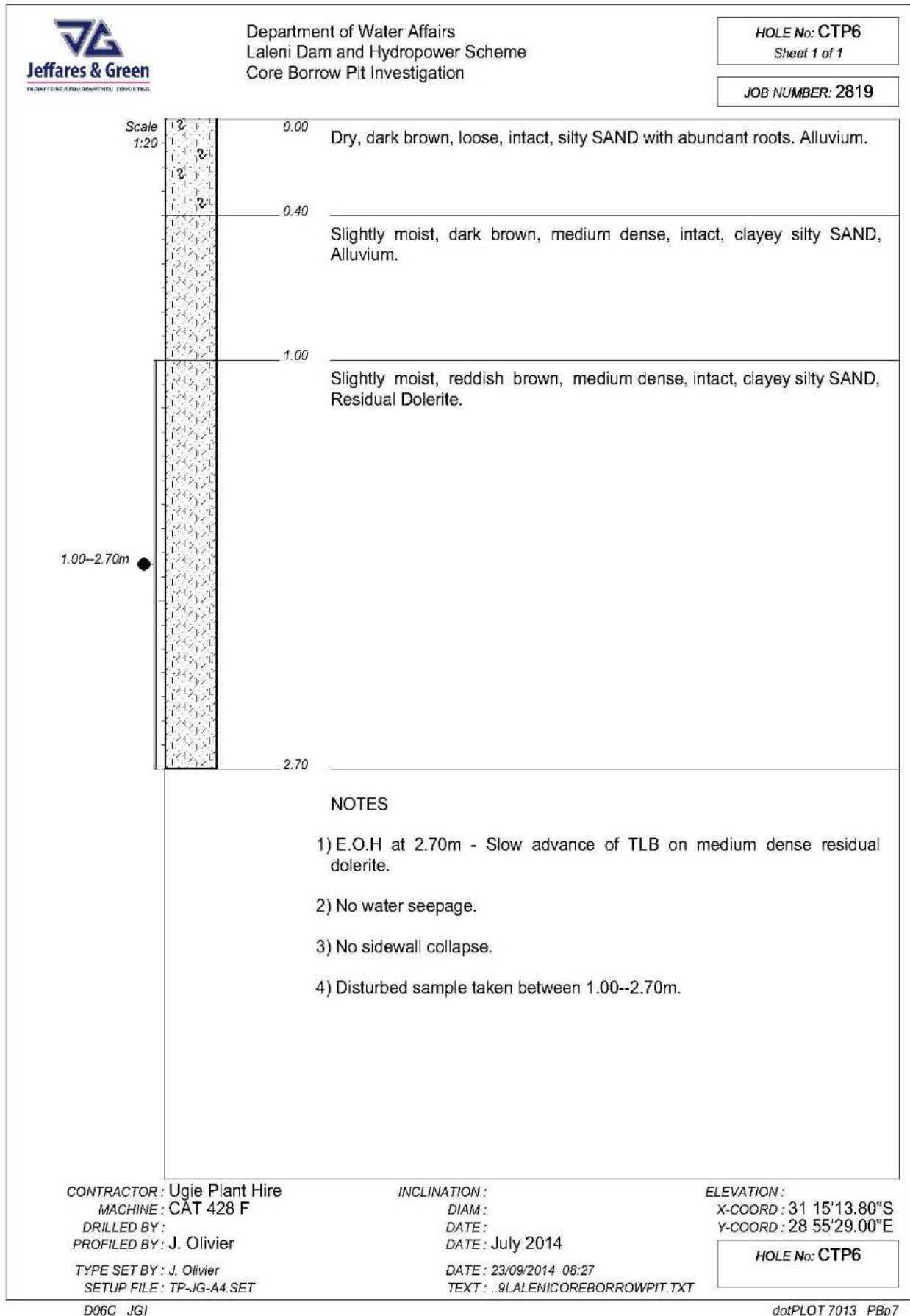


Fig F-2.6: Core Borrow Pit 6

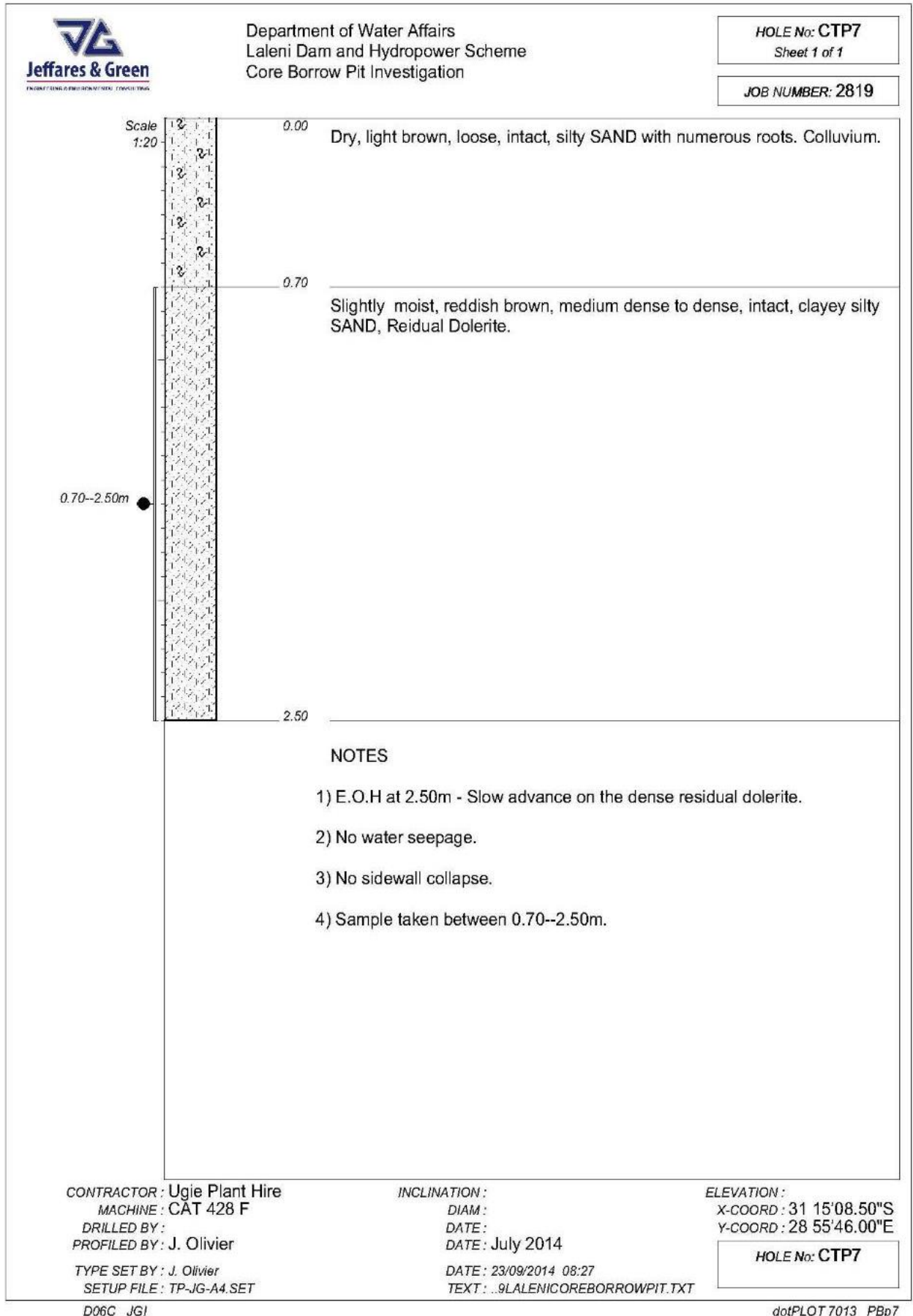


Fig F-2.7: Core Borrow Pit 7

F3:

SHELL BORROW PIT LOGS

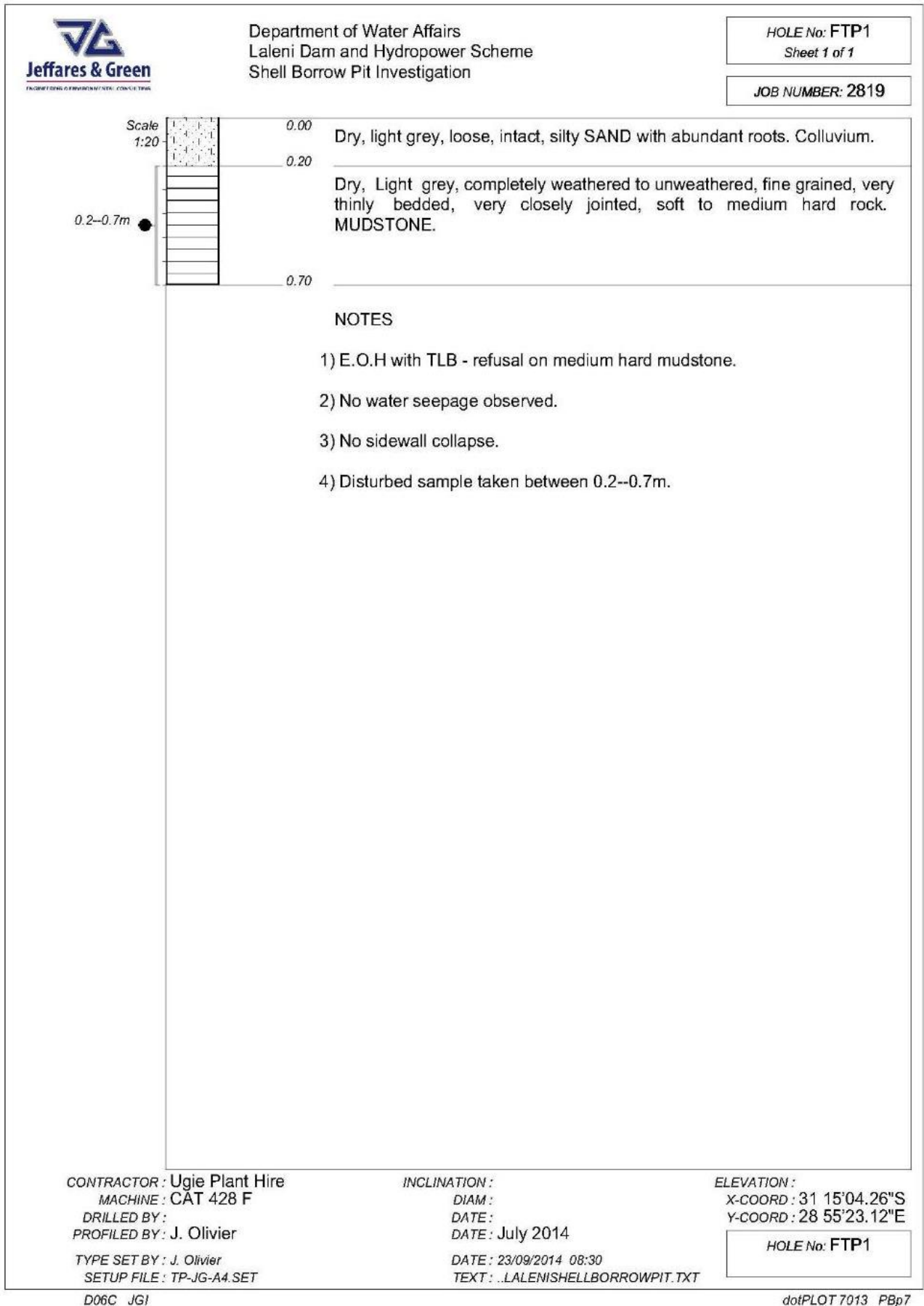


Fig F-3.1: Shell Borrow Pit 1

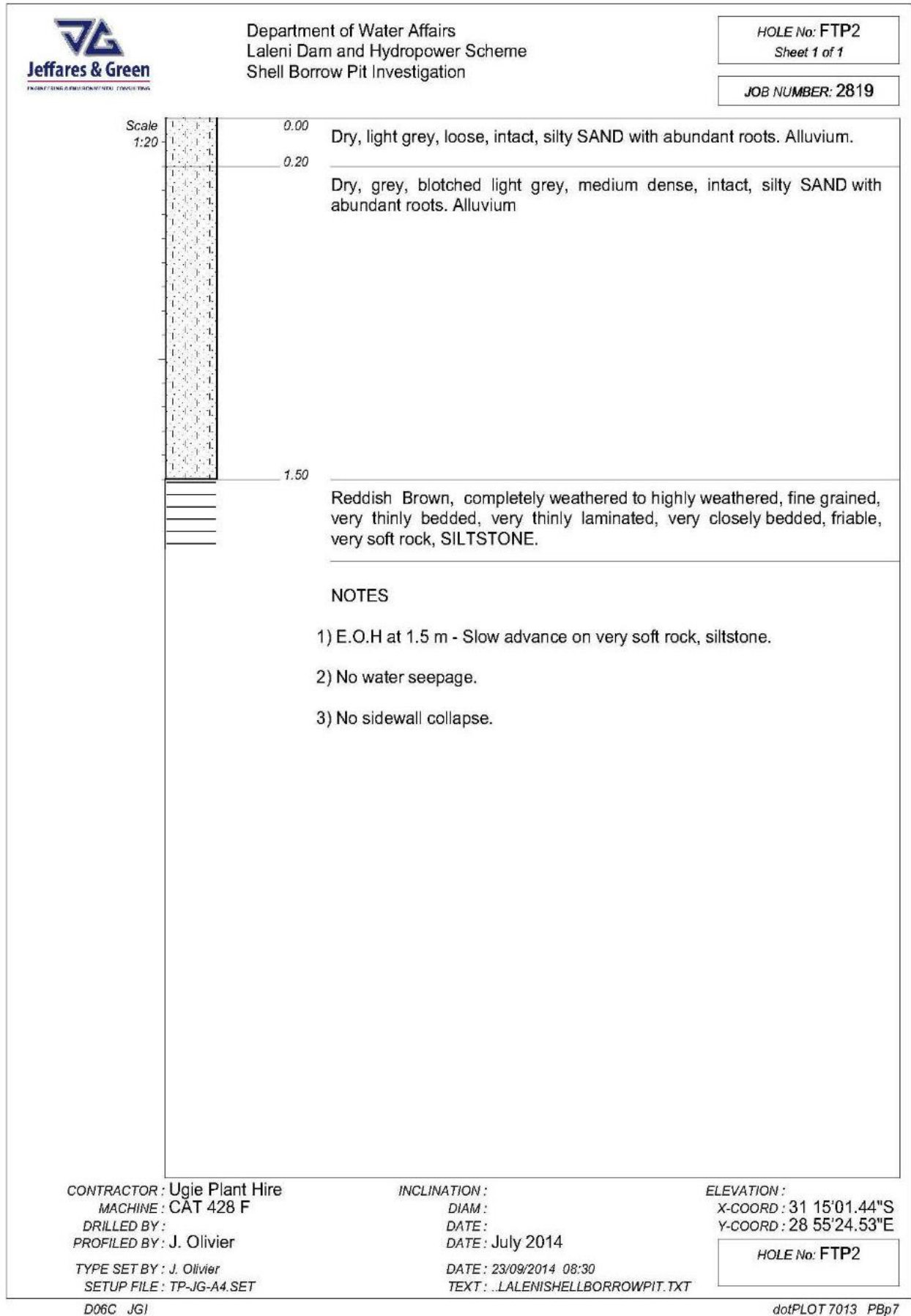


Fig F-3.2: Shell Borrow Pit 2

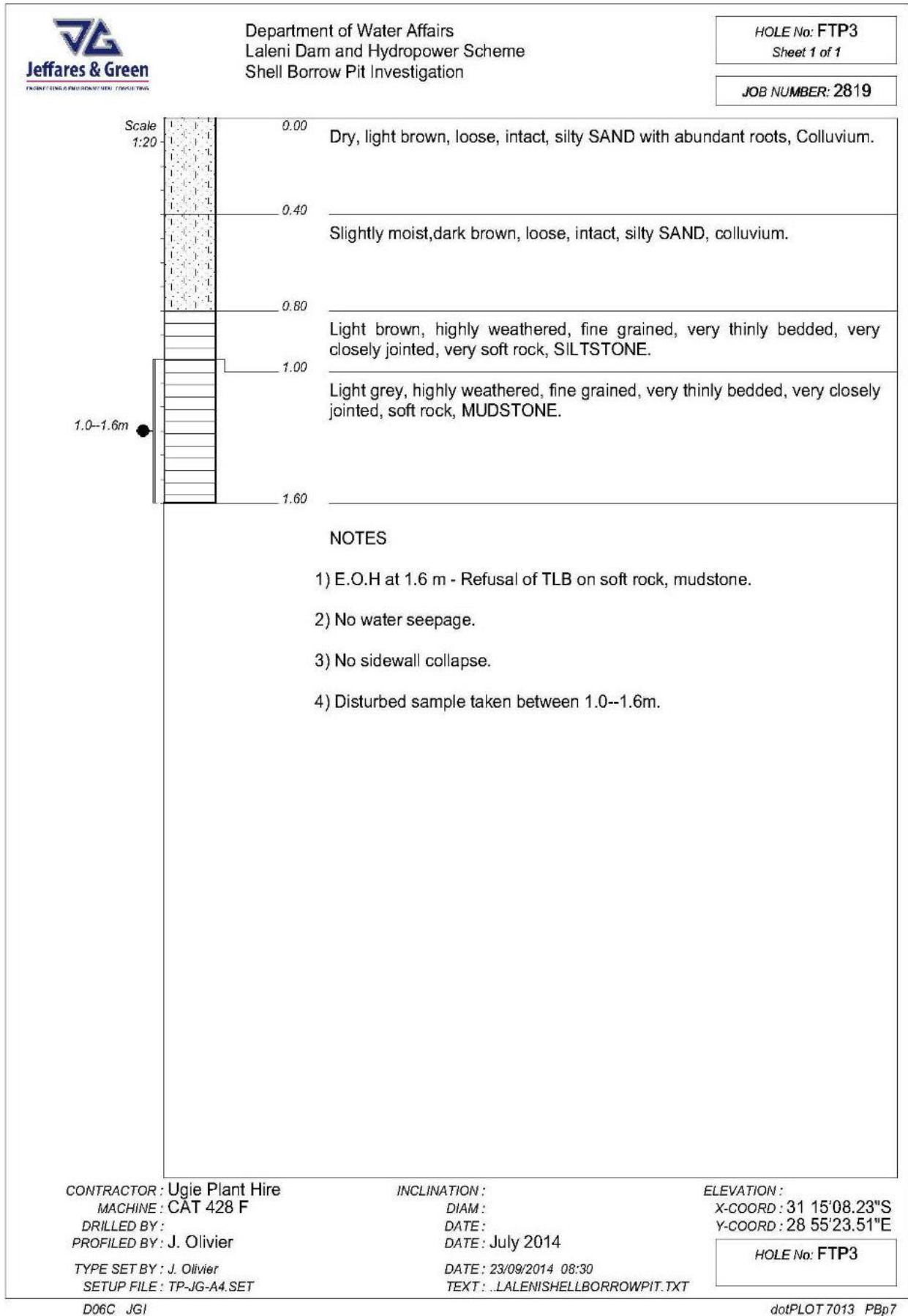


Fig F-3.3: Shell Borrow Pit 3

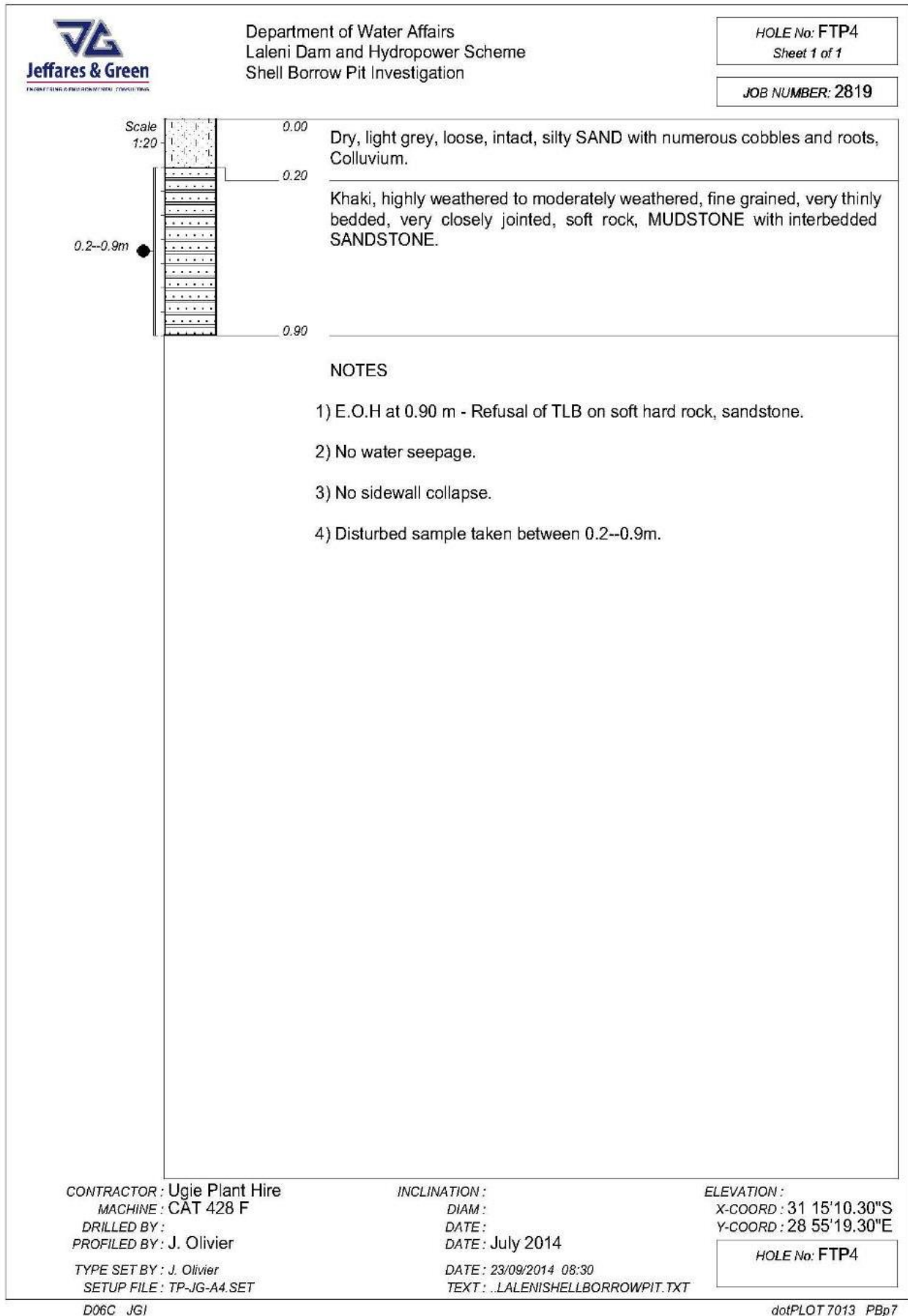


Fig F-3.4: Shell Borrow Pit 4

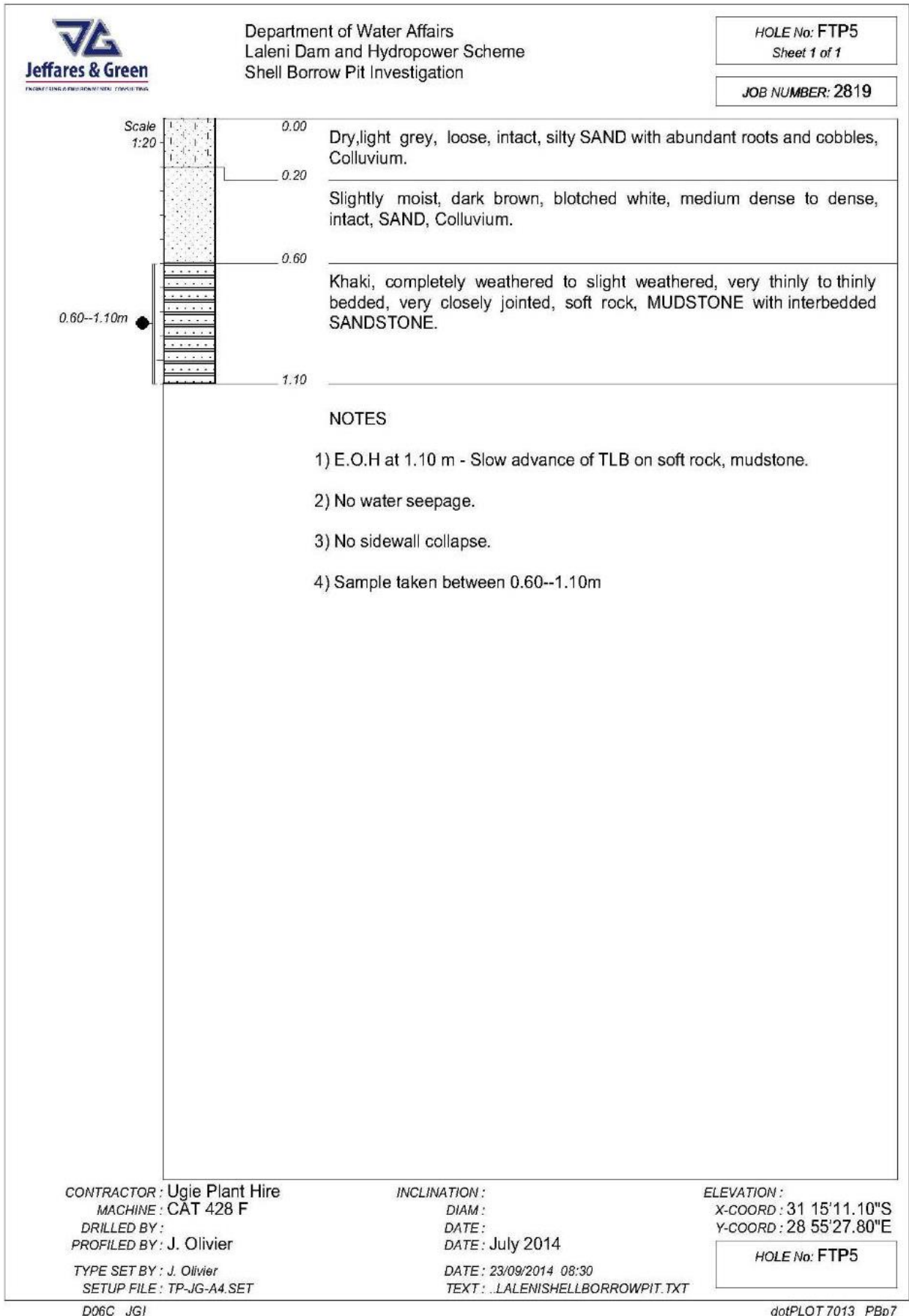


Fig F-3.5: Shell Borrow Pit 5

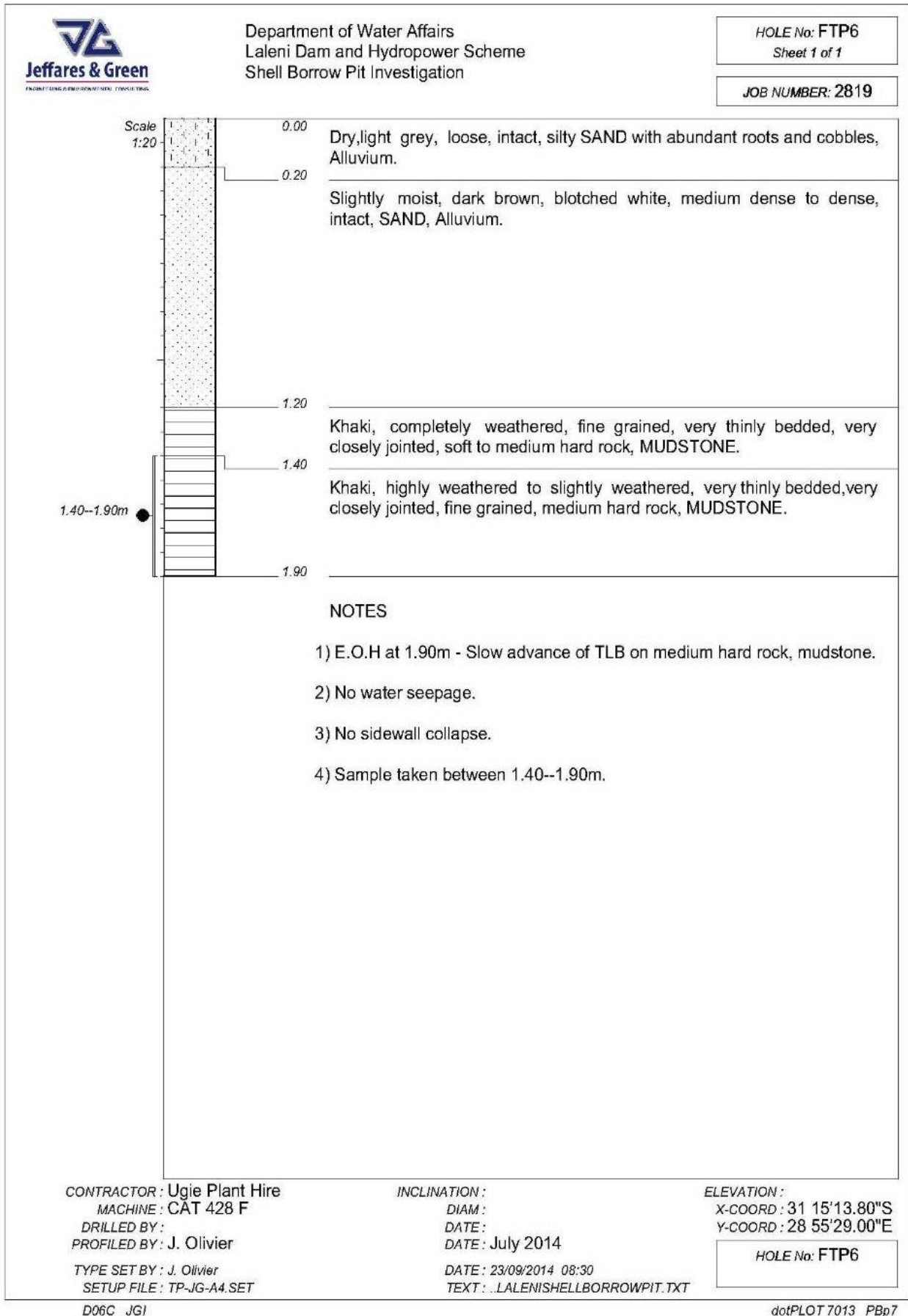


Fig F-3.6: Shell Borrow Pit 6

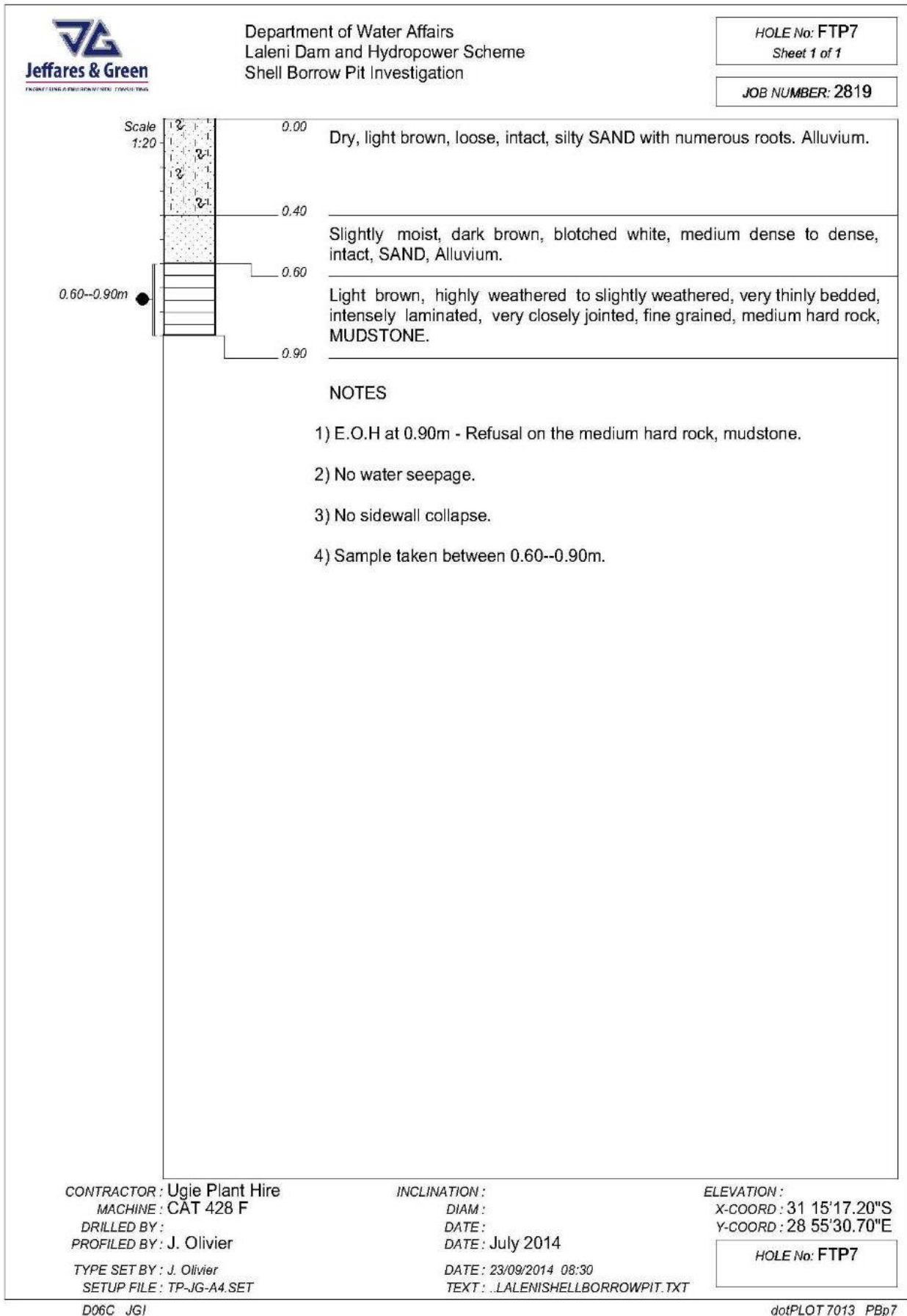


Fig F-3.7: Shell Borrow Pit 7

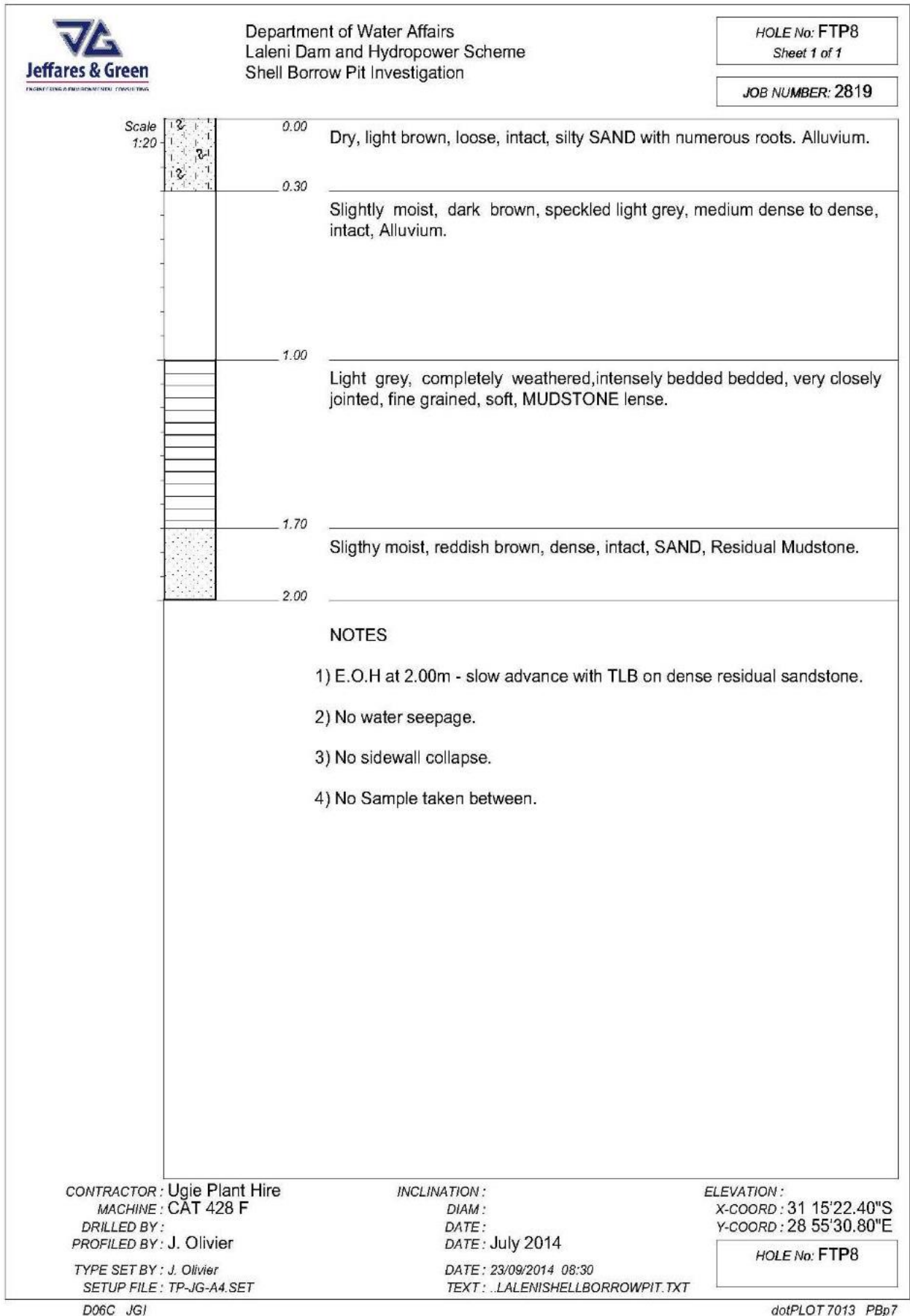


Fig F-3.8: Shell Borrow Pit 8

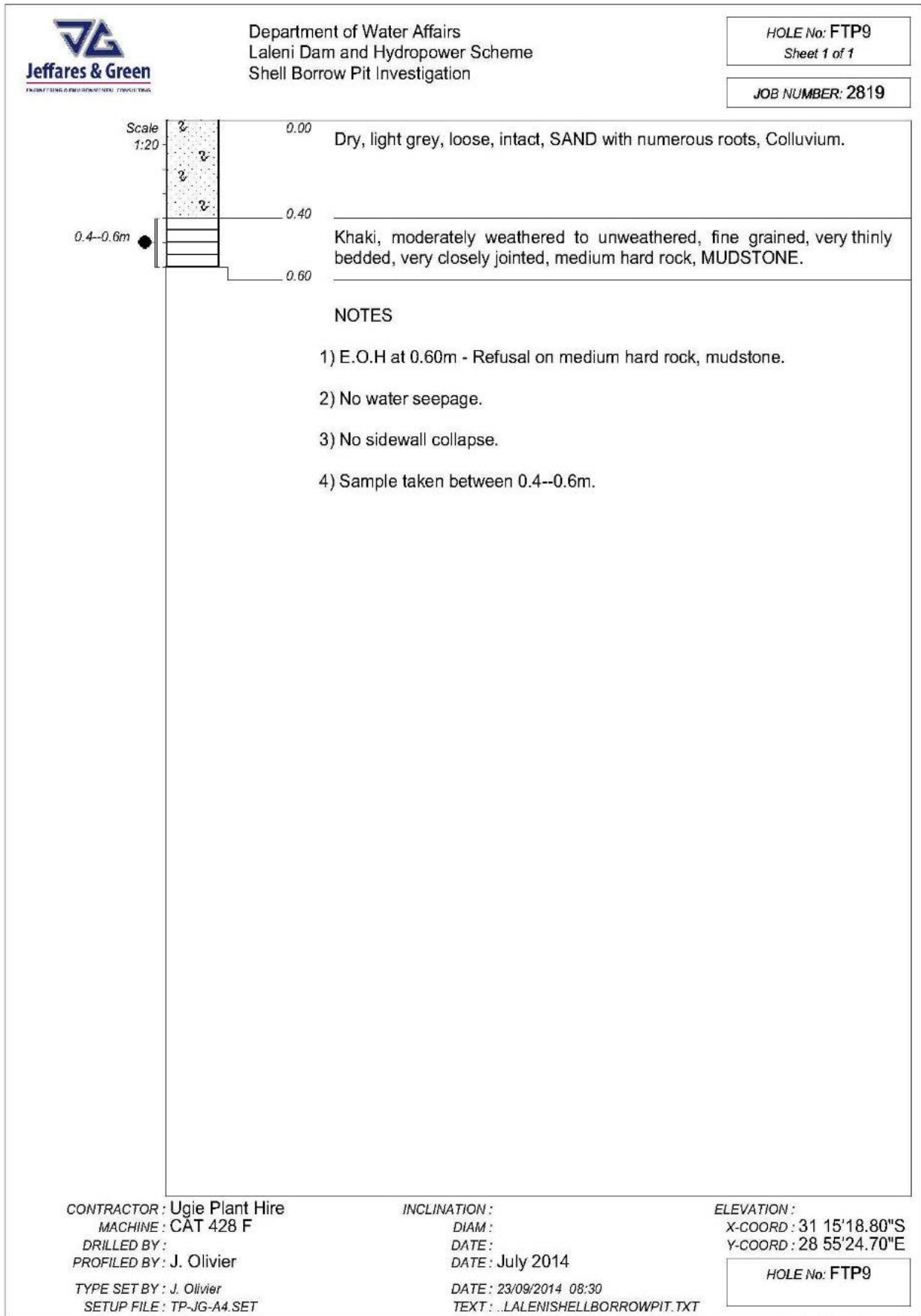


Fig F-3.9: Shell Borrow Pit 9

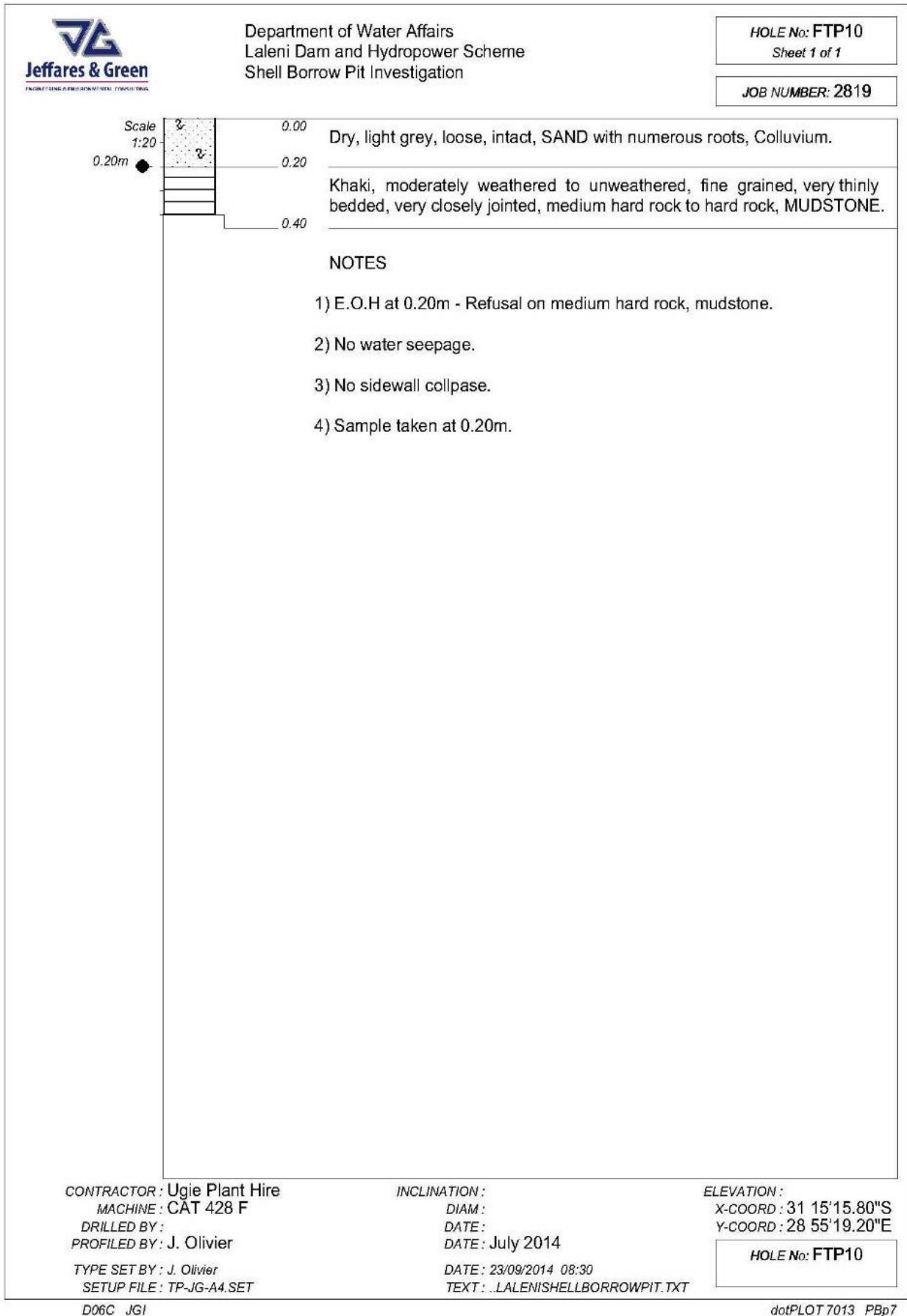


Fig F-3.10: Shell Borrow Pit 10

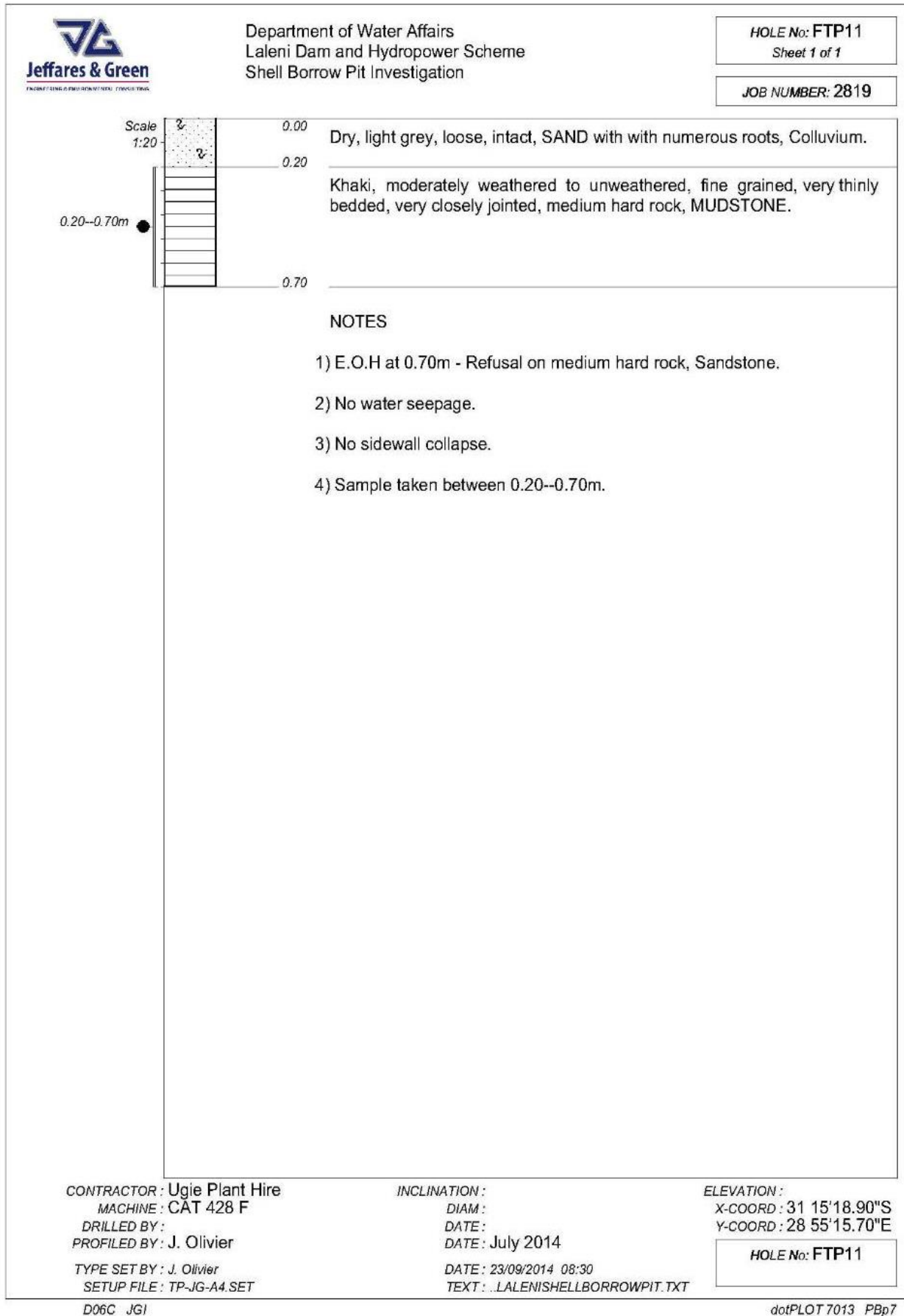


Fig F-3.11: Shell Borrow Pit 11

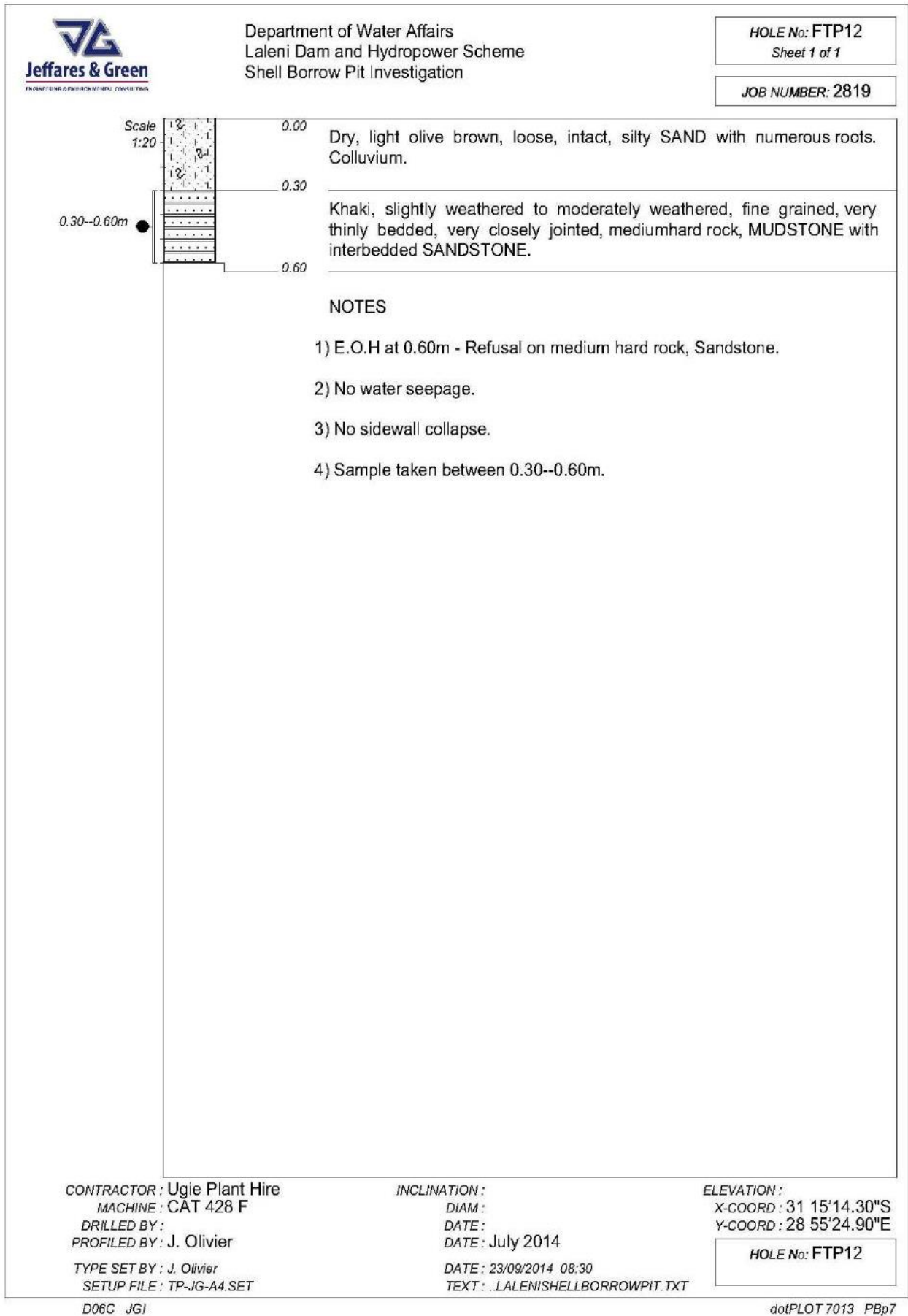


Fig F-3.12: Shell Borrow Pit 12

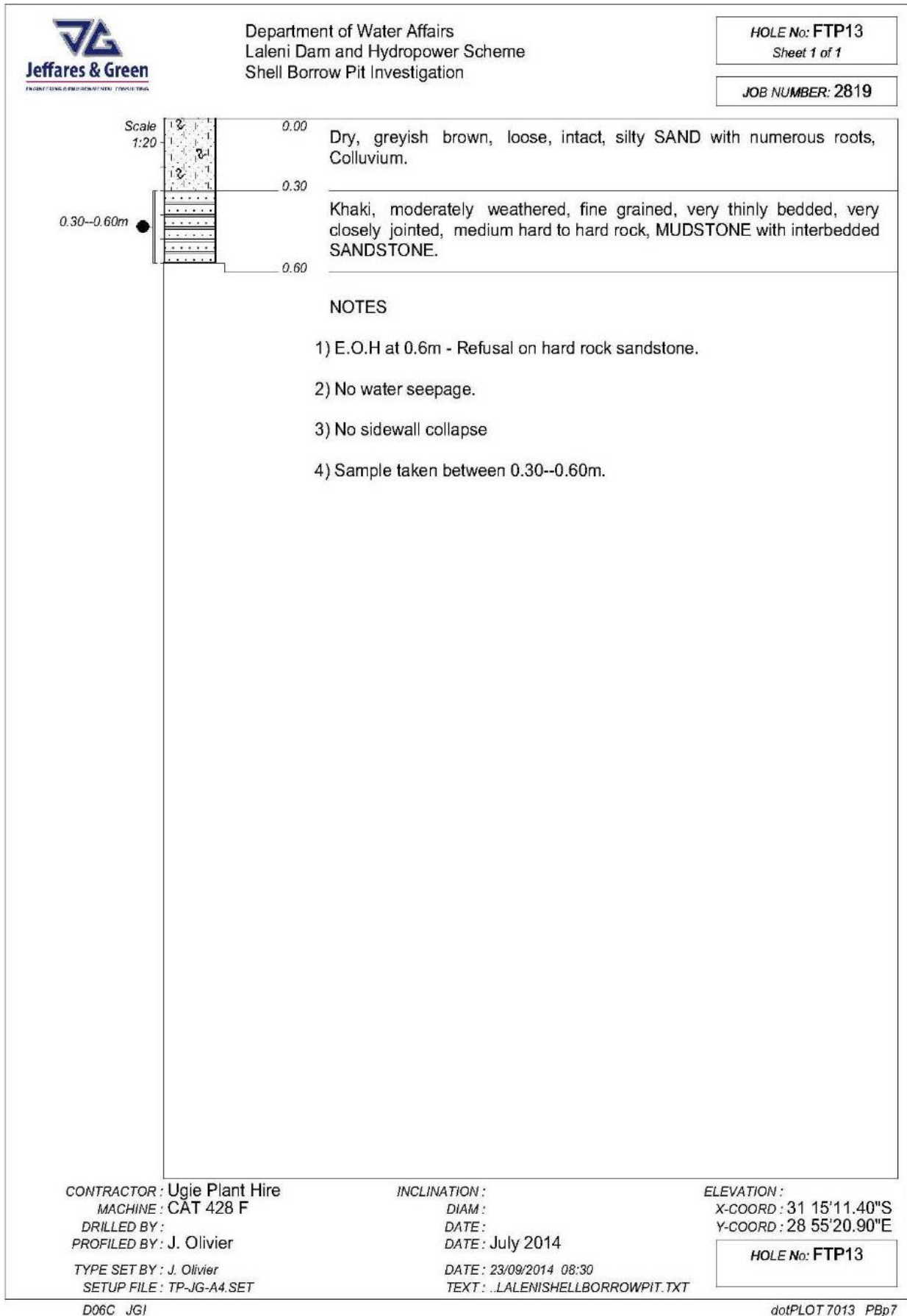


Fig F-3.13: Shell Borrow Pit 13

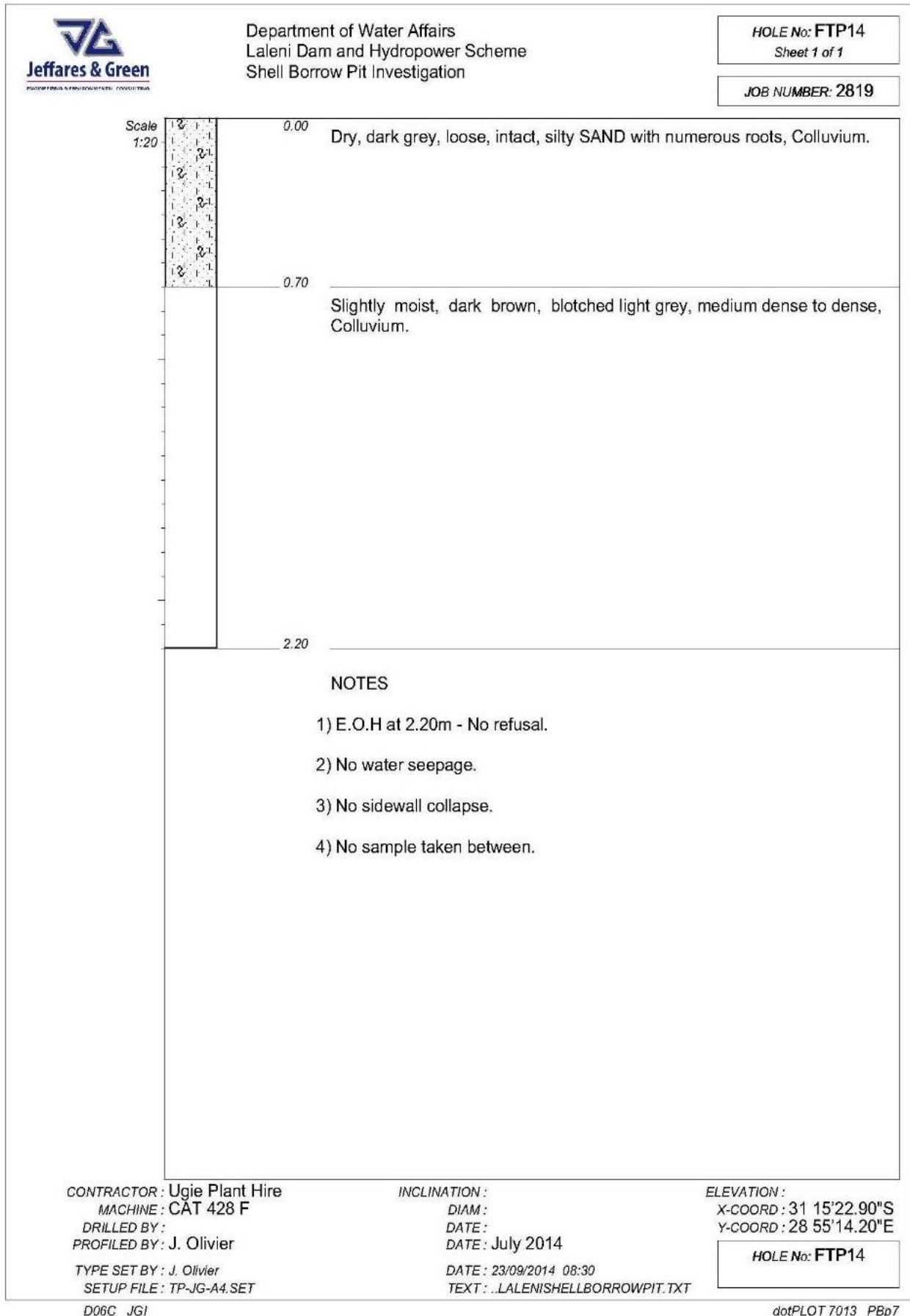


Fig F-3.14: Shell Borrow Pit 14

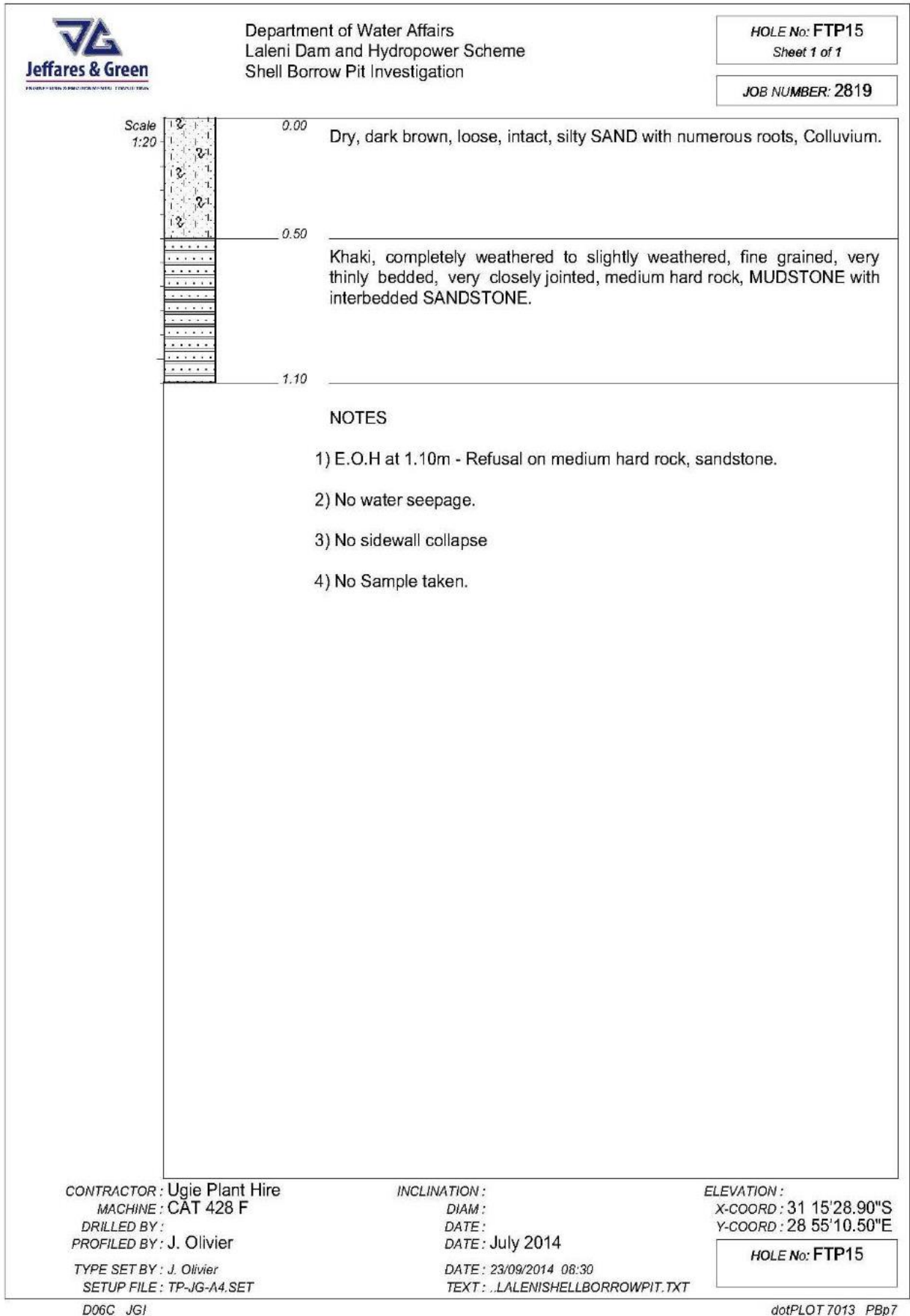


Fig F-3.15: Shell Borrow Pit 15